

HIGHER GROUND EDUCATION, INC LOCAL AREA TRANSPORTATION REVIEW

MONTGOMERY COUNTY, MARYLAND

July 23, 2021



HIGHER GROUND EDUCATION, INC

Local Area Transportation Review Montgomery County, Maryland

July 23, 2021

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HIGHER GROUND EDUCATION, INC

Section 1 INTRODUCTION

OVERVIEW

This report details a Local Area Transportation Review (LATR) for Guidepost A LLC c/o Higher Ground Education, Inc, the "Applicant", located at 7108 Bradley Boulevard, Bethesda, Maryland. The subject site is located along the south side of Bradley Boulevard (MD 191) within the Bethesda/Chevy Chase (Orange) Policy Area of Montgomery County, as shown on Figure 1. All study intersections are located within the Bethesda/Chevy Chase (Orange) and/or the Potomac (Yellow) Policy Areas of Montgomery County. This study was prepared in support of the Special Exception Modification and to satisfy the 2021 LATR requirements in accordance with the Maryland-National Capital Park and Planning Commission (M-NCPPC) guidelines.

The subject property is currently occupied by the Rochambeau French International School with restrictions on hours of operation during the AM and PM Peak periods. The Applicant will retain the private educational institution use within the existing facilities with modified hours of operation, from the current hours of 9:00 AM to 3:20 PM to 7:00 AM to 7:00 PM with a capacity of up to 180 children/students. Vehicular access is currently provided via a single curb cut along Bradley Boulevard (MD 191), and no changes to access are proposed within this application. The existing school building and parking lot will be used without any proposed increases to the building or parking lot footprint.

This application is subject to Local Area Transportation Review (LATR) since the proposed development is expected to generate 50 or more new peak hour person trips during the AM and PM peak periods. The scope of this LATR traffic study was established in consultation with M-NCPPC, Maryland State Highway and Montgomery County Department of Transportation (MCDOT) Staff. The Scope of Work Agreement is included in Appendix A. The Scope of work Agreement in Appendix A is based on the original request for a maximum of 200 students. Higher Ground Education, Inc has reduced the capacity request to a maximum of 180 students. Therefore, the LATR requirements/thresholds contained in this report have been slightly reduced based on the reduced student capacity.



EXECUTIVE SUMMARY

The Special Exception Modification proposes to change the current hours of operation from 9:00 AM to 3:20 PM (existing) to 7:00 AM to 7:00 PM (proposed) with a capacity of up to 180 children/students the subject property is located in the Bethesda/Chevy Chase Policy Area (Orange) of Montgomery County.

The application is subject to all of the 2021 LATR adequacy test. These include the Vehicle, Bicycle, Pedestrian, Transit and Vision Zero Adequacy Tests. A summary of the findings and mitigation for each is provided below:

- The AM and PM peak vehicle delays and/or CLVs at the study intersections operate well
 within the Bethesda/Chevy Chase Policy Area congestion standard of 80.0
 seconds/vehicle and Potomac Policy Area CLV congestion, under existing, background and
 total future conditions. No mitigation is required by this application to pass the motor
 vehicle adequacy test.
- The site does not pass the Bicycle System Adequacy Test within the applicable study area. A shared use path, for both bicyclists and pedestrians, is recommended for the north side of Bradley Boulevard from Redwood Avenue to Oak Forest Lane, between the proposed bus shelters, to satisfy the required mitigation.
- There are no existing ADA ramps or sidewalks within the required study area. Therefore, no ADA corrective measures are needed.
- The site does not pass the Pedestrian System Adequacy test due to the lack of pedestrian facilities. As noted in the Bicycle System Adequacy Test section, a shared use path is recommended for the north side of Bradley Boulevard from Redwood Avenue to Oak Forest Lane to satisfy the required mitigation.
- The site does not pass the Transit System Adequacy Test due to a lack of bus shelters. The installation of two bus shelters is recommended to satisfy the required mitigation.
- An evaluation of vision zero standards included a review of accidents and speed studies.
 They were in a high accident location within the study area. The speed data within the study area indicates that the 85th percentile speed of vehicles traveling along Bradley Boulevard is in excess of the 35-mph posted speed limit.



DESCRIPTION OF MULTI-MODAL ADEQUACY TESTS

The following section describes the various multi-modal tests for determining transportation adequacy per the LATR guidelines and the recently enacted Montgomery County Growth Policy:

Motor vehicle adequacy (Red Policy Areas are Exempt from this test). This test is required for any development generating 50 or more peak hour person trips. Intersections located within Orange policy areas are evaluated for adequacy using the Highway Capacity Manual analyses methodology. Intersections within Yellow policy areas are evaluated using the Critical Lane Volume (CLV) analyses methodology. The congestion standard (HCM delay based) for isolated signalized intersections in the Bethesda/Chevy Chase policy area is an overall average vehicle delay of 80 seconds per vehicle. The congestion standard (CLV) for intersections in the Potomac policy area is a CLV of 1,450. The scope of the study is based on the motor vehicle trip generation. For sites generating less than 250 peak hour trips, the study area is required to include a minimum of one significant intersection in each direction. The proposed application will generate less than 250 peak hour vehicle trips and is required to study at least one significant intersection in each direction from the site. The following study area was identified in consultation with Staff during the scoping process:

- 1. Site Driveway / Bradley Boulevard (Bethesda/Chevy Chase Policy Area)
- 2. Seven Locks Road / Bradley Boulevard (Potomac Policy Area)
- 3. Burdette Road / Bradley Boulevard (Bethesda/Chevy Chase Policy Area)
- 4. Fernwood Road / Bradley Boulevard (Bethesda/Chevy Chase Policy Area)

<u>Bicycle system adequacy</u>. This analysis considers the following:

Bicycle system adequacy is defined as providing a low Level of Traffic Stress (LTS-2) for bicyclists. Bicycle system analysis will be based on the following standards and scoping:

"For any site generating at least 50 peak-hour person trips, conduct an analysis of existing and programmed conditions to ensure low Level of Traffic Stress (LTS-2) conditions on all transportation rights-of-way within a certain distance of the site frontage, specified in Table 12. If current and programmed connections will not create adequate conditions, the applicant must construct side paths, separated bike lanes, or trails, consistent with the Bicycle Master Plan, that create or extend LTS-2 conditions up to the specified distance from the site frontage."

Based on the expected person trips to be generated by this site, the required distance for the bicycle study area is within 750' of the site.

<u>Pedestrian System Adequacy.</u> This analysis includes three components:

"Pedestrian Level of Comfort (PLOC). Pedestrian system adequacy is defined as providing a "Somewhat Comfortable" (PLOC-2) or "Very Comfortable" (PLOC-1) score on streets and



intersections for roads classified as Primary Residential or higher (excluding Controlled Major Highways and Freeways, and their ramps), within a certain walkshed from the site frontage."

"Street Lighting. The applicant must evaluate existing street lighting based on MCDOT standards along roadways or paths from the development to destinations within a certain walkshed from the site frontage."

"ADA Compliance. The applicant must fix Americans with Disabilities Act (ADA) noncompliance issues within a certain walkshed from the site frontage equivalent to half the walkshed specified."

Based on the expected person trips to be generated by this site, the required distances for the three components of the pedestrian study area are as follows:

- Pedestrian Level of Comfort and streetlighting study area
 750' in in all directions from the property
- ADA Compliance study area
 375' in all directions from the site

Bus Transit System Adequacy. This analysis considers the following:

"For any site generating at least 50 net new weekday peak-hour person trips in Red, Orange, and Yellow policy areas, bus transit system adequacy for LATR is determined by the conduct an analysis of existing and programmed conditions to ensure that there are bus shelters outfitted with real-time travel information displays and other standard amenities, along with a safe, efficient, and accessible path between the site and a bus stop, at a certain number of bus stops within a certain distance of the site frontage."

Based on the expected person trips to be generated by this site, the required distance for the transit study area is within 1000' of the site.

LATR Vision Zero Statement. This component considers the following:

"All LATR studies for a site that will generate 50 or more peak-hour person trips must develop a Vision Zero Statement. This statement must assess and propose solutions to high injury network and safety issues, review traffic speeds, and describe in detail how safe site access will be provided. With concurrence of the responsible agency, projects must implement or contribute to the implementation of safety countermeasures. The County Council may adopt predictive safety analysis as part of this statement, when available."

1. "Review High Injury Network segments: Document any segments on the High Injury Network (HIN) that are within a certain distance of the site frontage, as specified in Table 1."



- 2. "Assess proximate safety issues: Review the crash history for all segments and crossings within a certain distance of the site frontage, as specified in Table 1."
- 3. "Review traffic speeds: Conduct speed studies within a certain distance from the site frontage, specified locations and timing for the speed study will be determined by Planning staff in collaboration with MCDOT staff and will prioritize filling in gaps in the inventory of speed studies. Relevant speed studies that have been completed within the past three years may be used to fulfill this requirement if gaps do not remain in the inventory of speed studies."
- 4. "Describe site access: Address the safety issues identified in steps 1 through 3 and describe how site circulation promotes safety, outlining how safe access will be provided to the site. Note if the applicant is contributing a fee in lieu of constructing a countermeasure. Reference the Vision Zero Community Toolkit (forthcoming) or national best practices and research in outlining the appropriate treatments to address identified safety issues."

Tasks undertaken in this study included the following:

- Preparation of Motor Vehicle Adequacy Test
- Preparation of Bicycle System Adequacy Test
- Preparation of Pedestrian System Adequacy Test
- Preparation of Bus Transit System Adequacy Test
- Preparation of Vision Zero Statement
- Review of the proposed plans, background materials provided, and the Local Area Transportation Review Guidelines requirements for the Bethesda/Chevy Chase Policy Area.
- Coordination with M-NCPPC Staff to identify the necessary scope and analyses to be included in the LATR study.
- Collection of new vehicular turning movement, bicycle, and pedestrian counts at the study intersections.
- Adjusting the newly collected traffic count data using a factor of 1.07, as applicable, per the current Montgomery County policy on collecting traffic data.
- Calculation of adjusted existing conditions average vehicle delay and/or CLV for each study intersection.
- Forecast background future traffic volumes by compiling the adjusted existing peak hour traffic volumes and the traffic forecasted to be generated by pipeline projects that are currently approved or planned for development.
- Calculation of background future peak hour conditions average vehicle delay and/or CLV for each study intersection based on the background future traffic forecasts and the existing or planned intersection geometrics.
- Estimation of the number of AM and PM peak hour vehicle trips that will be generated by the proposed private school/daycare use based on the Trip



Generation Manual, 10th Edition, published by the Institute of Transportation Engineers and the Bethesda/Chevy Chase policy area adjustment factors.

- Assignment of the site trips based on previously approved distributions for the subject site.
- Forecast of total future traffic volumes by combining the new site trips generated by the proposed child daycare facility with the background traffic forecasts.
- Calculation of total future peak hour conditions average vehicle delay and/or CLV for each study intersection based on the total future traffic forecasts and existing or planned intersection geometrics.
- Review of available public bus transportation options proximate to the site.

Sources of data for this study include: the M-NCPPC, the MCDOT, the Maryland State Highway Administration (SHA), the Maryland Transit Administration (MTA), Institute of Transportation Engineers (ITE), and Wells + Associates Inc.





Figure 1 Site Plan



WA

Higher Ground Education Montgomery County, MD

SECTION 2 BACKGROUND DATA

OVERVIEW

This section presents the following background information for the LATR:

- Description of the proposed site user.
- Description of the existing vehicular ingress/egress.
- Definition of the study area.
- Description of the study area public road network and transportation facilities.
- Vehicular, pedestrian and bicycle traffic counts.

PLANNED SITE USER

This study considers a similar private school/daycare user for the subject property with modification of the hours of operation, from the current hours of 9:00 AM to 3:20 PM that would allow the user to operate from 7:00 AM to 7:00 PM with a capacity up to 180 children/students.

VEHICULAR ACCESS

The site is currently accessed via an existing site driveway located along the south side of Bradley Boulevard (MD 191). No changes to the existing site access are proposed.

PARKING

The new site user will occupy the existing building and utilize the existing surface parking area.

PUBLIC ROAD NETWORK

Existing Network/Site Access

Regional access and local direct vehicular access to the site is provided by Bradley Boulevard (MD 191).

<u>Bradley Boulevard (MD 191)</u> is a state-owned two-lane undivided roadway adjacent to the site. Traffic signals and additional turn lanes are typically provided at major intersections. Bradley Boulevard (MD 191) has a posted speed limit of 35 mph.

PROGRAMMED IMPROVEMENTS

No planned or programmed roadway improvements have been identified within the study area.



NON-AUTO TRANSPORTATION FACILITIES

Capital Bikeshare and Bicycle Facilities

The Bicycle Master Plan shows that there are Master Planned bikeways along the study area. Separated bikeways and striped bikeways are proposed on Bradley Boulevard between the Capital Beltway and Fairfax Road. Separated bikeways and bikeable shoulders are proposed on Bradley Boulevard between the Capital Beltway and Persimmon Tree Road.

The nearest Capital Bikeshare station is located adjacent to the National Institute of Health, approximately two (2) miles east of the site, at Old Georgetown Road and Southwick Street. Additional stations are located within downtown Bethesda and at the Bethesda Metro Station.

Sidewalks

Sidewalks are generally not provided within the required study area along Bradley Boulevard or side streets. There is a small section (approximately 50 feet in length) of sidewalk along the northeast quadrant of the Bradley Boulevard/Redwood Avenue intersection.

Metrorail

There are no Metrorail stations within the site vicinity.

WMATA Metrobus

There are no Metrobus Routes within the site vicinity.

Montgomery County RideOn

The site is located within walking distance of RideOn Bus 36, approximately 700' to the west of the property. Additional stops for Route 36 are located near all study intersections along Bradley Boulevard.



STUDY AREA DEFINITION

The study area for this LATR study was established through consultation with M-NCPPC Staff and is contained in the scoping letter provided in Appendix A. The study area assumed in this LATR is consistent with the requirements outlined in M-NCPPC's Local Area Transportation Review guidelines. The following intersections and driveway were included in the study as discussed with M-NCPPC Staff through the scoping process:

- 1. Bradley Boulevard (MD 191) / Site Access (Rochambeau French International School)
- 2. Bradley Boulevard (MD 191) / Seven Locks Road
- 3. Bradley Boulevard (MD 191) / Burdette Road
- 4. Bradley Boulevard (MD 191) / Fernwood Road

Figure 2 shows the existing lane use and traffic control for the study area.

EXISTING TRAFFIC COUNTS

Existing AM and PM peak hour vehicular, pedestrian, and bicycle traffic counts were collected on Thursday, May 6, 2021, at the study intersections, in accordance with the current County traffic count guidelines. Additional details about the current guidelines to address COVID-19 impacts to traffic volumes is provided attached to the scope in Appendix A. Traffic counts were collected at each of the study intersections from 6:30 AM to 9:30 AM and from 4:00 PM to 7:00 PM. Consistent with current County traffic count guidelines to address Covid-19 impacts to traffic volumes, the AM peak hour traffic volumes were multiplied by a factor of 1.07. Further, all PM traffic data occurring before 4:30 PM were also multiplied by the 1.07 adjustment factor. The following is a breakdown of the traffic data used for each intersection:

- 1. Bradley Boulevard (MD 191) / Site Access
 - AM adjustment factor of 1.07. PM peak hour occurred from 5:00-6:00 PM and no PM adjustment factor was included.
- 2. Bradley Boulevard (MD 191) / Seven Locks Road
 - AM adjustment factor of 1.07. PM peak hour occurred from 4:00-5:00 PM with the 4:00-4:30 volumes adjusted with a factor of 1.07.
- 3. Bradley Boulevard (MD 191) / Burdette Road
 - AM adjustment factor of 1.07. PM peak hour occurred from 4:30-5:30 PM and no PM adjustment factor was included.
- 4. Bradley Boulevard (MD 191) / Fernwood Road
 - AM adjustment factor of 1.07. PM peak hour occurred from 4:30-5:30 PM and no PM adjustment factor was included.

Figure 3 shows the adjusted existing AM and PM peak hour vehicular traffic volumes. Pedestrian and bicycle volumes at the study intersections are summarized on Figures 4 and 5, respectively, and the detailed count data is provided in Appendix B.



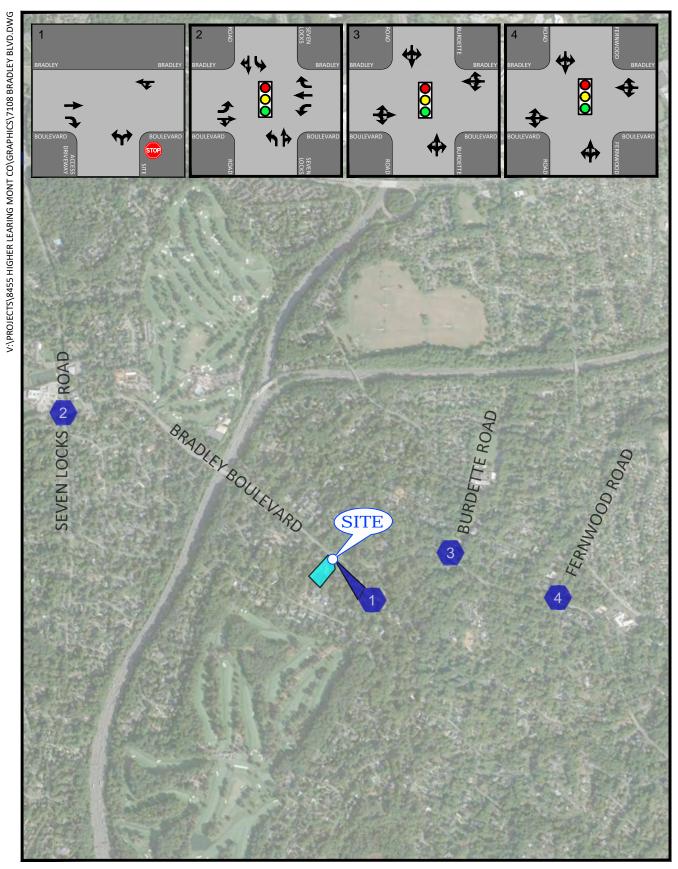
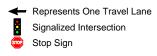


Figure 2
Existing Lane Use and Traffic Control



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NORTH
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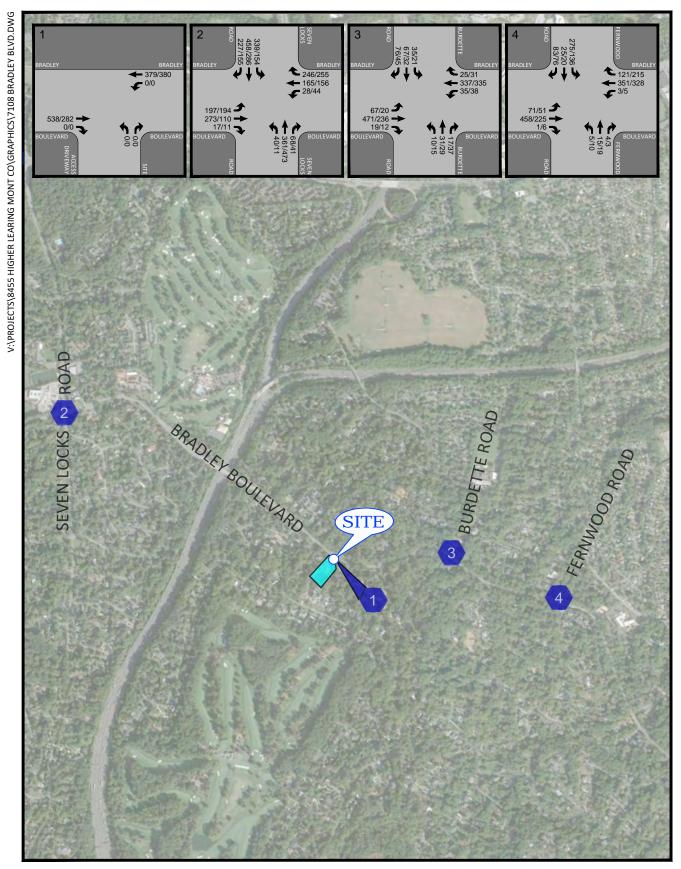
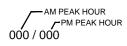


Figure 3
Adjusted Existing AM/PM Peak Hour Volumes





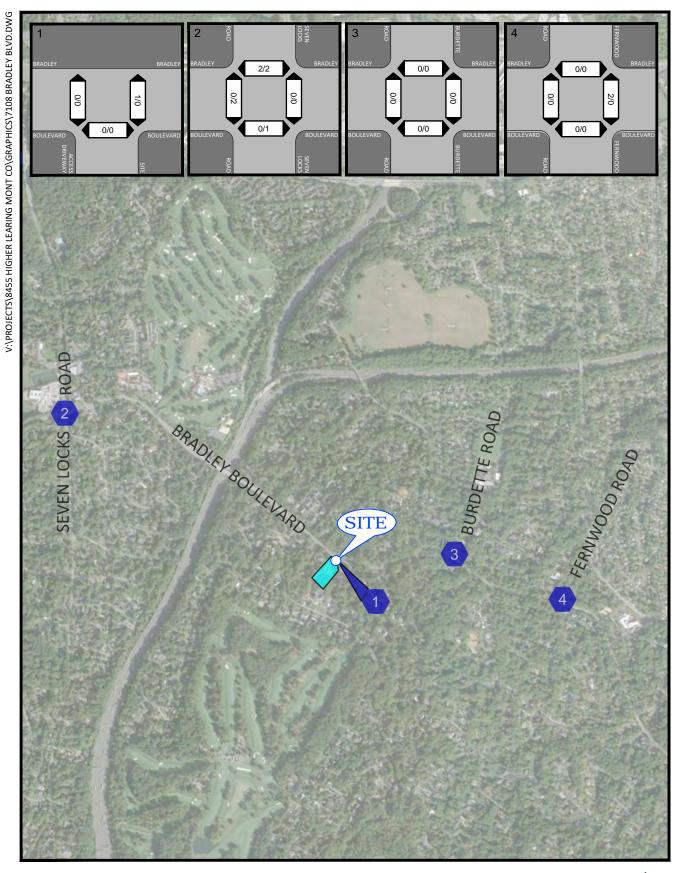


Figure 4
Existing Pedestrian Peak Hour Volume







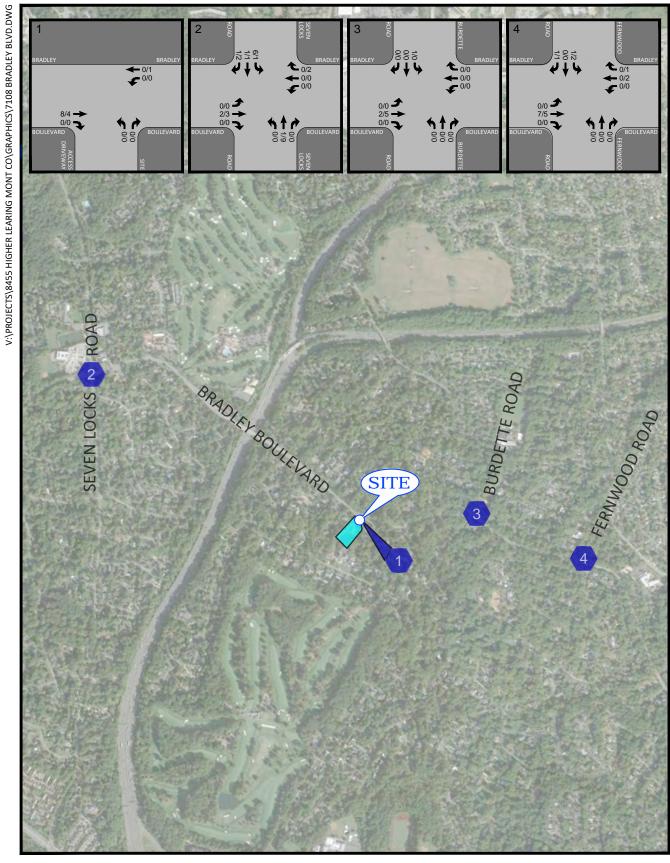
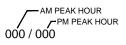


Figure 5
Existing Bicycle Peak Hour Volume



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SECTION 3 LOCAL AREA TRANSPORTATION REVIEW (LATR)

OVERVIEW

This section presents the details of the LATR. It includes: a listing of applicable congestion standards; analyses of existing critical lane volumes; a summary of site and pipeline trip generation projections; and analyses of future critical lane volumes without and with the site development. Additionally, there is a requirement for adequacy tests of different modes that include motor vehicle, pedestrian, bicycle, and bus transit.

CONGESTION STANDARD

Three of the four study intersections, including the subject site, are located within the Bethesda/Chevy Chase Policy Area (Orange) of Montgomery County. The Bradley Boulevard/Seven Locks Road intersection is located within the Potomac Policy Area (Yellow) of Montgomery County.

The adequacy for intersections located within the Bethesda/Chevy Chase Policy Area (Orange) is a maximum intersection delay of 80 seconds per vehicle. The critical lane volume congestion within the Potomac Policy Area (Yellow) is 1,450.

EXISTING CONDITIONS

Vehicular Analysis

Existing peak hour delays and/or CLVs were analyzed for each of the study intersections per the guidelines.

Calculation of the existing intersection delays and/or CLVs were based on: the existing lane use and traffic control shown on Figure 2, existing traffic signal phasing/timing obtained from Montgomery County Department of Transportation (MCDOT) (Appendix C), the adjusted existing vehicular traffic volumes shown on Figure 3, the HCM 2000 methodology for signalized and unsignalized intersections, and the LATR CLV methodology. CLV worksheets are presented for each intersection in Appendix D along with the HCM worksheets for Orange Policy Area intersections. The results of the existing analyses are summarized in Table 1.

Under the existing conditions all of the study intersections within the Bethesda/Chevy Chase Policy Area operate below the delay congestion standard of 80.0 seconds per vehicle, and the Bradley Boulevard/Seven Locks Road intersection operates below the Potomac CLV congestion standard of 1,450.



Table 1Higher Ground Education, Inc Existing Analyses Summary

			_	Existing itions
Intersection	Control	Congestion Standard	AM	PM
1. Bradley Boulevard / Site Access Driveway Bethesda/Chevy Chase (Orange) Policy Area:	Unsignalized			
Adequacy Based on HCM Delay		CLV	379	380
		HCM Delay: LOS (sec/veh)	0.0	0.0
HCM Average Veh	icle Delay Standard:	Max of 80 sec/veh	Pass	Pass
2. Bradley Boulevard / Seven Locks Road Potomac (Yellow) Policy Area: Adequacy Based	Signalized			
CLV if < 1,350	<u> </u>	CLV	1211	1117
	ongestion Standard:	Max of 1450	Pass	Pass
3. Bradley Boulevard / Burdette Road <u>Bethesda/Chevy Chase (Orange) Policy Area:</u> Adequacy Based on HCM Delay	Signalized	CLV	780	537
		HCM Delay: LOS (sec/veh)	11.1	9.4
HCM Average Veh	icle Delay Standard:	Max of 80 sec/veh	Pass	Pass
4. Bradley Boulevard / Fernwood Road Bethesda/Chevy Chase (Orange) Policy Area: Adequacy Based on HCM Delay	Signalized	CLV	934	841
·		HCM Delay: LOS (sec/veh)	22.6	13.4
HCM Average Veh	icle Delay Standard:	Max of 80 sec/veh	Pass	Pass

BACKGROUND FUTURE CONDITIONS

Pipeline Developments

The following two pipeline developments (approved, planned, or under construction and within the site vicinity) were identified during the scoping process for inclusion in this LATR study:

1. WMAL Bethesda (Plan No. 820170170)

• 309 dwelling units (159 detached, 150 townhouse) including 40 MPDUs, private roads, related residential amenities, and the associated Final Forest Conservation Plan.

2. Andrus Property (Plan No. 120180130)

• Subdivide seven (7) existing lots into a total of fifteen (15) lots, for a total of eight (8) new single-family detached lots/houses.

A graphic showing the location of each pipeline development in relation to the project is provided on Figure 6.

Pipeline Trip Generation

The trip generation for each pipeline development was obtained from their respective traffic study, traffic statement, or associated Staff reports. The pipeline developments are forecasted to add a combined 223 AM peak hour vehicle trips and 282 PM peak hour trips to the area road network at full build-out and occupancy. A portion of these trips would travel through the study intersections. A summary of the trip generation totals for each pipeline development is provided on Table 2.

Due to COVID-19, the existing school was not open, therefore the existing traffic volume data collected at the study intersections did not include the traffic volumes generated by the approved French International School. Therefore, the existing AM trips from the prior study were assumed to be on the road under the background conditions and were added to the approved development peak hour traffic volumes. These trips were assigned to the road network using distributions consistent with the M-NCPPC standards.

Pipeline Trip Assignments

The peak hour trip assignments for each of the pipeline developments were developed based on previously approved traffic studies for each of the respective developments. The combined approved development peak hour traffic volumes and existing site trip assignments traveling through the study intersections and the French International School are shown on Figure 7. The individual traffic assignments for each of the above listed approved development projects are shown in the traffic forecasting worksheets contained in Appendix E.

Background Traffic Forecasts

Background traffic forecasts represent future conditions without the Higher Ground Education, Inc user and proposed modification to hours of operation. Background traffic forecasts were



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estimated by adding the combined pipeline traffic assignments (Figure 7) to the adjusted existing peak hour traffic volumes shown on Figure 3. The resulting background future traffic forecasts are summarized on Figure 8.

Vehicular Analysis

The background peak hour intersection delays and/or CLVs without the proposed development were estimated based on: the background traffic forecasts; the existing lane use and traffic controls (Figure 2), the existing traffic signal phasing/timing, the HCM methodology, and the LATR CLV methodology. The background future intersection delays and CLVs are presented in Appendix F and summarized in Table 3.

As shown in Table 3, each of the study intersections will operate with delays or CLVs below the applicable policy area congestion threshold.



Table 2Higher Ground Education, Inc
Pipeline Development Vehicle Trip Generation

			Vehicle Trips							
			AM Peak Hour PM Peak Hour							
Development/Land Use	Amount	Unit	In	Out	Total	ln	Out	Total		
1. WMAL Property - Residential ⁽¹⁾										
Single Family Detached	170	DU	33	97	130	102	58	160		
Townhouse/ Condo Units	<u>158</u>	<u>DU</u>	<u>13</u>	<u>66</u>	<u>79</u>	<u>74</u>	<u>37</u>	<u>111</u>		
WMAL Property Subtotal			46	163	209	176	95	271		
2. Andrus Property - Residential ⁽²⁾										
Single Family Detached (9 Net New DU)	<u>9</u>	<u>DU</u>	<u>4</u>	<u>10</u>	<u>14</u>	<u>7</u>	<u>4</u>	<u>11</u>		
Andrus Property Subtotal			4	10	14	7	4	11		
Total Pipeline Development Trips			50	173	223	183	99	282		

Notes:

- 1. Trip generation based on approved WMAL Property LATR, dated October 28, 2016.
- 2. Trip generation based on Andrus Property Traffic Exemption Statement, dated January 5, 2018.

Table 3Higher Ground Education, Inc
Future Background Analyses Summary

			Background	l Conditions
Intersection	Control	Congestion Standard	AM	PM
1. Bradley Boulevard / Site Access Driveway Bethesda/Chevy Chase (Orange) Policy Area:	Unsignalized			
Adequacy Based on HCM Delay		CLV	427	383
		HCM Delay: LOS (sec/veh)	0.7	0.0
HCM Average Veh	icle Delay Standard:	Max of 80 sec/veh	Pass	Pass
2. Bradley Boulevard / Seven Locks Road Potomac (Yellow) Policy Area: Adequacy Based	Signalized on			
CLV if < 1,350		CLV	1225	1121
CLV C	ongestion Standard:	Max of 1450	Pass	Pass
3. Bradley Boulevard / Burdette Road <u>Bethesda/Chevy Chase (Orange) Policy Area:</u> Adequacy Based on HCM Delay	Signalized	CLV	804	553
		HCM Delay: LOS (sec/veh)	11.6	9.5
HCM Average Veh	icle Delay Standard:	Max of 80 sec/veh	Pass	Pass
4. Bradley Boulevard / Fernwood Road Bethesda/Chevy Chase (Orange) Policy Area: Adequacy Based on HCM Delay	Signalized	CLV	978	875
		HCM Delay: LOS (sec/veh)	24.3	14.0
HCM Average Veh	icle Delay Standard:	Max of 80 sec/veh	Pass	Pass

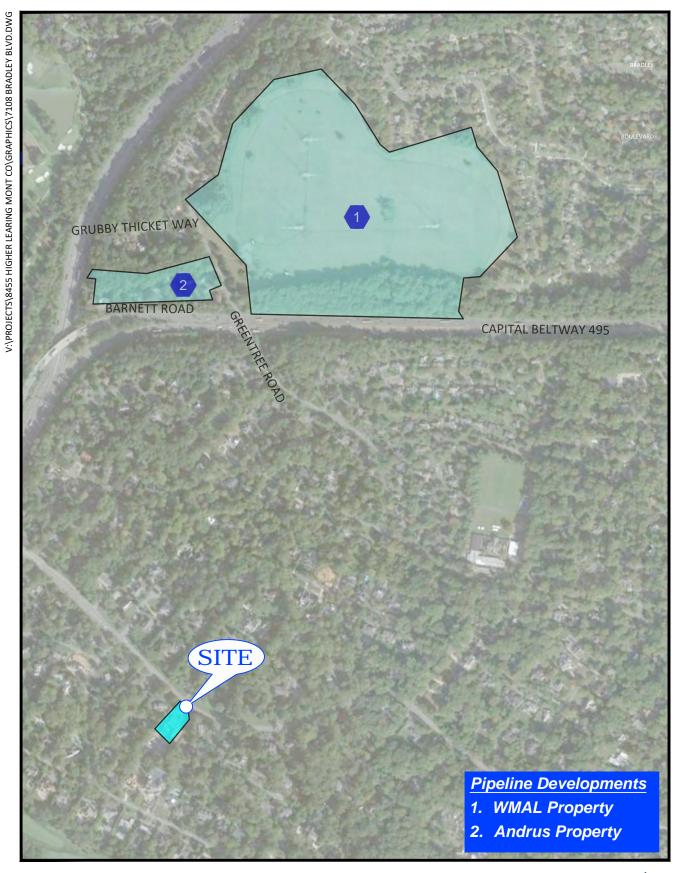


Figure 6
Pipeline Development Locations





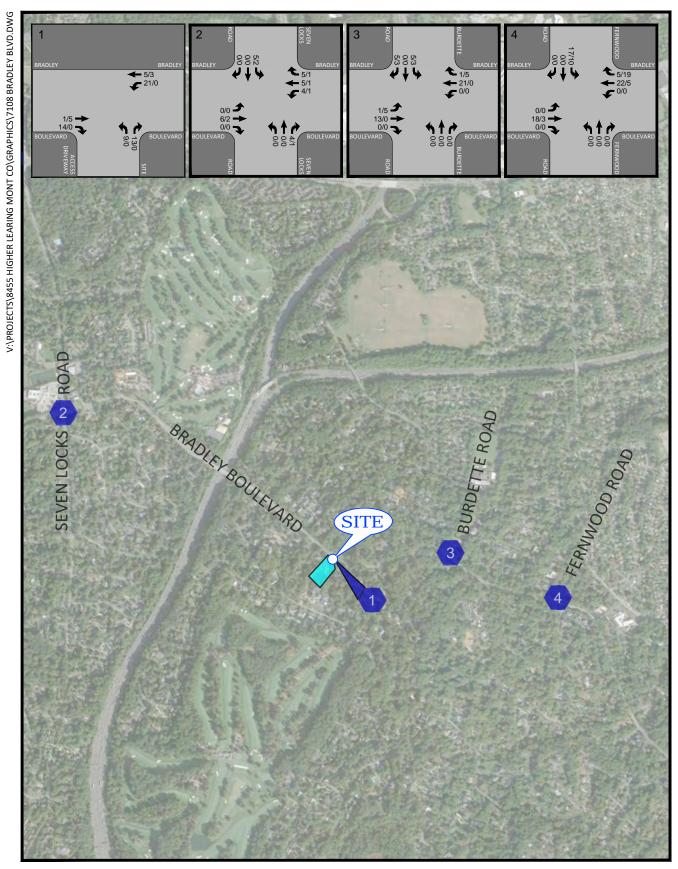


Figure 7
Total Pipieline Development Trip Assignments
(Includes Existing Site Trips (AM Peak Hour Only))

AM TRIPS
O00 / 000

AM TRIPS
PM TRIPS
NORTH
Higher Ground Education
Montgomery County, MD

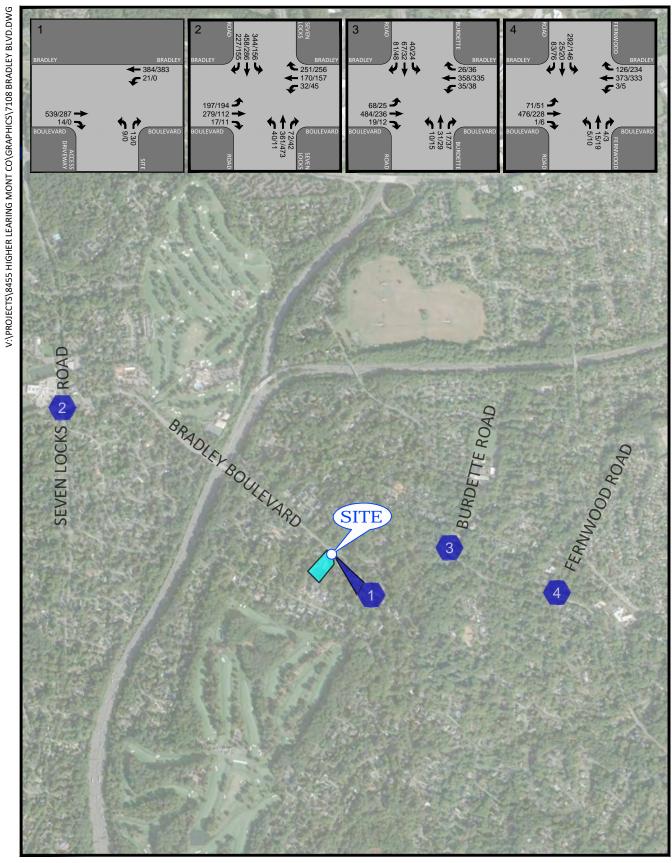


Figure 8
Background Future Traffic Forecasts





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TOTAL FUTURE CONDITIONS

The total future condition analyzes the impact of the repurposing of the existing school facilities for Higher Ground Education with the modification to hours of operation in combination with the future background conditions.

Trip Generation

Trip generation calculations for the Higher Ground Education are based on ITE trip generation rates and the Bethesda/Chevy Chase Policy Area adjustment factors provided in the LATR guidelines. The trip generation summary is shown in Table 4.

<u>Higher Ground Education</u>, a private school/child daycare facility, is expected to generate 156 new AM peak hour and 187 new PM peak hour **person trips**, 94 new AM peak hour and 113 new PM peak hour **auto-driver (vehicle) trips**, 20 new AM peak hour and 24 new PM peak hour **transit trips**, 15 new AM peak hour and 19 new PM peak hour **non-motorized (bicycle) trips**, and 35 new AM peak hour and 43 new PM peak hour **pedestrian trips**.

Table 4 shows the existing site trips (AM peak hour only) and the new vehicle and person trip generation for the proposed user with modification to hours of operation. For purposes of traffic forecasting, the existing site trips were included in background future conditions and for total future conditions, the increase (new trips due to modification of hours of operations) in site trips were applied to the road network.

Site Trip Assignments

The peak hour trip distributions for the site trips were developed consistently with the previously approved distributions for the site, as confirmed through the scoping process. (See Appendix A)

The following general distributions were utilized to assign the site trips:

To/From	(Distribution)
East via MD 191	60%
West via Md 191	40%
TOTAL	100%

The above trip distributions were used to assign the auto driver trips shown in Table 5 to the road network, as shown on Figure 9.



Total Future Forecasts

The total future traffic forecasts, shown on Figure 10, represent future conditions with Higher Ground Education, Inc. The total future traffic forecasts were developed by adding the proposed site traffic assignments shown on Figure 9 to the background future traffic forecasts shown on Figure 8.

Vehicular Analysis

The future intersection delays and/or CLVs were estimated for each of the study intersections based on: the total future traffic forecasts with the subject application; the existing lane use and traffic controls (Figure 2); existing traffic signal phasing/timing, the HCM methodology, and the LATR CLV methodology. The total future intersection capacity analyses with the project are presented in Appendix G and summarized in Table 6. Further, CLV and/or HCM worksheets are provided for each intersection in Appendix G.

As shown in Table 6, each of the study intersections would continue to operate within the applicable congestion threshold under total future conditions. In addition to this, each of the study intersections will operate within the applicable congestion standard, the proposed application passes the motor vehicle adequacy test.



Table 4Higher Ground Education, Inc
Trip Generation Summary

				ITE Trip Generat					ITE Trip Generation ¹ LATR Mode Sp								plit Adjustments ²							
				AM	Peak I	lour	PM	Peak I	lour			AM Pea	ak Hour					PM Pea	ak Hour					
Land Use	LUC	Amount	Unit	In	Out	Total	In	Out	Total	Auto Driver	Auto Passenger	Transit	Non-Motorized	Pedestrian	Total Person Trips	Auto Driver	Auto Passenger	Transit	Non-Motorized	Pedestrian	Total Person Trips			
Full-blood Hann																								
Existing Uses	3																							
Existing use- pre French s	chool			35	22	57																		
Proposed Uses																								
Private School (K-8)	534	180	Students	96	79	175	n/a	n/a	n/a	139	39	29	23	52	230									
Daycare	565	180	Students	n/a	n/a	n/a	67	75	142							113	32	24	19	43	187			
Net Trips (Proposed vs	. Existing)		61	57	118	67	75	142	94	27	20	15	35	156	113	32	24	19	43	187			

- Note:

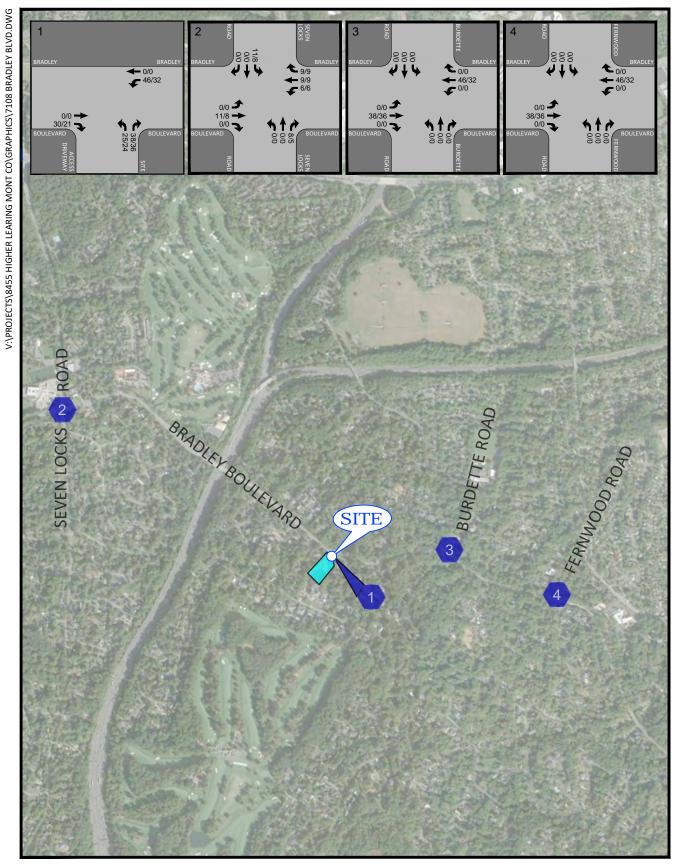
 1. Trip generation calculated using ITE <u>Trip Generation Manual</u>, 10th edition
- 2. Mode Split assumptions based on the Bethesda/Chevy Chase Policy, other land use category
- 3. Based on site driveway counts conducted by Wells & Associates on Wednesday, March, 1998.
- 4. Since the PM equation from ITE has an R 2 less than 0.75, the rate was used.

Table 5Higher Ground Education, Inc
Auto Driver Trip Generation for Proposed Site User

					Auto-D	river Tr	ip Gen	eratior	
				AM	Peak H	lour	PM	Peak F	lour
Land Use	LUC	Amount	Unit	In	Out	Total	In	Out	Total
Proposed Uses									
Private School (K-8)	534	180	Students	76	63	139	n/a	n/a	n/a
Daycare	565	180	Students	n/a	n/a	n/a	53	60	113
Auto Driver Trips					63	139	53	60	113

Table 6Higher Ground Education, Inc
Total Future Analyses Summary

				Future itions	
Intersection	Control	Congestion Standard	AM	PM	
1. Bradley Boulevard / Site Access Driveway	Unsignalized				
Bethesda/Chevy Chase (Orange) Policy Area:					
Adequacy Based on HCM Delay		CLV	493	475	
		HCM Delay: LOS (sec/veh)	1.9	1.6	
HCM Average Vehic	le Delay Standard:	Max of 80 sec/veh	Pass	Pass	
2. Bradley Boulevard / Seven Locks Road	Signalized				
Potomac (Yellow) Policy Area: Adequacy Based o	<u>n</u>				
CLV if < 1,350		CLV	1241	1143	
CLV Cor	gestion Standard:	Max of 1450	Pass	Pass	
3. Bradley Boulevard / Burdette Road	Signalized				
Bethesda/Chevy Chase (Orange) Policy Area:		CLV	829	585	
Adequacy Based on HCM Delay					
		HCM Delay: LOS (sec/veh)	11.6	9.1	
HCM Average Vehic	le Delay Standard:	Max of 80 sec/veh	Pass	Pass	
4. Bradley Boulevard / Fernwood Road	Signalized				
Bethesda/Chevy Chase (Orange) Policy Area:	-	CLV	1003	907	
Adequacy Based on HCM Delay					
		HCM Delay: LOS (sec/veh)	25.1	14.0	
HCM Average Vehic	le Delay Standard:	Max of 80 sec/veh	Pass	Pass	



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Figure 9
Site Traffic Trip Assignments

AM TRIPS
PM TRIPS

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NORTH

Higher Ground Education
Montgomery County, MD



D-:

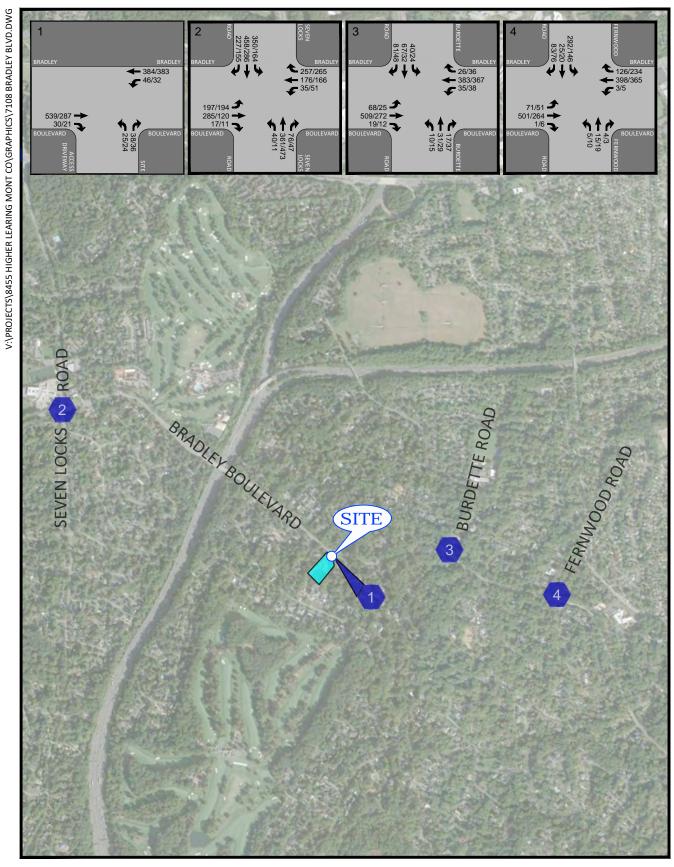


Figure 10
Total Future Peak Hour Traffic Forecasts with Development

AM TRIPS
PM TRIPS

000 / 000

NORTH

Higher Ground Education
Montgomery County, MD

LATR: July 2021

SECTION 4 BICYCLE, PEDESTRIAN, TRANSIT, and VISION ZERO

OVERVIEW

This section discusses the existing bicycle and pedestrian access and circulation in the vicinity of the site, as well as the transit and other non-automotive options in the study area; and the Bicycle Adequacy, Pedestrian System Adequacy, Bus Transit System Adequacy tests and the LATR Vision Zero Statement.

Bicycle System Adequacy

As previously discussed, "Bicycle system adequacy is defined as providing a low Level of Traffic Stress (LTS-2) for bicyclists."

The requirements for the Bicycle System Adequacy test are described in the Spring 2021 Local Area Transportation Review (LATR) Guidelines:

The applicable value for the proposed development would be 750' based on person trip generation between 100-199 and location within an Orange Policy Area. The Bicycle Adequacy Test Scoping Table is located within the 2021 LATR Guidelines on page 49.

There are no existing bicycle pathways along Bradley Boulevard, 750' from the site.

Bicycle Facilities

There are no bicycle facilities within the study area. However, the Bicycle Master Plan calls for a striped and separated bikeway on Bradley Boulevard between Seven Locks Road and Fernwood Road. The Bicycle Master Plan is shown on Figure 11.

Recommendations/Mitigations

As mentioned in the Pedestrian Level of Comfort Recommendations/Mitigation section, a shared use path, for both bicyclists and pedestrians, is recommended for the north side of Bradley Boulevard from the bus stop at Redwood Avenue to the bus stop at Oak Forest Lane, approximately 1,000' in length.

Pedestrian System Adequacy

As previously discussed, the Pedestrian System Adequacy Test consists of the following three components:

- Pedestrian Level of Comfort (PLOC)
- Street Lighting
- ADA Compliance



Pedestrian Facilities

The following provides a breakdown of sidewalk availability at the study intersections:

<u>Intersection 1: Bradley Boulevard (MD 191) / Site Access</u>

East Leg (Bradley Boulevard – MD 191)

South Side: Existing: No sidewalk provided. North Side: Existing: No sidewalk provided.

South Leg (Site Access Driveway)

East Side: Existing: No sidewalk provided. West Side: Existing: No sidewalk provided.

West Leg (Bradley Boulevard – MD 191)

North Side: Existing: No sidewalk provided. South Side: Existing: No sidewalk provided.

Intersection 2: Bradley Boulevard (MD 191) / Seven Locks Road

East Leg (Bradley Boulevard - MD 191)

North Side: Existing: Sidewalk provided. South Side: Existing: Sidewalk provided.

South Leg (Seven Locks Road)

East Side: Existing: Sidewalk provided. West Side: Existing: Sidewalk provided.

West Leg (Bradley Boulevard – MD 191)

North Side: Existing: Sidewalk provided. South Side: Existing: Sidewalk provided.

North Leg (Seven Locks Road)

East Side: Existing: Sidewalk provided. West Side: Existing: Sidewalk provided.

Intersection 3: Bradley Boulevard (MD 191) / Burdette Road

East Leg (Bradley Boulevard - MD 191)

North Side: Existing: No sidewalk provided. South Side: Existing: No sidewalk provided.

South Leg (Burdette Road)

East Side: Existing: No Sidewalk provided. West Side: Existing: No sidewalk provided.

West Leg (Bradley Boulevard - MD 191)

North Side: Existing: No sidewalk provided. South Side: Existing: No sidewalk provided.

North Leg (Burdette Road)

East Side: Existing: No sidewalk provided. West Side: Existing: No sidewalk provided.

Intersection 4: Bradley Boulevard (MD 191) / Fernwood Road

East Leg (Bradley Boulevard - MD 191)

South Side: Existing: No sidewalk provided.
North Side: Existing: No sidewalk provided.

South Leg (Fernwood Road)

East Side: Existing: No sidewalk provided.



LATR: July 2021

West Side: Existing: No sidewalk provided.

West Leg (Bradley Boulevard - MD 191)

South Side: Existing: No sidewalk provided.
North Side: Existing: No sidewalk provided.

North Leg (Fernwood Road)

East Side: Existing: No sidewalk provided. West Side: Existing: No sidewalk provided.

Crosswalks and curb ramps are provided at one study intersection as shown on Figure 12. Pedestrian signal heads with accompanying push buttons are provided wherever marked crosswalks are provided at the signalized intersections. Marked crosswalks are provided for all four (4) legs of the Seven Locks Road intersection at Bradley Boulevard (MD 191).

The crossing times were reviewed for the "walk" and "flashing don't walk" indicators. The "don't walk" indicator illuminates when the respective green cycle changes to amber. The pedestrian then has the amber and all-red phase of the cycle to complete the crossing, approximately an additional five (5) to seven (7) seconds.

Table 7 presents a summary of the crossing distances for each leg of the Bradley Boulevard / Seven Locks Road intersection, as well as the required crossing times and the provisions of crossing time for those legs based on the signal timing plans obtained from MCDOT. As shown, there is adequate "Walk" time plus "Flashing Don't Walk" time available for pedestrians to cross within the crosswalks on each street.

When considering only the "Flashing Don't Walk" time, there is adequate time to cross within crosswalks on each street as well.

Pedestrian Level of Comfort (PLOC)

The requirements for the PLOC portion of the Pedestrian Adequacy Test are described in the Spring 2021 Local Area Transportation Review (LATR) Guidelines:

The applicable value for the proposed development would be 750' in all directions based on person trip generation between 100-199 and location within an Orange Policy Area. The Pedestrian Adequacy Test Scoping Table is located within the 2021 LATR Guidelines on page 46.

Existing PLOC for the subject site area was not available on Montgomery County's website viewer: https://mcatlas.org/pedplan/ Therefore, field observations were made to identify the PLOC within the PLOC study boundary.

Figure 15 shows the existing PLOC along with the applicable 750' from the site boundary. The following applicable deficiencies (beyond somewhat or very comfortable scores) have been identified within the 750' walkshed of the proposed development boundary:



Roadway:

Bradley Boulevard: Much of the section between Redwood Avenue and Oak Forest Lane and beyond does not provide a pedestrian pathway on either side of the roadway and is rated as "Undesirable".

As shown on Figure 15, Bradley Boulevard is a non-urban roadway without pedestrian sidewalk or pathway on majority of the road, and has a posted speed limit of 35 mph. Per the Montgomery Planning Pedestrian Level of Comfort Methodology, Version 1.2 (Section V - Pathway Evaluation Table), Bradley Boulevard has a score of 4, which is considered undesirable.

Recommendations/Mitigations

The Pedestrian Level of Comfort is below the county standards; therefore, mitigation is required. However, there is little to no Rights of Way (ROW) on the south side of Bradley Boulevard. Therefore, a shared use path, for both bicyclists and pedestrians, is recommended for the north side of Bradley Boulevard from the bus stop at Redwood Avenue to the bus stop at Oak Forest Lane, approximately 1,000' in length.

Streetlight Inventory

Streetlights are located within the study area as depicted on Figure 14. The inventory was prepared via field observations. A field survey during evening hours showed that all light fixtures were operational.

Street Lighting

The requirements for the Street Lighting portion of the Pedestrian Adequacy Test are described in the Spring 2021 Local Area Transportation Review (LATR) Guidelines:

The applicable value for the proposed development would be 750' in all directions from the property for approximately based on person trip generation between 100-199 and location within an Orange Policy Area.

Recommendations/Mitigations

A field verified inventory of streetlights along Bradley Boulevard, within the 750' study area boundary, is provided on Figure 14, and photos of each streetlight are provided in Appendix B. The streetlight functionality was field verified during evening hours and all lights are functioning. Therefore, no mitigation is required.



ADA Compliance

The requirements for the ADA Compliance portion of the Pedestrian Adequacy Test are described in the 2021 Local Area Transportation Review (LATR) Guidelines:

The applicable value for the proposed development would be one-half of 750' (375') based on person trip generation between 100-199 and location within an Orange Policy Area.

There are no ADA ramps or sidewalks located within the ADA study area along Bradley Boulevard.

Recommendations/Mitigations

No mitigations are required.

Bus Transit System Adequacy

The site is directly served by the following bus service east and west of the site (Bus stop locations shown on Figure 13):

RideOn 36 (Information regarding RideOn Route 36 is provided in Appendix H.)

- RideOn 36 Bus Stop (WB and EB) along Bradley Boulevard at Oak Forest Lane, providing service to and from the development, RideOn 36 Bus Stop (WB and EB) along Bradley Avenue at Redwood Avenue.
- o Bethesda Metrorail Station and River Road.

Bus shelters are not provided in the vicinity of the subject property.

The requirements for the Bus Transit Adequacy test are described in the Spring 2021 Local Area Transportation Review (LATR) Guidelines:

The applicable requirement for the proposed development would be two (2) shelters within 1,000 feet of the site based on person trip generation between 100-199 and location within an Orange Policy Area. The Bus Transit Adequacy Test Scoping Table is located within the 2021 LATR Guidelines on page 52.

The existing bus transit stops within the study area are shown on Figure 13. None of the existing bus stops within the study area currently include bus shelters. Photos of each of the existing bus transit stops within the study area are provided in Appendix B.

Recommendations/Mitigations

The project does not pass the bus transit system adequacy test. Therefore, it is recommended that two (2) bus shelters should be constructed within 1000' of the site property.



Vision Zero Statement

The LATR Vision Zero Statement requirement consists of the following:

"All LATR studies for a site that will generate 50 or more peak-hour person trips must develop a Vision Zero Statement. This statement must assess and propose solutions to high injury network and safety issues, review traffic speeds, and describe in detail how safe site access will be provided. With concurrence of the responsible agency, projects must implement or contribute to the implementation of safety countermeasures. The County Council may adopt predictive safety analysis as part of this statement, when available."

1. "Review High Injury Network segments: Document any segments on the High Injury Network (HIN) that are within a certain distance of the site frontage, as specified in Table 1."

The subject study area does not include any segments identified as High Injury Network segments by Montgomery County.

2. "Assess proximate safety issues: Review the crash history for all segments and crossings within a certain distance of the site frontage, as specified in Table 1."

One crash was identified during the scoping process for inclusion in the study. A crash report (MCP2942000P), which was within 750' of the site, was identified by M-NCPPC Staff. The incident occurred on June 26, 2015, at the Bradley Boulevard/Oak Forest Lane intersection and consisted of a rear-end collision between a bicyclist and a driver. No severe injuries were reported.

3. "Review traffic speeds: Conduct speed studies within a certain distance from the site frontage, specified in Table 1. Locations and timing for the speed study will be determined by Planning staff in collaboration with MCDOT staff and will prioritize filling in gaps in the inventory of speed studies. Relevant speed studies that have been completed within the past three years may be used to fulfill this requirement if gaps do not remain in the inventory of speed studies."

Per Table 1 from the LATR Vision Zero Statement Guidelines, up to six (6) speed studies could be required for the subject application. During the scoping process, four (4) speed study locations were identified as follows:

1. Bradley Blvd: I-495

Redwood Ave

2. Bradley Blvd: Burdette Rd

Fernwood Road
3. Burdette Rd: Greentree Rd

Bradley Blvd
4. Burdette Rd: Hillmead Road

Bradley Blvd

The studies began on May 18, 2021, at 12:00 AM and concluded on May 20, 2021, lasting a total of 48 hours. Traffic statistics were recorded in 15-minute time periods. For each study location, two mechanical counters were used for redundancy, and the average speeds were



calculated between them. As shown on Table 8, the 85th percentile speed for vehicles driving along Bradley Boulevard was in excess of the 35-mph posted speed limit.

4. "Describe site access: Address the safety issues identified in steps 1 through 3 and describe how site circulation promotes safety, outlining how safe access will be provided to the site. Note if the applicant is contributing a fee in lieu of constructing a countermeasure. Reference the Vision Zero Community Toolkit (forthcoming) or national best practices and research in outlining the appropriate treatments to address identified safety issues."

No changes to the existing site access are proposed with the subject modification. As shown on the site plan (Figure 1), significant on-site storage is available for stacking within the existing drive aisles.



Table 7Higher Ground Education, Inc
Pedestrian Crossing Times

Pedestrian			Time		Net				Adequate tin	•
Crossing	Leg of Intersection	Distance of Pedestrian Crossing* (feet)		Clearance Time Reduction (seconds)	Crossing Time Required ² (seconds)	Walk (seconds)	Flashing- don't walk (seconds)	Total Walk Time ³ (seconds)	With respect to Walk + Flash Don't Walk	With respect to Flash Don't Walk
1: Bradley Boulevard (MD 191) / Se	even Locks Roa	ad								
Across Bradley Boulevard (MD 191)	East	66	19.0	6	13.0	7	15	22	Yes	Yes
Across Bradley Boulevard (MD 191)	West	52	15.0	6	9.0	7	15	22	Yes	Yes
Across Seven Locks Road	South	53	15.0	6	9.0	7	14	21	Yes	Yes
Across Seven Locks Road	North	63	18.0	6	12.0	7	14	21	Yes	Yes

Notes:

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⁽¹⁾ Based on existing single timings obtained from Montgomery County DOT or field observations.

⁽²⁾ Net Crossing Time Required = Time needed for 3.5 feet/sec - Clearance Time Reduction

⁽³⁾ Total Walk Time = Walk + Flash Do Not Walk Time

^{*} Distance from curb to far edge of traveled lane

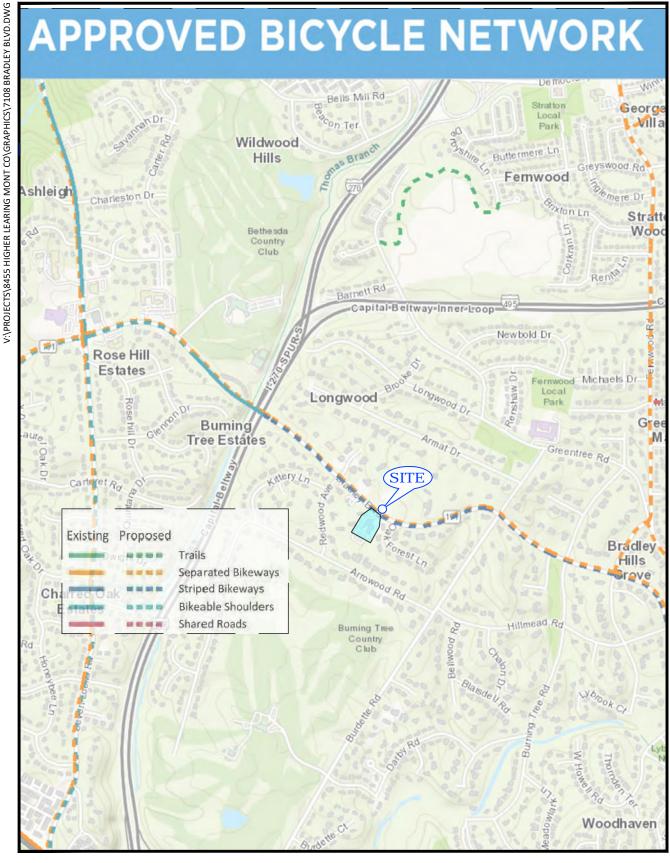


Figure 11 Bicycle Master Plan



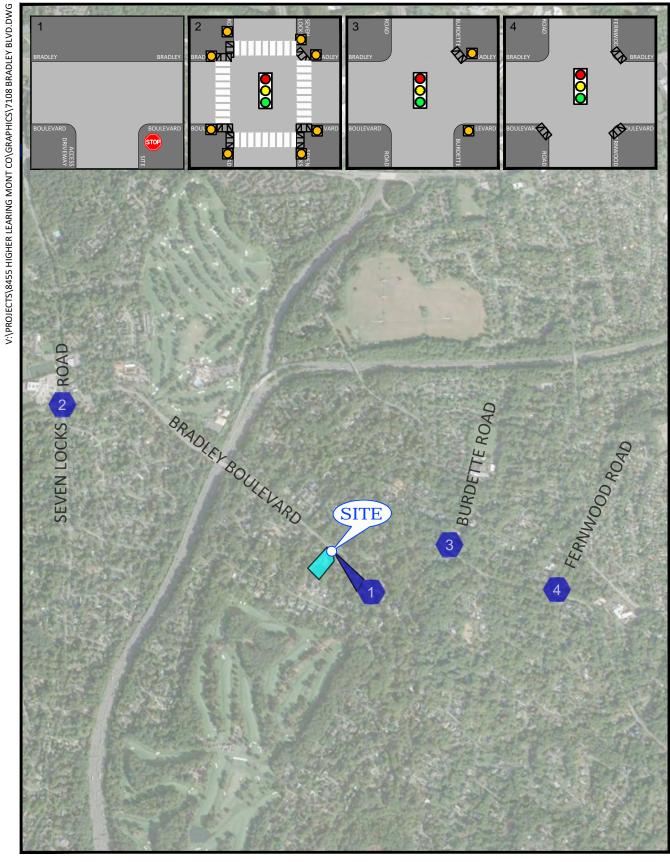
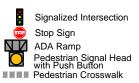


Figure 12
Pedestrian Facilities





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Figure 13 RideOn Bus Stop Locations





Montgomery County, MD

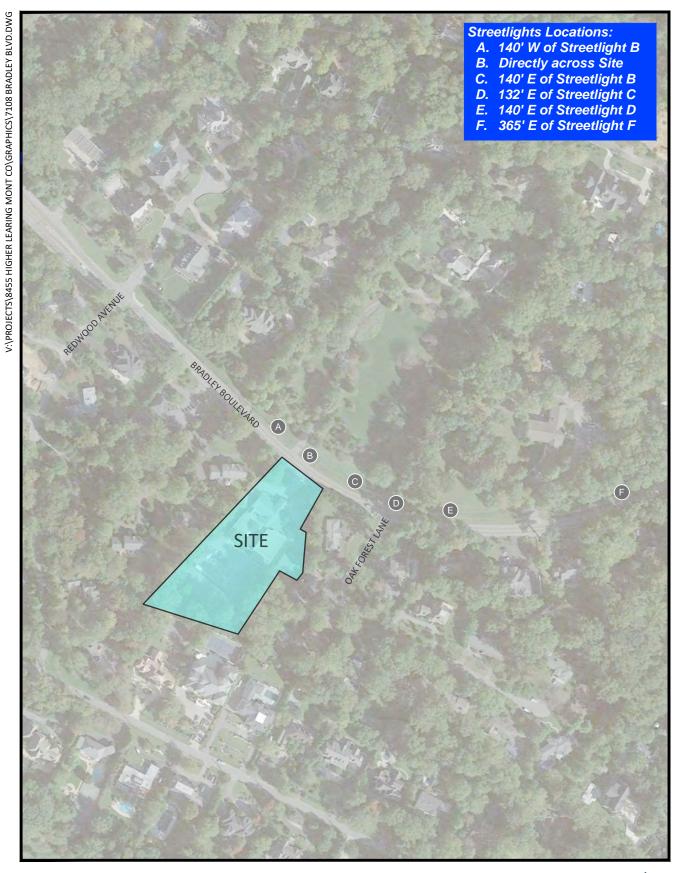


Figure 14
Streetlight Locations





Higher Ground Education Montgomery County, MD

Figure 15
Pedestrian Level of Comfort Study



NORTH Higher Ground Education Montgomery County, MD



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Table 8Higher Ground Education, Inc Summary of Speed Study

Counter Label	Road	Direction	Road Segm	ent Between	10-Mile Per Hour Pace (mph)	Avg. Speed (mph)	85th Percentile (mph)	48-hr Volume (veh)	ADT	Speed Limit (mph)
358	Burdette Road	NB	Hillmeade Road	Bradley Boulevard	15-24 mph	22	26.35	1422	711	35
1064		NB	Tillineade Road	bradicy bodicvard	15-24 mph	20	24.39	1451	726	35
	LOCATIO	N "A-NB"		Average		21	25.37	1436.5	718	
641	Burdette Road	SB	Hillmeade Road	Bradley Boulevard	20-29 mph	24	28.59	1580	790	35
1264		SB	Tillineade Road	bradiey bodievard	20-29 mph	24	28.75	1574	787	35
	LOCATIO	N "A-SB"		Average		24	28.67	1577	789	
366	Burdette Road	NB	Greentree Road	Bradley Boulevard	20-29 mph	21	27.61	1598	799	35
876	Burdette Road	NB	Greentree Road	Bradley Boulevard	15-24 mph	22	27.29	1597	799	35
	LOCATIO	N "B-NB"		Average		22	27.45	1597.5	799	
1134	Dundatta Daad	SB	Creenture Deed	Duadlay Daylayand	15-24 mph	22	27.31	1711	856	35
1212	Burdette Road	SB	Greentree Road	Bradley Boulevard	15-24 mph	21	25.65	1145	573	35
	LOCATIO	N "B-SB"		Average		22	26.48	1428	714	
588	Due die e De eleve ed	EB	Donalatta Danal	Farmer and Danad	30-39 mph	34	41.63	7767	3884	35
1211	Bradley Boulevard	EB	Burdette Road	Fernwood Road	30-39 mph	36	43.14	7787	3894	35
	LOCATIO	N "C-EB"		Average		35	42.39	7777	3889	Exceeds
589	Decillo De la col	WB	B. old H. Brook	E	30-39 mph	35	40.94	7415	3708	35
875	Bradley Boulevard	WB	Burdette Road	Fernwood Road	30-39 mph	33	38.98	7493	3747	35
	LOCATIO	N "C-WB"		Average		34	39.96	7454	3727	Exceeds
1202	Due die e De eleve ed	WB	Daltara	Doduce ed Access	40-49 mph	45	52.25	7148	3574	35
2569	Bradley Boulevard	WB	Beltway	Redwood Avenue	35-44 mph	41	47.86	7144	3572	35
	LOCATIO	N "D-WB"		Average		43	50.06	7146	3573	Exceeds
1263	Due die e De de e e e	EB	Daltaria	De dune e d Avec	35-44 mph	24	48.75	7653	3827	35
6064	Bradley Boulevard	EB	Beltway	Redwood Avenue	35-44 mph	41	47.18	7513	3757	35
	LOCATIO			Average		33	47.97	7583	3792	Exceeds

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Section 5 CONCLUSIONS

The Special Exception Modification proposes to change the current hours of operation from 9:00 AM to 3:20 PM (existing) to 7:00 AM to 7:00 PM (proposed) with a capacity of up to 180 children/students the subject property is located in the Bethesda/Chevy Chase Policy Area (Orange) of Montgomery County.

- The proposed modification is expected to result in 156 new AM person trips and 187 new PM person trips resulting in approximately 94 new AM auto driver (vehicle) trips and 113 new PM auto driver (vehicle) trips.
- The AM and PM peak vehicle delays and/or CLVs at the study intersections operate well within the Bethesda/Chevy Chase Policy Area congestion standard of 80.0 seconds/vehicle and Potomac Policy Area CLV congestion, under existing, background and total future conditions. Therefore, no mitigation is required by this application to pass the motor vehicle adequacy test.
- The site does not pass the Bicycle System Adequacy Test within the applicable study area.
 A shared use path for both bicyclists and pedestrians is recommended for the north side of Bradley Boulevard from Redwood Avenue to Oak Forest Lane, between the two proposed bus shelters.
- There are no existing ADA ramps or sidewalks within the required study area. Therefore, no ADA corrective measures are needed.
- Using the Montgomery's Pedestrian Level of Comfort Methodology (Version 1.2), the
 pathways of the study area for this development were considered undesirable due to the
 lack of pedestrian facilities. As mentioned in the Bicycle System Adequacy Test section, a
 shared use path is recommended for the north side of Bradley Boulevard from Redwood
 Avenue to Oak Forest Lane, between the two proposed bus shelters.
- The site is served by RideOn bus service (Routes 36) with two (2) bus stops (without shelters) within 1000' of the site. The Bus Transit Adequacy Test requires the installation of two bus shelters.



Higher Ground Education, Inc July 2021

APPENDIX A SCOPING LETTER

(WITH MAP ADJUSTMENTS TO SCOPE DUE TO STUDENT POPULATION DECREASE)



Local Area Transportation Review

TRANSPORTATION IMPACT STUDY SCOPE OF WORK AGREEMENT

Updated Winter 2021

Scoping Approval - Prior to initiating a Local Area Transportation Review study or supplemental traffic study, scoping *must be approved* by relevant agencies, including the Planning Department, the Montgomery County Department of Transportation, and the State Highway Administration (where relevant). It is the responsibility of the Applicant to obtain approval, which is demonstrated below via signature or electronic signature of the relevant agency representatives. Generally, the Applicant should anticipate a turnaround time of ten (10) business days for form review. Substantially large projects may require additional time and/or may warrant a scoping meeting.

Torin review. Substantially la	Tge projects may require a	duitioi	iai tiirie a	ilu/oi iliay wa		ping meeting.
Montgomery County Plan	ning Department					
Name (print):	Signati	ıre: _			[Date:
Montgomery County Department	artment of Transportati	on				
Name (print):	Signati	ıre:			[Oate:
State Highway Administra	ation (where relevant)					
Name (print):		ıre: _			[Date:
Applicant Contact Info	rmation					
Transportation Consultant (company, contact name, email, and phone number)	William L. Zeid, PE - Wells Christine G. Bairan, EIT - Wel			~		` /
Name of Applicant / Developer	Higher Learning Educati	on				
Project Information	Incl	ude Ta	ables/Gra	aphics, As Ne	eeded	
Project Name (include plan no. if known)	7108 Bradley Blvd - Hi	gher L	earning E	Education		
Project Location (include address if known)	7108 Bradley Blvd					
Policy Area(s) (subdivision staging policy map)	Bethesda/Chevy Chase			Plan(s) / Plan Area(s)		
Application Type(s)	□Preliminary Plan	□ S	ite Plan	Sketch/Cone Preliminary	• •	□ Amendment
,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,			ocal Map	☐ APF at E	Building	□ Other:

A-1 D-55

Project Description & Previous Approvals (proposed land uses, zoning, no. of units, square footage, construction phasing, prior approvals and proposals, existing uses, site operations, year built, status of Adequate Public Facilities [APF], other relevant info)	The Conditional Use for 7108 Bradley Boulevard (the "Property") was originally approved in 1998 and thereafter modified in 2000. Our client is proposing to modify the long-standing Special Exception. The use will remain unchanged – the Property will continue to be used as a Private Education Institution. However, our client is seeking to modify the hours of operation, from the current hours of 9:00 AM to 3:30 PM to 7:00 AM to 7:00 PM with no more than 200 children/students. The existing occupied building and parking lot will not be enlarged.
1.Site Access (proposed access location(s), existing/adjacent/opposite curb cuts, interparcel connections, access configurations and restrictions, internal circulation, private roads, parking/loading areas, other relevant info)	The site is accessed via an existing site driveway located along the south side of Bradley Boulevard (MD 191). No changes to the existing site access are proposed.
2.Transportation Analysis Requirement	☐ Transportation Study Exemption Statement Generates 50 or more total weekday peak hour person trips (vehicular, transit, bicycle, and/or pedestrian) with no reductions other than a credit for existing developments over 12 years old, AND is outside of the White Flint and White Oak Policy Areas. Fill out remainder of this form and include in transportation impact study appendix. ☐ Transportation Study Exemption Statement Generates 49 or fewer total weekday peak hour person trips (vehicular, transit, bicycle, and/or pedestrian) with no reductions other than a credit for existing developments over 12 years old, OR within White Flint and White Oak Policy Areas.
3.Project-based Transportation Demand Management Plan Required (see Chapter 42, Articles I and II)	☐ Yes (In Transportation Management District [TMD]) ☐ Amend Existing TMAg
4.Established Transportation Management District (TMD)?	☑ No ☐ Yes TMD Name:
Transportation Impact S	
5.Study Years / Phases 6 Study Periods	Existing Year: 2021 Phases / Build-out Year(s): 2022
D STUDY PARIODS	M AN IN PN NIGHON STURGAV SUNGAV Other

7.Study Intersections (For projects generating 50 or more person trips, list all signalized & significant unsignalized intersections, and site driveways traffic counts must be collected within 12-months of completed and accepted application)	For the purpose of a subject site should be taken in the should be t	determining the num also include nearby ui	ber of tie nbuilt pro than a cro ool) oad	rrent LATR Guidelines): 1-Tier iers of study intersections, trip calculation for the roperties in common ownership. No trip reductions redit for existing developments over 12 years old. 7) 8) 9) 10) 11) attach more rows if necessary					
8.Trip Generation (clearly cite sources and methodology including use of average rates vs. equation; include trip generation for existing site, current approvals, proposed uses, and net changes)	Total Person Trips AM: 177 PM: 207 * Only required if to of all vehicle, transi	it, and non-motorized	AM PM n trips and	fi: 22 fi: 26 e 50 or more	Walking Trips* (non-motorized + transit) AM: 40 PM: 46 e in either the AM or Fulivalent of total personal pe	Bicycling Trips* (non-motorized) AM: 18 PM: 20 PM peak hour. Sum on trips. Use table at			
9.Trip Reductions (include justification and supporting documentation for internal capture, pass-by, diverted, Transportation Demand Management)	No reductions proposed at this time.								
10.Trip Distribution % (include a map of the proposed project in addition to a list or table)	Same as prior approved study - 60 percent to from east on Bradley Boulevard 40 percent to from west on Bradley Boulevard- At Seven locks road/Bradley Blvd 15 percent to north 15 percent to west and 10 percent to south					ey Blvd 15			
11.Pipeline Developments to be considered as background traffic (include name, plan #, land uses, and sizes for approved but unbuilt developments or concurrently pending applications; info can be obtained from the M-NCPPC Pipeline website: - website is updated quarterly)	WMAL Bethesda #820170170 Andrus Property #120180130								
12.Pipeline Transportation Projects to be considered as background condition (fully funded for construction in County Capital Improvement Program, State Consolidated Transportation Program, developer projects, etc. within the next 6 years)	No projects hav	e been identified.							

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13. Vision Zero Statement	 Trigger: All LATR studies for a site that generates 50 or more weekday peak hour person trips must develop a Vision Zero Statement. Requirements: The Vision Zero Statement consists of four components: 1. Review High Injury Network segments: Document any segments on the High Injury Network (HIN) that are within a certain distance of the site frontage. 2. Assess proximate safety issues: Review the crash history for all segments and crossings within a certain distance of the site frontage. 3. Review traffic speeds: Conduct speed studies within a certain distance from the site frontage. 4. Describe site access: Address the safety issues identified in steps 1 through 3 and describe how site circulation promotes safety, outlining how safe access will be provided to the site. The applicant should refer to the LATR Guidelines to determine the applicable scoping distance pertaining to steps 1 through 3 and requirements pertaining to steps 1 through 4.

Preliminary Mitigatio	n Analysis	*Refer to the LATR Guidelines for details on how to mitigate
14.Vehicular Analysis	 ✓ Vehicular Analysis Anticipated (Vehicular mitigation to be determined after study) 	• TEST: The motor vehicle adequacy test will not be applied in "Red" policy areas and these areas will not be subject to LATR motor vehicle mitigation requirements. If the plan generates 50 or more weekday peak hour person trips, HCM Analysis is required to be provided for all intersections analyzed in studies for: 1) "Orange" policy areas, and 2) intersections with a CLV of more than 1,350 in "Yellow & Green" policy areas. 3) With the exception of intersections located within "Red" policy areas, CLV analysis required for all intersections
		regardless of policy area. CLV assessment and signal timing worksheets are to be included in the study appendix. • MITIGATION: The applicant must mitigate its impact on vehicle delay or down to the applicable policy area standard, whichever is less.
15.Pedestrian Analysis	☑ Pedestrian Mitigation Anticipated	 TEST: If the plan generates 50 or more weekday peak hour person trips, mitigation of surrounding pedestrian conditions is required. MITIGATION: Mitigation consists of three components: (1) Pedestrian Level of Comfort (PLOC). Pedestrian system adequacy is defined by providing a "Somewhat Comfortable" or "Very Comfortable PLOC score on streets and intersections for roads classified as Primary Residential or higher within a certain walkshed from the site. (2) Street Lighting. The applicant must evaluate existing street lighting based on MCDOT standards along roadways and paths from the development within a certain walkshed from the site frontage. Where standards are not met, the
		applicant must upgrade the street lighting to meet the applicable standard. (3) ADA Compliance. The applicant must fix ADA noncompliance issues within a certain walkshed from the site frontage equivalent to half the walkshed specified in

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	the	e required scoping distance.
	det	e applicant should refer to the <i>LATR Guidelines</i> to termine the applicable scoping walkshed distance quirement for each component described above.
16.Bicycle Analysis	person require Bicycle Mitigation Anticipated Stress within the proprovide equival cyclists The ap	ATION: Required to ensure a low Level of Traffic (LTS-2) on all existing transportation rights-of-way a certain distance of the site frontage; Alternatively, oject may provide a master planned improvement that es an elent improvement in the level of traffic stress for s within a certain distance of the site frontage.
17.Bus Transit Analysis	TEST: trips in require exemp Mitigation Anticipated MITIGA standa accessi numbe The ap	If the plan generates 50 or more peak hour person mitigation of surrounding transit conditions is ed. Projects located within "Green" policy areas are of from the bus transit adequacy test. ATION: Required to ensure that there are bus shelters ed with realtime traveler information displays and other and amenities, along with a safe, efficient, and eible path between the site and a bus stop, at a certain er of bus stops within a certain distance from the site. Explicant should refer to the LATR Guidelines to mine the applicable number of bus stop and scoping irement.
Additional Analysis or Software Required	⊠Queuing Analysis □ Signal Warrant Analysis	Accident Analysis
M-NCPPC Clarifications		Additional Assumptions & Special Circumstances for Discussion
requirements of the LATR Gu If physical improvements transportation impact study w to right-of-way and utility rele If the development propo transportation impact stu Applicant will work with M-NC accurately reflect the new pro A receipt from MCDOT sho study review fee has been pa the time the development app Minimum of seven paper of	sal significantly changes after this dy scope has been agreed to, the CPPC staff to amend the scope to oposal. wing that the transportation impact hid will be provided to M-NCPPC DARC at plication is submitted. copies (more if near the County line or of PDF copies of the transportation	High Injury Network (HIN) Map Speed Studies - Maximum of 6 speed studies within 900' of the site

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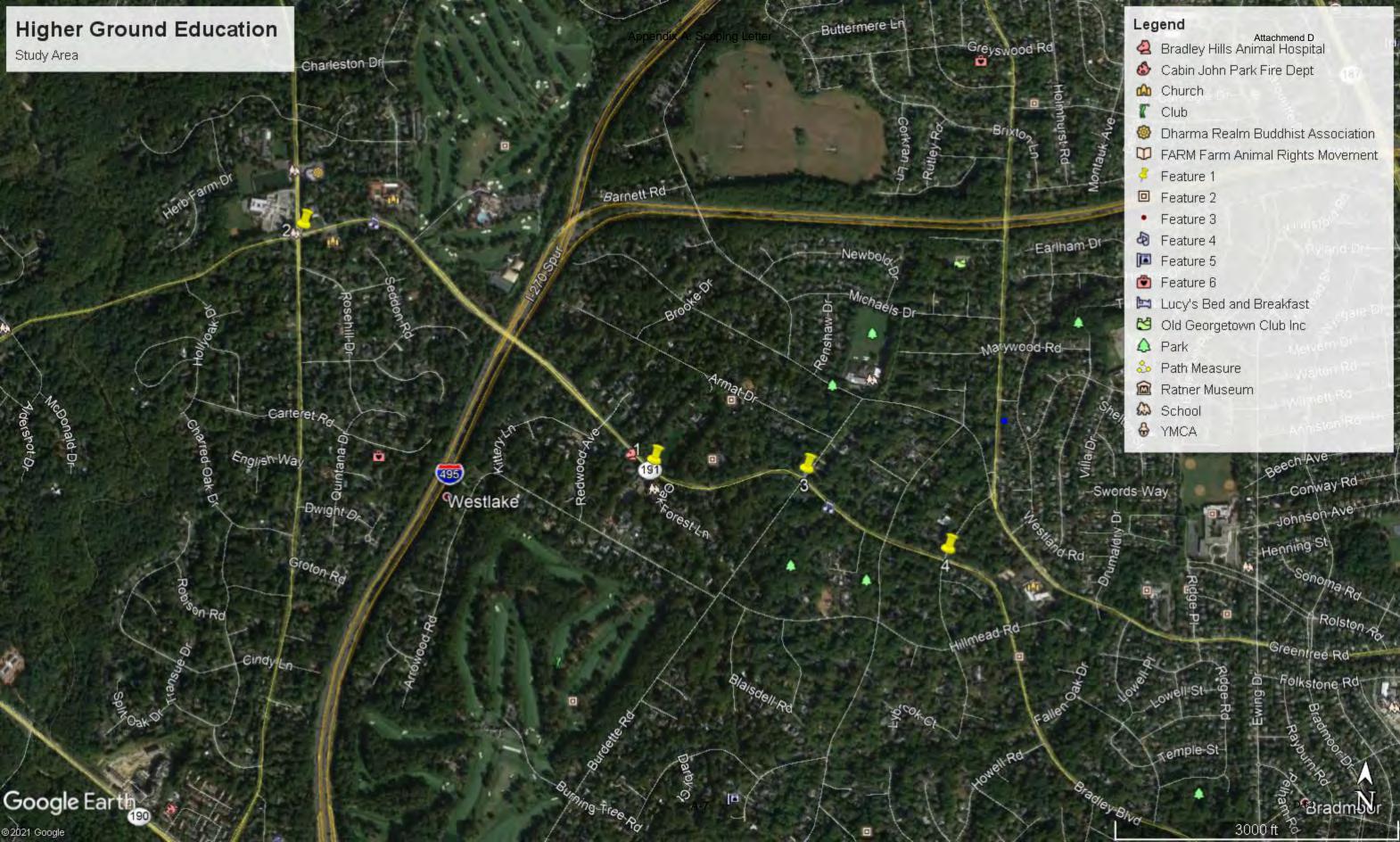
Site Trip Generation

					ITE	Trip Ge	enerati	on ¹					L	LATR M	lode Spli	t Adjust	ments	2			
				AM	Peak I	lour	PM	Peak H	Hour			AM Pea	ak Hour					PM Pea	k Hour		
Land Use	LUC	Amount	Unit	In	Out	Total	In	Out	Total	Auto Driver	Auto Passenger	Transit	Non-Motorized	Pedestrian	Total Person Trips	Auto Driver	Auto Passenger	Transit	Non-Motorized	Pedestrian	Total Person Trips
Existing Uses Existing use- pre French so	chool ³			35	22	57															
Proposed Uses Private School (K-8) Daycare	534 565	200 200	Students Students	106 n/a	86 n/a	192 n/a	n/a 74	n/a 84	n/a 158												
Net Trips (F	Proposed vs	s. Existing)		71	64	135	74	84	158	107	30	22	18	40	177	125	35	26	20	46	207

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Note:

- 1. Trip generation calculated using ITE Trip Generation Manual, 10th edition
- 2. Mode Split assumptions based on the Bethesda/Chevy Chase Policy, other land use category
- 3. Based on site driveway counts conducted by Wells & Associates on Wednesday, March, 1998.
- 4. Since the PM equation from ITE has an R² less than 0.75, the rate was used.



Higher Ground Education, Inc July 2021

APPENDIX A STUDY AREAS OF MULTI-MODAL ADEQUACY TESTS BASED ON 200 STUDENTS

Project: Higher Learning (7108 Bradley Blvd)

Policy Area: Bethesda/Chevy Chase (Orange)

Person Trip Generation: 200-349 Net New Person Trips based on PM peak hour

Total Person Trips: AM Peak Hour – 177 / PM Peak Hour – 207

TL2.3 Pedestrian System Adequacy

1. Pedestrian Level of Comfort: From Table T4 – 900'

2. Street Lighting: From Table T4 = 900'3. ADA Compliance: ½ of Table T4 – 450'

Table T4. Pedestrian Adequacy Test Scoping

Peak-Hour Person Trips Generated	Red and Orange Policy Area Walkshed*	Yellow and Green Policy Area Walkshed*
50 - 99	400'	250'
100 - 199	750'	400'
200 - 349	900'	500'
350 or more	1,000'	600'

^{*} The maximum required length of sidewalk and streetlighting improvements beyond the frontage is 4 times the appropriate value in this column. The maximum span required for ADA improvements beyond the frontage is equal to the appropriate value in this column.

TL2.4 Bicycle System Adequacy

1. LTS-2: From Table T5 - 900'

Table T5. Bicycle Adequacy Test Scoping

Peak-Hour Person Trips Generated	Red and Orange Policy Areas	Yellow and Green Policy Areas		
50 – 99	400'	250'		
100 – 199	750'	400° 500°		
200 - 349	900'			
350 or more	1,000'	600'		

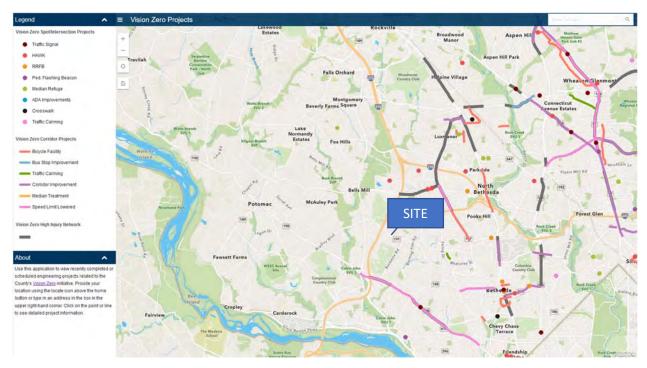
TL2.5 Bus Transit System Adequacy

1. Bus Shelters: From Table T6 – 3 shelters within 1,300'

Table T6. Transit Adequacy Test Scoping

Peak-Hour Person Trips Generated	Red and Orange Policy Areas	Yellow Policy Areas
50 – 99	2 shelters within 500'	1 shelters within 500'
100 – 199	2 shelters within 1,000'	2 shelters within 1,000'
200 - 349	3 shelters within 1,300'	2 shelters within 1,300'
350 or more	4 shelters within 1,500'	3 shelters within 1,500'

A-8 D-63

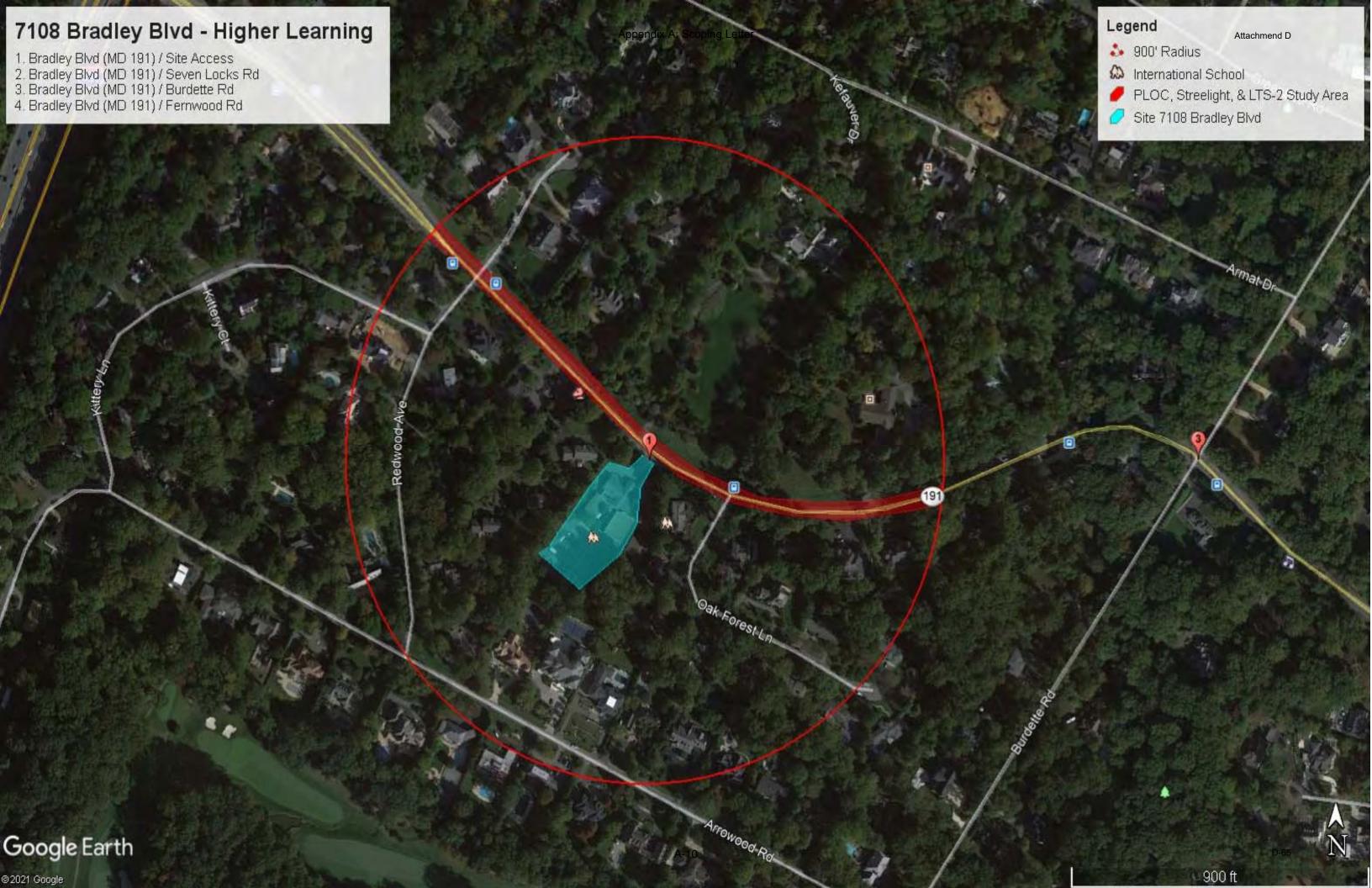


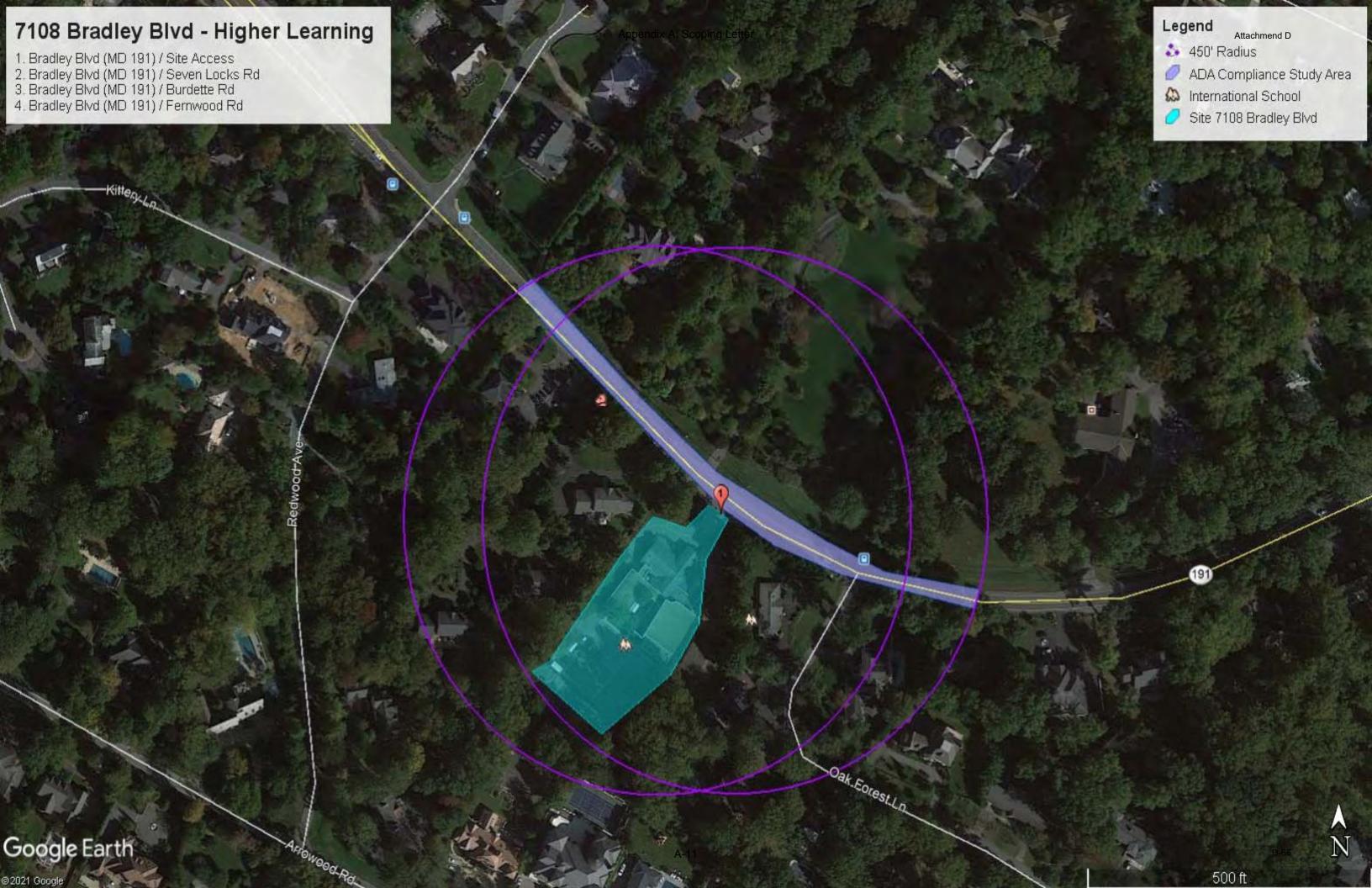
Vision Zero: Site is not located within a High Injury Network.

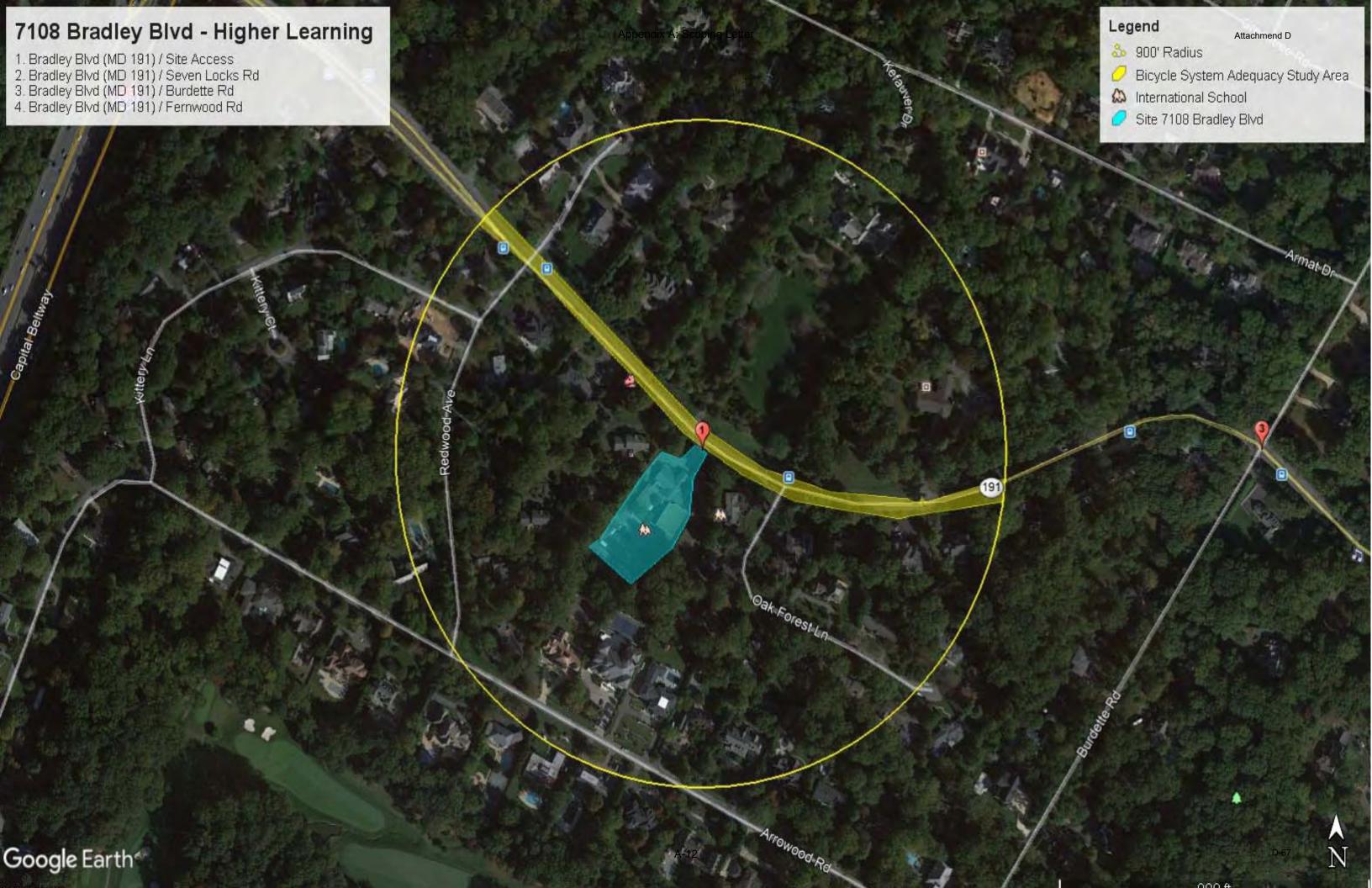
Crash Data: Per Table below: Within 900' of the site – One crash identified by M-NCPPC Staff.

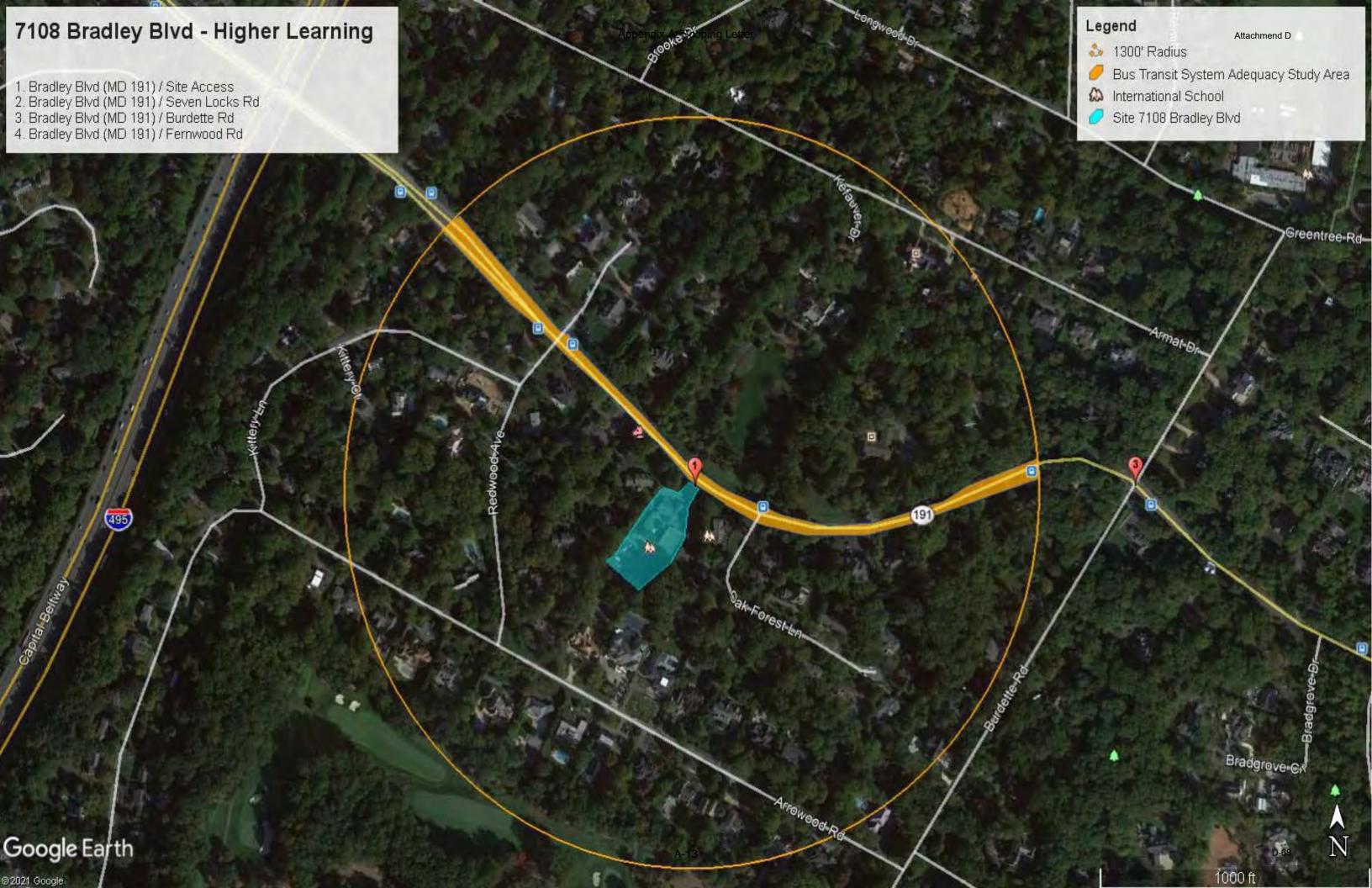
- 4 Speed Studies identified by M-NCPPC Staff listed below. See attached.
- 1. Bradley Boulevard between the Beltway and Redwood Ave
- 2. Bradley Boulevard between Burdette Rd and Fernwood Rd
- 3. Burdette Road between Greentree Rd and Bradley Blvd
- 4. Burdette Road between Hillmead Rd and Bradley Blvd

Peak-Hour Person Trips Generated	Distance from Site Frontage		Max. Number of Speed Studies	
	Red and Orange Policy Areas	Yellow and Green Policy Areas	Red and Orange Policy Areas	Yellow and Green Policy Areas
50-99	400′	250′	2	1
100-199	750′	400′	4	2
200-349	900′	500′	6	3
350 or more	1,000′	600′	8	4













Higher Ground Education, Inc July 2021

APPENDIX A STUDY AREAS OF MULTI-MODAL ADEQUACY TESTS BASED ON 180 STUDENTS

Project: Higher Learning (7108 Bradley Blvd)

Policy Area: Bethesda/Chevy Chase (Orange)

Person Trip Generation: 100-199 Net New Person Trips based on PM peak hour

Total Person Trips: AM Peak Hour – 156 / PM Peak Hour – 187

TL2.3 Pedestrian System Adequacy

1. Pedestrian Level of Comfort: From Table T4 – 750'

2. Street Lighting: From Table T4 = 750'3. ADA Compliance: ½ of Table T4 – 375'

Table T4. Pedestrian Adequacy Test Scoping

Peak-Hour Person Trips	Red and Orange Policy	Yellow and Green Policy
Generated	Area Walkshed*	Area Walkshed*
50 – 99	400'	250'
100 – 199	750'	400'
200 – 349	900'	500'
350 or more	1,000'	600'

^{*} The maximum required length of sidewalk and streetlighting improvements beyond the frontage is 4 times the appropriate value in this column. The maximum span required for ADA improvements beyond the frontage is equal to the appropriate value in this column.

TL2.4 Bicycle System Adequacy

1. LTS-2: From Table T5 - 750'

Table T5. Bicycle Adequacy Test Scoping

Table 13. Dicycle Adequacy Test Scoping					
Peak-Hour Person Trips	Red and Orange	Yellow and Green			
Generated	Policy Areas	Policy Areas			
50 – 99	400'	250'			
100 – 199	750'	400'			
200 - 349	900'	500'			
350 or more	1,000'	600'			

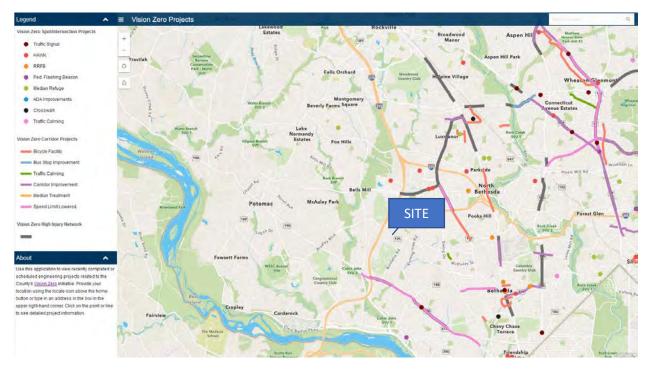
TL2.5 Bus Transit System Adequacy

1. Bus Shelters: From Table T6 – 2 shelters within 1,000'

Table T6. Transit Adequacy Test Scoping

Peak-Hour Person Trips	Red and Orange	Yellow	
Generated	Policy Areas	Policy Areas	
50 – 99	2 shelters within 500'	1 shelters within 500'	
100 – 199	2 shelters within 1,000'	2 shelters within 1,000'	
200 – 349	3 shelters within 1,300'	2 shelters within 1,300'	
350 or more	4 shelters within 1,500'	3 shelters within 1,500'	

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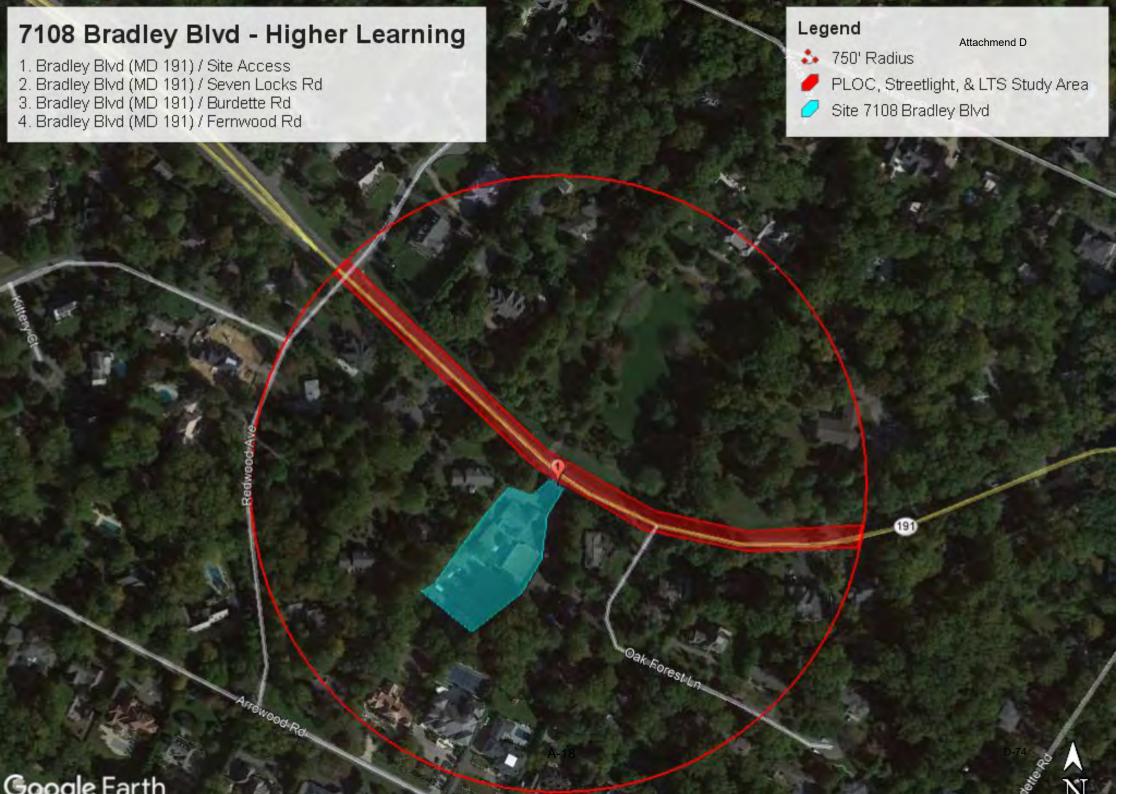
Vision Zero: Site is not located within a High Injury Network.

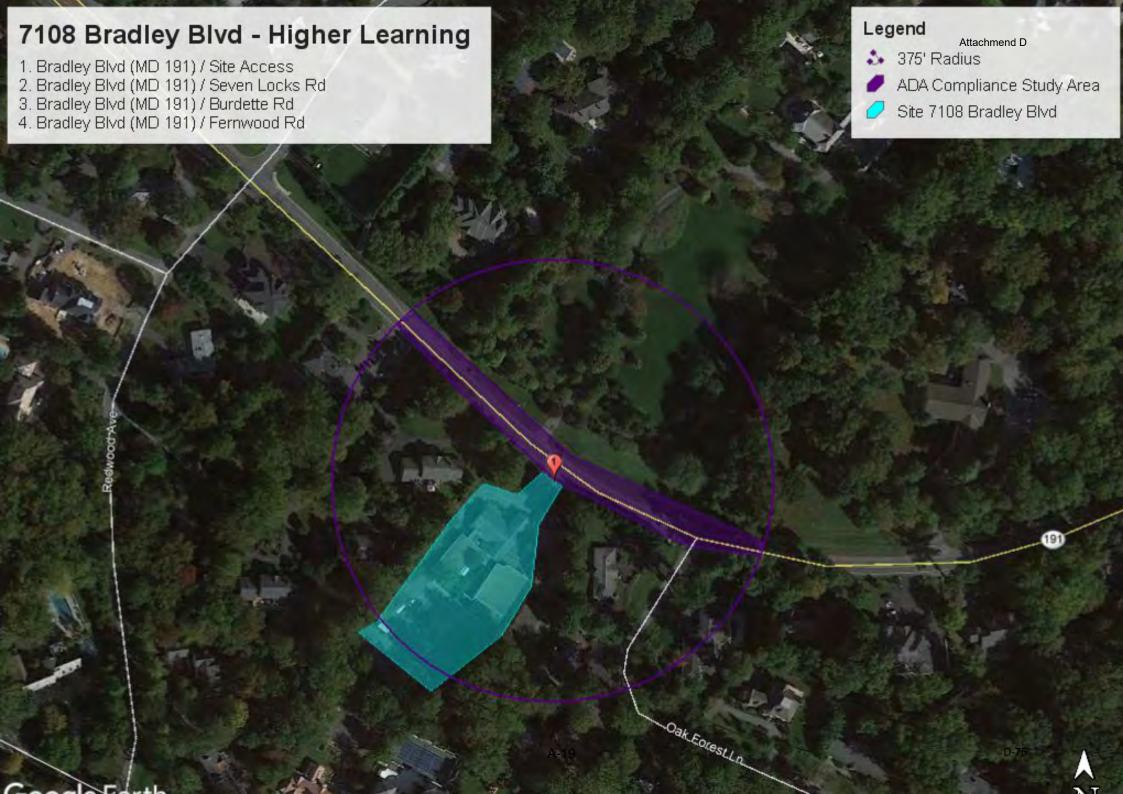
Crash Data: Per Table below: Within 750' of the site – One crash identified by M-NCPPC Staff.

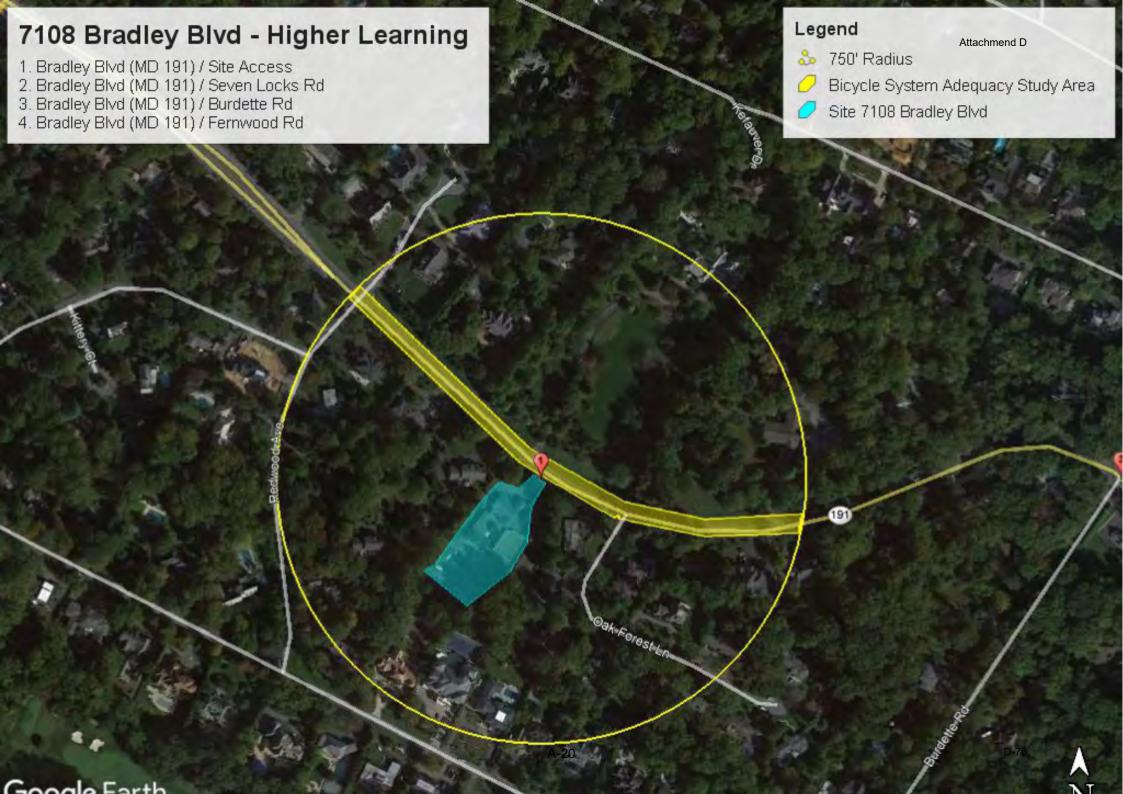
- 4 Speed Studies identified by M-NCPPC Staff listed below. See attached.
- 1. Bradley Boulevard between the Beltway and Redwood Ave
- 2. Bradley Boulevard between Burdette Rd and Fernwood Rd
- 3. Burdette Road between Greentree Rd and Bradley Blvd
- 4. Burdette Road between Hillmead Rd and Bradley Blvd

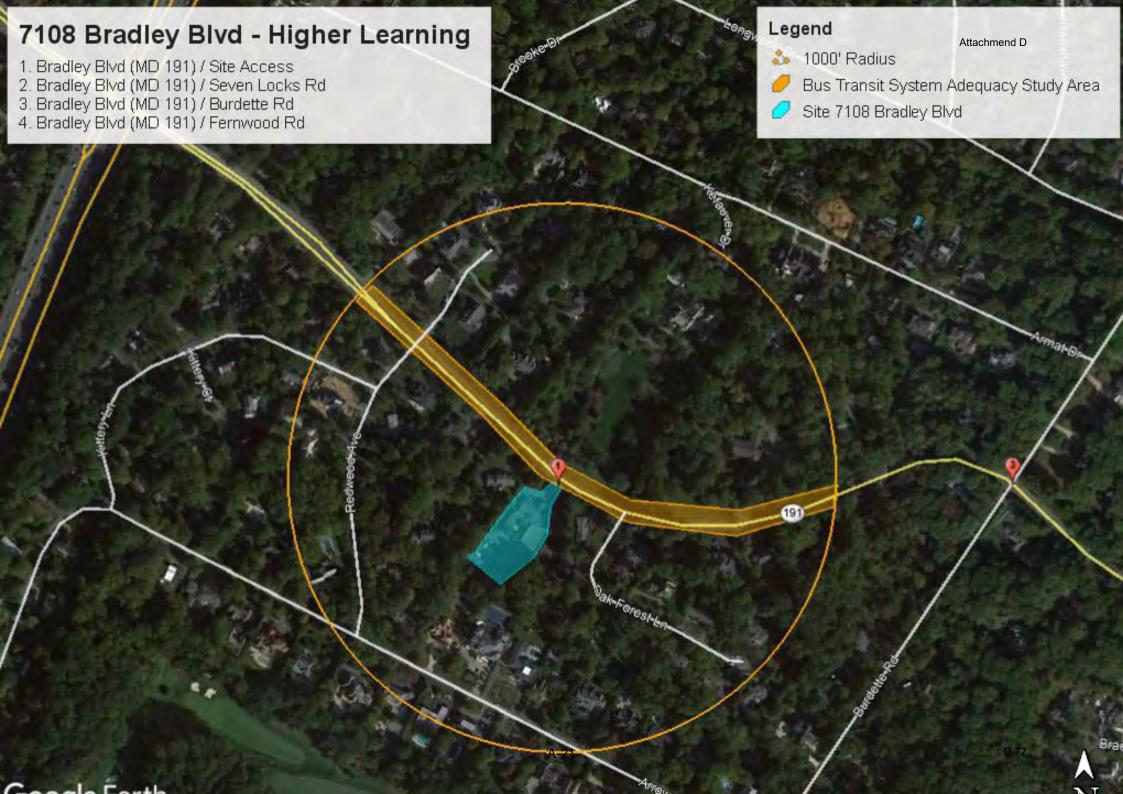
Peak-Hour Person Trips	Distance from	Site Frontage	Max. Number o	f Speed Studies
Generated	Red and Orange Policy Areas	Yellow and Green Policy Areas	Red and Orange Policy Areas	Yellow and Green Policy Areas
50-99	400′	250′	2	1
100-199	750′	400′	4	2
200-349	900′	500′	6	3
350 or more	1,000′	600′	8	4

A-17 D-73









MCPB Item No. 10 Date: 09/24/20

Item Name: Briefing on Updated Traffic Count Data Collection Policy During COVID-19 Pandemic

Eb

Eric Graye, Planning Supervisor, Countywide Planning and Policy Division, eric.graye@montgomeryplanning.org, 301-495-4632

Jason Sartori, Chief, Countywide Planning and Policy Division, jason.sartori@montgomeryplanning.org, 301-495-2172

Completed: 09/17/20

Description

On March 14, 2020, the Montgomery County Planning Department initiated an interim policy to not accept transportation impact studies using traffic counts taken during COVID-19 pandemic. The policy was updated on May 7, 2020 to reflect changing conditions and to allow the use of existing counts taken prior to the pandemic, with potential modifications.

The COVID-19 pandemic continues to add uncertainty into traffic analysis in Montgomery County. The impact on business, public offices, schools, other facilities, transit, coupled with the expansion of telework, has substantially reduced the total amount of motor vehicle and other traffic in the county. Nevertheless, Planning staff believes current conditions represent a new normal and that as the pandemic conditions subside, it will take quite a while for traffic volumes to slowly return to prepandemic levels.

The Planning Board will be briefed on Planning Department plans to move forward with the collection of traffic counts and the acceptance of transportation impact studies during the pandemic.

Attachment: Planning Staff Memorandum – Traffic Counts During COVID-19 Pandemic – Policy Update

A-22 D-78

ATTACHMENT

September 17, 2020

MEMORANDUM

TO: Gwen Wright, Planning Director

VIA: Jason Sartori, Chief, Countywide Planning & Policy Division

FROM: Eric Graye, Planning Supervisor, Countywide Planning & Policy Division

SUBJECT: Traffic Counts During COVID-19 Pandemic -Policy Update

Background

On March 13, 2020, as the State of Maryland and Montgomery County entered a state of emergency and increased restrictions due to the COVID-19 pandemic, Montgomery Planning informed transportation consultants that the Department would not accept any transportation impact studies (TIS) based on traffic counts conducted on March 14, or later. On May 7, 2020, the Department initiated an updated temporary policy pertaining to traffic counts to provide opportunities for transportation studies to move forward using historical traffic counts. This temporary policy also provided time to observe how the effect of increased teleworking in the region is manifested in observed traffic conditions. While it is unclear when (or if) traffic will return to pre-March 2020 levels, it appears likely that traffic growth will increase gradually over time.

Recent data obtained from the Maryland State Highway Administration show that current statewide daily traffic volumes have leveled off at approximately 83% of traffic compared with 2019 levels. These volumes, while lower than the pre-March 2020 volumes, reflect the existing new normal daily traffic conditions. While there is limited comparable data available to assess peak-hour traffic conditions, it appears reasonable that existing peak-hour volumes would reflect new normal traffic conditions as well.

Recommendation

The current interim traffic count data collection policy will end on September 30, 2020. A new interim traffic count data collection and TIS acceptance policy with the following options shall apply as of October 1, 2020:

• Option A. New Counts.

 As long as the county remains in Phase 2 (or advances to Phase 3) of the COVID-19 recovery plan, a TIS may use traffic counts collected on or after October 1, 2020 with the application of an adjustment factor to account for the Montgomery County

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Public School building closure and remote learning status. Should county public schools re-open for full in-person instruction, new traffic counts may be used without an adjustment factor and in accordance with the 2017 Local Area Transportation Review Guidelines.

- The applicable adjustment factor will be determined by Montgomery Planning staff in advance of October 1.
- This traffic count option will immediately cease to be available for transportation impact studies should the county revert to Phase 1 of the COVID-19 recovery plan.
- Should the county fully reopen, new traffic counts may be used without an adjustment factor and in accordance with the 2017 Local Area Transportation Review Guidelines, effective three months after the declaration of the county's open status.
- **Option B. Historical Counts.** Transportation Impact Studies based on traffic counts collected before March 14, will be accepted as follows:
 - Existing counts collected within a year of the application's 65-day deadline can be used in a TIS without adjustment.
 - Existing counts collected between one and three years prior to the application's 65-day deadline may only be used if modified by a growth factor. The growth factor must be developed based on the past ten-year historical traffic volume data for the nearest SHA roadway. This growth rate should be approved by the Planning Department in coordination with the Development Review Committee prior to acceptance of the TIS.
 - Traffic counts collected more than three years earlier than the application's 65-day deadline will not be accepted and may not be used to assess the finding of Adequate Public Facilities.

Combinations of the two options will be accepted as each is allowed, but under no circumstance will traffic counts collected between March 14 and September 30, 2020 be accepted.

This policy will be re-evaluated during the summer of 2021, or earlier if deemed necessary.

A-24 D-80

From: Graye, Eric
To: Sartori, Jason

Cc: Anspacher, David; Sanders, Carrie; Hisel-McCoy, Elza; Broullire, Bridget; Stern, Tanya; Provost, Russell; Wright,

Gwen; Kronenberg, Robert; Pereira, Sandra; Dickel, Stephanie; Butler, Patrick; Reed, Patrick; Freer, Walker; Thompson, Iftin; Mencarini, Katherine; Van Alstyne, Chris; Campbell, Lauren; Gonzalez, Elwyn; Andy Smith; Anne M. (Nancy) Randall; Carl Wilson; Chris L. Kabatt; Christopher Turnbull; David Nelson; David Samba; Ed Papazian; Erwin N. Andres; Glenn Cook; Joe Caloggero; Joe Mehra; Katie Wagner; Michael J. Workosky; Mike Lenhart; Mike Nalepa;

Paquilla Jones; Shahriar Etemadi; Wes Guckert; William L. Zeid; mlewisdegrace@symmetradesign.com;

ndriban@lenharttraffic.com

Subject: RE: Montgomery Planning Policy on Traffic Counts during COVID-19 pandemic

Date: Tuesday, December 15, 2020 10:52:05 AM

Attachments: <u>image002.png</u>

image004.png image006.png image010.png image010.png image021.png image023.png image025.png image027.png image027.png

Good Morning:

The purpose of this note is to inform you of a **minor** clarification regarding the Planning Department's current policy pertaining to the collection of new traffic counts during the COVID-19 pandemic. This clarification is reflected as revisions to the language provided in the September 30th e-mail announcement of the policy. (Please see the revisions below in red text and strikethrough.)

As a reminder, it should also be noted that new traffic counts will not be accepted in the event the county returns to Phase 1 of the COVID-19 recovery plan. We hope we don't find ourselves in this situation!

You are welcome to contact me (or Jason Sartori) should you have any questions concerning this matter. Thanks!

Regards,

Eric



Eric S. Graye, AICP, PTP

Planning Supervisor

Montgomery County Planning Department 2425 Reedie Drive, Wheaton, MD 20902 eric.graye@montgomeryplanning.org p:301.495.4632 c:202.236.4483





WE'VE MOVED! -

THE NEW PARK AND PLANNING HEADQUARTERS IS NOW LOCATED AT 2425 REEDIE DRIVE, WHEATON, MD 20902

A-25 D-81

From: Sartori, Jason < Jason. Sartori@montgomeryplanning.org>

Sent: Wednesday, September 30, 2020 3:44 PM

To: Sartori, Jason < Jason. Sartori@montgomeryplanning.org>

Cc: Graye, Eric <eric.graye@montgomeryplanning.org>; Anspacher, David

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<carrie.sanders@montgomeryplanning.org>; Hisel-McCoy, Elza <elza.hisel-</pre>

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Stern, Tanya <tanya.stern@montgomeryplanning.org>; Provost, Russell

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Kronenberg, Robert <robert.kronenberg@montgomeryplanning.org>; Pereira, Sandra

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Freer, Walker < Walker. Freer@montgomeryplanning.org >; Thompson, Iftin

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<nalepa@streettrafficstudies.com>; Paquilla Jones <pjones@wellsandassociates.com>; Shahriar Etemadi

<etemadi.sts@gmail.com>; Wes Guckert <wguckert@trafficgroup.com>; William L. Zeid
<wlzeid@mjwells.com>

Subject: Montgomery Planning Policy on Traffic Counts during COVID-19 pandemic

Good afternoon,

Montgomery Planning would like to share an update on our policy pertaining to traffic counts used for Transportation Impact Studies. Planning staff briefed the Planning Board on the updated policy last Thursday. The details of the policy are outlined in this <u>September 17 memo</u>. Below is a summary of the options available regarding traffic counts, including additional information not in the aforementioned memo (namely, the adjustment factor to be used for new counts and its specific application):

1. New Counts.

- a. We will begin accepting new counts taken on or after October 1, as long as the county remains in Phase 2 or 3 of the COVID-19 recovery plan.
- b. The new counts must be adjusted by a factor to account for Montgomery County Public Schools not being in session in-person. The calculated adjustment factor of **1.07** must be applied as follows:
 - i. AM peak period Apply the 1.07 adjustment factor to all AM peak hour period traffic counts.

- ii. PM peak period Apply the 1.07 adjustment factor to any PM peak hour period traffic counts captured before 4:30 pm. No adjustment factor is required for counts captured after 4:30 pm.
- c. Adjustment factor requirements will be lifted under certain conditions pertaining to the county and/or county schools reopening.

2. Historical Counts.

- a. We will continue to accept historical counts generally taken within the last three years.
- b. Certain other restrictions apply, and an adjustment factor is required under certain conditions.
- c. This option is a continuation of the interim policy that we have had in place since May. The details of that policy can be found in this <u>April 30 memo</u>.

For any given project, an applicant can use a combination of the above options, as applicable, to ensure all critical intersections are evaluated.

Please let us know if you have any questions.

Thank you, Jason



Jason K. Sartori

Chief, Countywide Planning & Policy Division

Montgomery County Planning Department
2425 Reedie Drive, 13th Floor | Wheaton, MD 20902
jason.sartori@montgomeryplanning.org
c: 240.877.9388 | o: 301.495.2172







WE'VE MOVED!

THE NEW PARK AND PLANNING HEADQUARTERS IS NOW LOCATED AT 2425 REEDIE DRIVE, WHEATON, MD 20902

A-27 D-83

Higher Ground Education, Inc July 2021

APPENDIX B TRAFFIC COUNTS AND FIELD DATA/INVENTORY

McLean, Virginia

Existing Traffic Count

DATE: PROJECT: 5/6/2021 SOUTHBOUND ROAD: Higher Learning W & A JOB NO.: DAY: Site Access 8455 Thursday NORTHBOUND ROAD: Bradley Boulevard Bradley Boulevard INTERSECTION: Bradley Blvd. & Site Access WEATHER: clear WESTBOUND ROAD: LOCATION: Montgomery Co.,MD COUNTED BY: Agan EASTBOUND ROAD: INPLITED BY: agan

			-					INPUTE		agan								 			<u> </u>
		0- "	ha !		Γ			Moveme	nts	K1= :41 *	ha !			F. (1			Ι				
Time		South	bound 1		,	Westl Bradley E	bound Boulevar	.d		North Site A	bound		,	Eastt Bradley E	oound Soulevar	Ч	North	East	Total	PHF	Time
Period	1	2	3		4	5	6	u	7	8	9		10	11	12		& &	& &	Total		Period
	Right	Thru	Left	Total	Right	Thru	Left	Total	Right	Thru	Left	Total	Right	Thru	Left	Total	South	West			
AM																					
6:30-6:45	0	0	0	0	0	6	0	6	0	0	0	0	0	14	0	14	0	20	20		6:30-6:45
6:45-7:00	0	0	0	0	0	9	0	9	0	0	0	0	0	21	0	21	Ö	30	30		6:45-7:00
7:00-7:15	0	0	0	0	0	29	1	30	0	0	0	0	0	26	0	26	0	56	56		7:00-7:15
7:15-7:30	0	0	0	0	0	33	0	33	0	0	0	0	0	60	0	60	0	93	93		7:15-7:30
7:30-7:45	0	0	0	0	0	58	0		0	0	0	0	0	56	0	56	0	114	114		7:30-7:45
7:45-8:00	0	0	0	0	0		0	_	0	0	0	0	0	110		110		236	236		7:45-8:00
8:00-8:15	0	0	0	0	0	79	0	_	0	0	0	0	0	158		158		237	237		8:00-8:15
8:15-8:30		0	0	0	0	78	0	_	0	0	0	0	0	139		139		217	217		8:15-8:30
8:30-8:45 8:45-9:00	0	0	0	0	0	71 60	0		0	0	0	0	0	96 105		96 105		167 165	167 165		8:30-8:45 8:45-9:00
9:00-9:15		0				59	0	59	0	0	0	0		63	0	63		122	122		9:00-9:15
9:15-9:30		0	0		0	36	0	36	0	0	0	0	0	54	0	54	0	90	90		9:15-9:30
		_																			
3 Hour																					
Totals	0	0	0	0	0	644	1	645	0	0	0	0	0	902	0	902	0	1,547	1,547		
1 Hour																					
Totals		^	_				4		۾ ا				_	404		404	_	400	400	0.50	0.00 7.00
6:30-7:30	0	0					1	78 130	0	_	0	0	0	1	0			199			6:30-7:30
6:45-7:45 7:00-8:00	0	0	0	0	0	1	1	130 247	0	0	٥	0	0	163 252	l			293 499	293 499		6:45-7:45 7:00-8:00
7:00-8:00		0			0	296	0		0	0	0	0		384		384		680	680		7:15-8:15
7:30-8:30		0	0		0	341	0	l I	0	0	0	0	0	463	0	463		804	804		7:30-8:30
7:45-8:45	0	0	0	0	•		0		0	0	0	0	0					857	857		7:45-8:45
8:00-9:00	0	0	0	0	0		0		0	0	0	0	0			498		786	786		8:00-9:00
8:15-9:15	0	0	0	0	0		0	268	0	0	0	0	0	403	0	403	0	671	671	0.77	8:15-9:15
8:30-9:30	0	0	0	0	0	226	0	226	0	0	0	0	0	318	0	318	0	544	544	0.81	8:30-9:30
AM Peak																					AM Peak
7:45-8:45	0	0	0	0	0	354	0	354	0	0	0	0	0	503	0	503	0	857	857	0.90	7:45-8:45
PM 4:00-4:15		0	0	0	0	81	1	82	0	0	0	0	0	63	0	63	_	145	145		4:00-4:15
4:15-4:30		0	0			102	0	l	0	0	2	2	2	55	0	57		159	161		4:15-4:30
4:30-4:45		0			0	102	0	l	0	0	0	0	0	59	0	59		163	163		4:30-4:45
4:45-5:00	0	0	0	0	0	90	0	90	0	0	0	0	0	67	0	67	Ö	157	157		4:45-5:00
5:00-5:15	0	0	0	0	0	81	0		1	0	0	1	0	71	0	71	1	152	153		5:00-5:15
5:15-5:30	0	0	0	0	0	107	0	107	0	0	0	0	0	66	0	66	0	173	173		5:15-5:30
5:30-5:45	0	0	0	0	0	81	0		0	0	0	0	0	81	0	81	0	162	162		5:30-5:45
5:45-6:00	0	0	0	0	0		0		1	0	0	1	0	64	0	64		175			5:45-6:00
6:00-6:15		0	0		0	89	0	ا ح	0	0	0	0	0	50 50	0	50 50		139	139		6:00-6:15
6:15-6:30 6:30-6:45		0	0		0	69 61	1	70 62	1	0	0	1	0	59 61	0	59 61	0	129 123	129 124		6:15-6:30 6:30-6:45
6:45-7:00	0	0	0		0	48	0	48	0	0	0	0	0	51	0	51		99	99		6:45-7:00
3. 10 7.00		U					U		J										33		0.107.00
3 Hour																					
Totals	0	0	0	0	0	1,024	3	1,027	3	0	2	5	2	747	0	749	5	1,776	1,781		
1 Hour																					
Totals		_							٦			_	_	2		0.15				0.00	4.00 5.00
4:00-5:00	0	0	0	0			1	378	0	0	2	2	2	244				I	626		4:00-5:00
4:15-5:15 4:30-5:30	0	0	0		0		0	I I	1	0	2	3	2 0	1				631 645	634 646		4:15-5:15 4:30-5:30
4:30-5:30 4:45-5:45	0	0	0		0		0	I I	1	0	ا ا	1	0	1				644	645		4:30-5:30
5:00-6:00	0	0	0	0					2	0	0	2	0								5:00-6:00
5:15-6:15	0	0		0	0		0		1	0	0	1	0	261	0	261	1	649	650		5:15-6:15
5:30-6:30	0	0	Ő	0	0		1	351	1	0	o o	1	Ő	254	o o	254	1	605	606		5:30-6:30
5:45-6:45	0	0	0	0	0		2		2	0	0	2	0	234	0	234	2	566	568		5:45-6:45
6:00-7:00	0	0	0	0	0	267	2	269	1	0	0	1	0	221	0	221	1	490	491	0.88	6:00-7:00
PM Peak																					PM Peak
5:00-6:00	0		0	0	0	380	0	380	2	0	0	2	0	282	0	282	2	662	664	0.04	5:00-6:00

B-1

Project Name: Higher Learning

Project Number: 8455

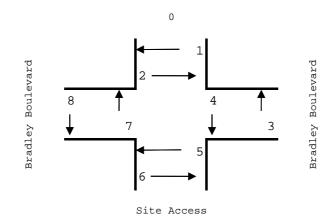
Location: Montgomery Co.,MD

Intersection: Bradley Blvd. & Site Access

Weather: clear

Date: 5/6/2021

Surveyor: Agan



Hourly Pedestrian Count

			1	2	3	4	5	6	7	8					
		From:	SE	NE	SW	SE	SW	NW	NW	NE	Total	1 & 2	3 & 4	5 & 6	7 & 8
	Period	To:	NE	SE	SE	SW	NW	SW	NE	NW					
AM PEAK															
6:30	7:30		0	1	0	0	1	0	0	0	2	1	0	1	0
6:45	7:45		0	1	0	0	1	0	0	0	2	1	0	1	0
7:00	8:00		0	1	0	0	0	0	0	0	1	1	0	0	0
7:15	8:15		0	1	0	0	0	0	0	0	1	1	0	0	0
7:30	8:30		1	0	0	0	0	0	0	0	1	1	0	0	0
7:45	8:45		1	0	0	0	0	0	0	0	1	1	0	0	0
8:00	9:00		1	0	0	0	0	0	0	0	1	1	0	0	0
8:15	9:15		2	0	0	0	0	0	0	0	2	2	0	0	0
8:30	9:30		1	0	0	0	0	0	0	0	1	1	0	0	0
PM PEAK															
16:00	17:00		0	0	0	0	0	0	0	0	0	0	0	0	0
16:15	17:15		0	0	0	0	0	0	0	0	0	0	0	0	0
16:30	17:30		0	0	0	0	0	0	0	0	0	0	0	0	0
16:45	17:45		0	0	0	0	0	0	0	0	0	0	0	0	0
17:00	18:00		0	0	0	0	0	0	0	0	0	0	0	0	0
17:15	18:15		0	0	0	0	0	0	0	0	0	0	0	0	0
17:30	18:30		0	0	0	0	0	0	0	0	0	0	0	0	0
17:45	18:45		0	0	0	0	0	0	0	0	0	0	0	0	0
18:00	19:00		0	0	0	0	0	0	0	0	0	0	0	0	0

B-2 D-86

McLean, Virginia

Existing Traffic Count

0

4:30-5:30

0

0

0

0

PROJECT: DATE: 5/6/2021 **SOUTHBOUND ROAD:** 0 **Higher Learning** DAY: W & A JOB NO.: Thursday **NORTHBOUND ROAD: Site Access** 8455 INTERSECTION: **Bradley Boulevard WEATHER:** WESTBOUND ROAD: Bradley Blvd. & Site Access clear COUNTED BY: Agan **Bradley Boulevard** LOCATION: **EASTBOUND ROAD:** Montgomery Co.,MD **BIKES INPUTED BY:** agan **Turning Movements** Eastbound Southbound Westbound Northbound Site Access PHF Time **Bradley Boulevard Bradley Boulevard** North East Total Time 2 Period 3 10 12 & 5 11 & Period Right Thru Left Total Right Thru Left Total Right Thru Left Total Right Thru Left **Total** South West AM 6:30-6:45 0 6:30-6:45 6:45-7:00 6:45-7:00 11 7:00-7:15 7:00-7:15 7:15-7:30 7:15-7:30 7:30-7:45 7:30-7:45 7:45-8:00 7:45-8:00 8:00-8:15 8:00-8:15 8:15-8:30 8:15-8:30 8:30-8:45 8:30-8:45 8:45-9:00 8:45-9:00 9:00-9:15 0 9:00-9:15 9:15-9:30 9:15-9:30 3 Hour 25 25 26 **Totals** 0 0 0 26 1 Hour **Totals** 0.32 6:30-7:30 6:30-7:30 13 14 17 16 6:45-7:45 16 17 0.39 6:45-7:45 7:00-8:00 11 12 12 0.50 7:00-8:00 10 10 7:15-8:15 10 10 0.42 7:15-8:15 7:30-8:30 11 11 11 11 0.46 7:30-8:30 7:45-8:45 8:00-9:00 0 0 0.25 8:00-9:00 0 0 8:15-9:15 0.25 8:15-9:15 8:30-9:30 0.25 8:30-9:30 AM Peak **AM Peak** 17 6:45-7:45 0 0 0 0 0 16 17 0.39 6:45-7:45 16 PM 4:00-4:15 0 4:00-4:15 4:15-4:30 4:15-4:30 4:30-4:45 4:30-4:45 4:45-5:00 4:45-5:00 5:00-5:15 5:00-5:15 5:15-5:30 5:15-5:30 5:30-5:45 5:30-5:45 5:45-6:00 5:45-6:00 6:00-6:15 6:00-6:15 6:15-6:30 6:15-6:30 6:30-6:45 6:30-6:45 6:45-7:00 6:45-7:00 3 Hour 0 0 0 0 0 14 14 Totals 1 Hour Totals 4:00-5:00 0 0.58 4:00-5:00 4:15-5:15 0.50 4:15-5:15 6 0.67 4:30-5:30 4:30-5:30 0.58 4:45-5:45 4:45-5:45 5:00-6:00 0.42 5:00-6:00 5:15-6:15 0.58 5:15-6:15 5:30-6:30 0.50 | 5:30-6:30 0.38 | 5:45-6:45 5:45-6:45 0.25 6:00-7:00 6:00-7:00 PM Peak PM Peak

0

0

B-3

0

0

0

0

6

0

8

8

0.67 4:30-5:30

McLean, Virginia

Existing Traffic Count

DATE: PROJECT: 5/6/2021 SOUTHBOUND ROAD: Seven Locks Road Higher Learning Seven Locks Road DAY: Thursday W & A JOB NO.: 8455 NORTHBOUND ROAD: INTERSECTION: Bradley Blvd. & Seven Locks Rd. Bradley Boulevard - 191 clear WEATHER: WESTBOUND ROAD: Bradley Boulevard - 191 LOCATION: COUNTED BY: Agan & Christine EASTBOUND ROAD: Montgomery Co.,MD

LOOATION.			ery Co.,iv					INPUTE		agan			LACID				Diagram	Douicvan			
		0 4						Moveme	nts	NI di				- u							
Time	S		bound cks Roa	Н	Brad	Westl dley Bou		191	S	Northl even Lo	oound cks Roa	Ч	Brad		oound levard -	191	North	East	Total	PHF	Time
Period	1	2	3		4	5	6		7	8	9		10	11	12	101	&	&	. o.a.		Period
	Right	Thru	Left	Total	Right	Thru	Left	Total	Right	Thru	Left	Total	Right	Thru	Left	Total	South	West			
AM																					
6:30-6:45	18	28	11	57	5	3	0	8	2	15	1	18	0	6	2	8		16			6:30-6:45
6:45-7:00	25	31	15	71	2	5	0	7	3	12	1	16	2	7	5	14	87	21	108		6:45-7:00
7:00-7:15	19	43	26	88	10	3	6	19	3	13	3	19	2	7	4	13	107	32	139		7:00-7:15
7:15-7:30	24	46	31	101	16	15	4	35	8	30	4	42	3	15	7	25	143	60	203		7:15-7:30
7:30-7:45 7:45-8:00	45 70	76 63	39 75	160 208	24 51	23 71	5	49 127	16	48 89	21	65 126	2	22 46	18 42	42 90	225 334	91 217	316 551		7:30-7:45 7:45-8:00
8:00-8:15	69	111	102	282	80	44	7	131	20	110	9	139	9	103	75	187	421	318	739		8:00-8:15
8:15-8:30	36	134	71	241	60	20	7	87	14	82	2	98	4	60	37	101	339		527		8:15-8:30
8:30-8:45	37	120	69		39	19	7	65	14	56	5	75	1	46	30	77	301	142	443		8:30-8:45
8:45-9:00	41	99	70	210	43	19	12		10	62	7	79	1	25	24	50	289	124	413		8:45-9:00
9:00-9:15	51	85	62	198	41	25	5	71	5	65	3	73	3	13	47	63	271	134	405		9:00-9:15
9:15-9:30	33	74	61	168	31	7	5	43	15	54	1	70	5	11	38	54	238	97	335		9:15-9:30
O Llour																					
3 Hour Totals	468	910	632	2,010	402	254	60	716	119	636	65	820	34	361	329	724	2,830	1,440	4,270		
1 Hour																					
Totals											_		!							<u> </u>	0.00 = 55
6:30-7:30	86				33	26	10		16		9	95									6:30-7:30
6:45-7:45	113	196	111	420 557	52	46	12	110	23	103	16	142	9	51	34 71	94 170	562	204	766		6:45-7:45
7:00-8:00	158	228 296	171 247	557 751	101 171	112 153	17 18	230	36 53	180 277	36 42	252 372	9 16	90	71 142	170 344	809	400 686	1,209		7:00-8:00
7:15-8:15 7:30-8:30	208 220	296 384	247 287	891	215	158	21	342 394	53 59	329	42	428	17	186 231	172	420	1,123 1,319		1,809 2,133		7:15-8:15 7:30-8:30
7:45-8:45	212	428	317	957	230	154	26		64	337	37	438	16		184	455	1,319		2,133		7:45-8:45
8:00-9:00	183	464	312	959	222	102	33		58	310	23	391	15		166	415	1,350	772	2,122		8:00-9:00
8:15-9:15	165	438	272	875	183	83	31	297	43	265	17	325	9	144		291	1,200	588	1,788		8:15-9:15
8:30-9:30	162	378	262	802	154	70	29	253	44	237	16	297	10		139	244	1,099		1,596		8:30-9:30
AM Peak 7:45-8:45	212	428	317	957	230	154	26	410	64	337	37	438	16	255	184	455	1,395	865	2,260	0.76	AM Peak 7:45-8:45
PM	212	720	317	931	230	134	20	710	U-T	331	31	+30	10	200	104	733	1,333	003	2,200	0.70	7.45-0.45
4:00-4:15	33	75	44	152	53	42	12	107	8	111	2	121	0	40	60	100	273	207	480		4:00-4:15
4:15-4:30	34	64	26	124	58	35	5	98	10	103	1	114	4	23	40	67	238	165	403		4:15-4:30
4:30-4:45	35	66	36	137	66	43	10	119	13	128	4	145	2	15	40	57	282	176	458		4:30-4:45
4:45-5:00	43	62	38	143	61	26	14	101	7	100	3	110	4	25	41	70	253		424		4:45-5:00
5:00-5:15	32	62	51	145	65	23	7	95	5	92	6	103	8	25	31	64	248	159	407		5:00-5:15
5:15-5:30	29	46	42	117	64	32	10	106	7	121	7	135	5	15	37	57	252	163	415		5:15-5:30
5:30-5:45 5:45 6:00	21	66 63	39	126	51 71	19	14	84	14	120	4	138	2	27	37	66 50	264 279	150	414		5:30-5:45
5:45-6:00 6:00-6:15	36 31	62 54	37 27	135 112	71 46	36	8 10	115	10 14	125	8 2	143 115	4	23 14	31 30	58 46	278 227	173	451 358		5:45-6:00
6:00-6:15 6:15-6:30	31 48	54 51	27 27	112	46 33	29 22	10 11	85 66	14	99 91	<u> </u>	115	2	14 24	30 24	46 54	227 236	131 120	358 356		6:00-6:15 6:15-6:30
6:30-6:45	13	48	42	103	46	18	10	74	6	65	, 0	71	4	19	23	46	174	120	294		6:30-6:45
6:45-7:00	29	52	22	103	32	14	7	53	10	73	4	87	3	13	28	44	190	97	287		6:45-7:00
3 Hour Totals	384	708	431	1,523	646	339	118	1,103	116	1,228	48	1,392	44	263	422	729	2,915	1,832	4,747		
1 Hour	304	7 00	431	1,323	040	333	110	1,103	110	1,220	40	1,532	44	203	422	1 23	2,313	1,032	7,141		
Totals	=																10:5			2.25	4.00 = 55
4:00-5:00	145		144		238	146	41	425	38		10		10		181	294	1,046		1,765		4:00-5:00
4:15-5:15	144	254	151	549	250	127	36		35	423	14	472	18		152	258	1,021	671	1,692		4:15-5:15
4:30-5:30	139	236	167 170	542 531	256	124	41 45	421	32	441	20	493 496	19 10		149	248 257	1,035		1,704		4:30-5:30
4:45-5:45 5:00-6:00	125 118	236 236	170 169	l	241 251	100 110	45 39	386 400	33 36	433 458	20 25	486 519	19 19		146 136	257 245	1,017 1,042	643 645	1,660 1,687		4:45-5:45 5:00-6:00
5:00-6:00 5:15-6:15	117	236	145		232	116	39 42		45	456 465	25 21	531	13		135	245 227	1,042	617	1,687		5:15-6:15
5:30-6:30	136	233	130		201	106	42 43	350	50	435	21	506	14		122	224	1,021		1,579		5:30-6:30
5:45-6:45	128	215	133		196	105	39	340	42	380	17	439	16		108	204	915		1,459		5:45-6:45
6:00-7:00	121	205	118	l	157	83	38	278	42	328	13	383	15		105	190		468	1,295		6:00-7:00
PM Peak																					PM Peak
4:00-5:00	145	267	144	556	238	146	41	425	38	442	10	490	10	103	181	294	1,046	719	1,765		4:00-5:00

B-4

Seven Locks Road

Project Name	e: Higher I	Learning	_			_	_								
Project Numb	per: 845	55	_	191		1		191							
Location:	Montgome	ery Co.,MD		ا لا		2		h D							
Intersection	n: Bradley	Blvd. & Seve	en Locks Rd.	Leva	8	-	4	Boulevard							
Weather:	clear		-	Boulevard	7		↓ 3	Bou]							
Date:		5/6/2021	-	Bradley		← 5		Bradley							
Surveyor:	Agan & (Christine	-	Bra		6 ──		Bra							
			<u>-</u>		Se	ven Locks R	oad								
Hourly Peo	destrian (Count													
			1	2	3	4	5	6	7	8					
		From:	SE	NE	SW	SE	SW	NW	NW	NE	Total	1 & 2	3 & 4	5 & 6	7 & 8
Time 1	Period	To:	NE	SE	SE	SW	NW	SW	NE	NW					
AM PEAK															
6:30	7:30		1	1	0	1	0	1	0	2	6	2	1	1	2
6:45	7:45		1	1	0	1	0	1	0	2	6	2	1	1	2
7:00	8:00		1	1	0	1	0	1	0	0	4	2	1	1	0
7:15	8:15		1	1	0	1	0	1	0	0	4	2	1	1	0
7:30	8:30		0	0	0	0	0	0	0	0	0	0	0	0	0
7:45	8:45		0	0	0	0	0	0	1	1	2	0	0	0	2
8:00	9:00		0	0	0	0	0	0	1	1	2	0	0	0	2
8:15	9:15		3	0	3	0	0	2	1	4	13	3	3	2	5
8:30	9:30		6	0	6	0	3	2	9	8	34	6	6	5	17
PM PEAK															
16:00	17:00		0	0	1	0	1	1	2	0	5	0	1	2	2
16:15	17:15		0	0	2	0	1	1	1	0	5	0	2	2	1
16:30	17:30		0	0	1	0	1	1	1	0	4	0	1	2	1
16:45	17:45		1	0	1	0	2	3	1	2	10	1	1	5	3
17:00	18:00		1	0	1	0	1	3	0	2	8	1	1	4	2
17:15	18:15		1	0	0	0	1	5	0	2	9	1	0	6	2
17:30	18:30		1	0	0	0	1	5	0	2	9	1	0	6	2
17:45	18:45		0	0	0	0	0	3	0	0	3	0	0	3	0
18:00	19:00		0	1	0	0	0	3	0	0	4	1	0	3	0

McLean, Virginia

Existing Traffic Count

PM Peak 4:00-5:00

DATE: Seven Locks Road PROJECT: 5/6/2021 SOUTHBOUND ROAD: Higher Learning W & A JOB NO.: DAY: Seven Locks Road 8455 Thursday NORTHBOUND ROAD: WEATHER: clear WESTBOUND ROAD:

INTERSECTION: Bradley Blvd. & Seven Locks Rd. Bradley Boulevard - 191

OCATION	:	Montgon		even Lock MD				COUNT			Christin	e	EASTB	OUND F			_	Boulevar Boulevar			
								Moveme		91					_						
		South	bound			West	bound			North	bound			Eastl	bound				1		
Time	S	Seven Lo	cks Roa	ad	Bra	ıdley Boı	ulevard -	191	S	Seven Lo	cks Roa	ıd	Bra	dley Βοι	ulevard -	191	North	East	Total	PHF	Tim
Period	1 Right	2 Thru	3 Left	Total	4 Right	5 Thru	6 Left	Total	7 Right	8 Thru	9 Left	Total	10 Right	11 Thru	12 Left	Total	& South	& West			Perio
AM																					0.00.0
30-6:45	0	0	0	0	0	0	0	0	0		0	0	0	2	0	2	0	2	2		6:30-6:
45-7:00	0	0	0	0	0	9	0	9	0	0	0	0	0	0	0	0	0	9	9		6:45-7:
00-7:15	0	0	1	1	0	1	0	1	1	0	0	1	0	0	0	0	2	1	3		7:00-7:
15-7:30	0	0	2	2	0	1	0	1	0	0	0	0	0	0	0	0	2	2 1	3		7:15-7:
30-7:45	0	0	1	1	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1		7:30-7
45-8:00	0	0	6	6	0	0	0	0	0	1	0	1	0	1	0	1	7	' 1	8		7:45-8:
00-8:15	1	1	0	2	0	0	0	0	0	0	0	0	0	0	0	0	2	2 0	2		8:00-8:
15-8:30	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	0	1	1		8:15-8:
30-8:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		8:30-8:
45-9:00	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1		8:45-9:
00-9:15	0	0	0	0	0	0	0	0	0	2	0	2	0	0	0	0	2	2 0	2		9:00-9:
15-9:30	0	2	0	2	0	0	0	0	0	1	0	1	0	1	0	1	3	1	4		9:15-9
3 Hour																					
Totals 1 Hour	1	4	10	15	0	11	0	11	1	4	0	5	0	5	0	5	20	16	36		
Totals																					
30-7:30	0	0	3	3	0	11	0	11	1	0	0	1	0	2	. n	2	4	. 13	17	0.47	6:30-7
45-7:45	0		4	4			o	11		<u> </u>		1	0	1) n	_	5	11	16		6:45-7
00-8:00	0		10	10		1		2		1		2				1	12	3	15		7:00-8
15-8:15	0		10	11		1		1	'			1			0		12		14		7:15-8
			9				0	1			0	1		1							
30-8:30	1 4	1	<i>'</i>	9	_		0	0	0	1	0	1	0	2	0	2	10		12		7:30-8
45-8:45	1	1	6				0	0	_	1	0	1	0	2	0	2			11		7:45-8
00-9:00	1	2	0	3			0	0		0	0	0	0	1	0	1	3	$\begin{bmatrix} 1 \\ 1 \end{bmatrix}$	4		8:00-9:
15-9:15	0		0	1	0	0	0	0	0	2	0	2	0	1	0	1	3	1	4		8:15-9:
30-9:30	0	3	0	3	0	0	0	0	0	3	0	3	0	1	0	1	6	1	7	0.44	8:30-9:
M Peak 30-7:30	0	0	3	3	0	11	0	11	1	0	0	1	0	2	0	2	4	13	17	0.47	AM Pea 6:30-7:
PM				_	_			_	_			_	_								
00-4:15	2	1	1	4	0	0	0	0	0	6	0	6	0	0	0	0	10	0	10		4:00-4:
15-4:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		4:15-4:
30-4:45	0	0	0	0	1	0	0	1	0	1	0	1	0	3	0	3	1	4	5		4:30-4:
45-5:00	0	0	0	0	1	0	0	1	0	2	0	2	0	0	0	0	2	2 1	3		4:45-5:
00-5:15	1	1	1	3	0	0	0	0	0	0	0	0	0	0	1	1	3	1	4		5:00-5
15-5:30	0	2	1	3	0	0	0	0	0	1	0	1	0	1	0	1	4	1	5		5:15-5
30-5:45	0	3	0	3	0	0	0	0	0	0	0	0	0	1	0	1	3	1	4		5:30-5:
45-6:00	0	1	0	1	0	0	0	Ô	0	0	0	Ó	0	0	0	0	1	0	1		5:45-6
00-6:15	0	n	0	0	0	1	0	1	0	n	0	o o	0	1	1	2	0	3	3		6:00-6
15-6:30	0	l n		<u> </u>		0	l	0	0	1	l	1	<u> </u>	0	n		1		1		6:15-6:
30-6:45	0) n	0	n	0	1		1	0	, ,		, ,) n	0	, ,	1	1		6:30-6
45-7:00	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1	Ö	1		6:45-7
3 Hour																					
Totals 1 Hour	3	9	3	15	2	2	0	4	0	11	0	11	0	6	2	8	26	12	38		
Totals																					
00-5:00	2	1	1	4	2	0	0	2	0	9	0	9	0	3	0	3	13	5	18	0.45	4:00-5
15-5:15	1	1	1	3	2		0	2		3	0	3	0	3	1	4	6	_	12		4:15-5:
30-5:30	1	3	2	6	2	0	0	2	0	4	0	4	0	4	. 1	5	10		17		4:30-5
45-5:45	1	6	2	9	-	<u> </u>	<u> </u>	1	<u> </u>	3	<u> </u>	3	<u> </u>	2	1	3	12		16		4:45-5
00-6:00	1	7	2	10		n	o	, ,	0	1	o	1	0	2		3	11		14		5:00-6
15-6:15	Ö	6	1	7	^	1	^	1	^	1			^	2		1	8		13		5:15-6
	0			'				1			^	1		3			6				5:30-6
30-6:30				4												3] 4	2		
45-6:45	0		0] 1	0	2	0	2			0	1		1] 1	2	2	4			5:45-6
00-7:00	0	1		1		2	0	2		1	0	1		1	1	2	2	4	6	0.50	6:00-7
																					+
		•						•		•		i	•		•		•				•

B-6

PM Peak 0.45 4:00-5:00

McLean, Virginia

Existing Traffic Count

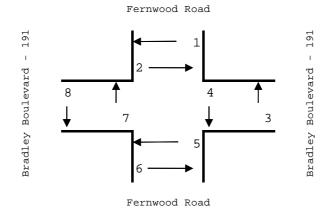
PROJECT: DATE: Fernwood Road Higher Learning 5/6/2021 SOUTHBOUND ROAD: W & A JOB NO.: DAY: Fernwood Road 8455 Thursday NORTHBOUND ROAD: Bradley Boulevard - 191 Bradley Boulevard - 191 INTERSECTION: Bradley Blvd. & Fernwood Rd. WEATHER: WESTBOUND ROAD: clear

LOCATION: COUNTED BY: Agan Montgomery Co.,MD EASTBOUND ROAD:

LOCATION:		workgon	ery Co.,N	ال <u>ا</u>	_	_		INPUTE		agan	_	_	LAGID	OUND F		_	Di auley I	Boulevard	u - 181	_	
		South	bound				Turning bound	Moveme	nts	North	bound			Facth	oound						
Time	ļ	Fernwo	od Road			dley Bou	levard -	191		Fernwoo	od Road				levard -	191	North	East	Total	PHF	Time
Period	1 Right	2 Thru	3 Left	Total	4 Right	5 Thru	6 Left	Total	7 Right	8 Thru	9 Left	Total	10 Right	11 Thru	12 Left	Total	& South	& West			Period
АМ																					
6:30-6:45	3	1	14	l I	9	3	0	12	0	0	1	1	0	10	3	13		1			6:30-6:45
6:45-7:00 7:00-7:15	11	1	21 39	31 53	11 18	5 18	1	16 37	1	1	1	3	0	16 21	9	25 32	34 57	41 69	75 126		6:45-7:00 7:00-7:15
7:00-7:13 7:15-7:30	12	2	41	55	19	24	0	43	1	2	1	4	1	31	10	32 42	59	85	144		7:15-7:30
7:30-7:45	20	7	42	69	21	58	0	79	0	1	0	1	0	52	14	66		145			7:30-7:45
7:45-8:00	26	6	52		25	86	2	113	0	3	2	5	0	81	12	93					7:45-8:00
8:00-8:15	25	4	62		24	91	0	115	2	4	0	6	0	129	21	150		265	362		8:00-8:15
8:15-8:30	14	5	74		39	84	1	124	1	2	1	4	1	119	18	138		262			8:15-8:30
8:30-8:45	13	8	69		25	67	0		1	5	2	8	0	99	15	114					8:30-8:45
8:45-9:00	14 10	5	41	60	32 27	58 51	0	90	0	3	2	5	0	91	14 11	105 79		195	l l		8:45-9:00
9:00-9:15 9:15-9:30	10 Ω	4	32 29		26	42	0	79 68	1	4	- 0	5	1	68 54	10	79 65	49 45	158 133	207 178		9:00-9:15 9:15-9:30
	0	3	29	40	20	42	U	00	'	4	O	3	'	34	10	03	43	100	170		9.13-9.30
3 Hour Totals	165	49	516	730	276	587	5	868	9	27	13	49	3	771	148	922	779	1,790	2,569		
1 Hour																					
Totals 6:30-7:30	35	7	115	157	57	50	1	108	1	3	5	12	1	78	33	112	169	220	389	U 68	6:30-7:30
6:45-7:45	52	13		1	69	105	1	175	4 4	3 ⊿		12	1	120	44	165	220	340			6:45-7:45
7:00-8:00	69	18	1	I I	83	186	3	1	3	6	5	14	1	185	47	233	275	505			7:00-8:00
7:15-8:15	83	19		299	89	259	2	350	3	10	3	16	1	293	57	351	315	1	1,016		7:15-8:15
7:30-8:30	85	22		I I	109	319	3	I I	3	10	3	16	1	381	65	447	353	878	· ·	0.85	7:30-8:30
7:45-8:45	78	23		358	113	328	3		4	14	5	23	1	428	66	495	381	939	1,320		7:45-8:45
8:00-9:00	66	22	246	I I	120	300	1	421	4	14	5	23	1	438	68	507	357	928	1,285		8:00-9:00
8:15-9:15	51	22		I I	123	260	2	385	2	12	6	20	1	377	58	436		821	1,130		8:15-9:15
8:30-9:30	45	20	171	236	110	218	1	329	2	14	5	21	1	312	50	363	257	692	949	0.78	8:30-9:30
AM Peak 7:45-8:45	78	23	257	358	113	328	3	444	4	14	5	23	1	428	66	495	381	939	1,320	0.91	AM Peak 7:45-8:45
PM					İ																
4:00-4:15	21	5	31	57	47	61	0	108	1	2	0	3	0	47	9	56		164	224		4:00-4:15
4:15-4:30	19	2	28		53 49	80 87	1	134	1	4	2	7	1	44	12	57	56	191	247		4:15-4:30
4:30-4:45 4:45-5:00	15 23	4 5	33 38		49 57	87 81	2	137 140	1	2	7	4	1	58 56	14 10	74 67	56 73		267 280		4:30-4:45 4:45-5:00
5:00-5:15	20	5 6	39		61	78	1	140	0	5 5	2	7	2	58	11	71	73 72		283		5:00-5:15
5:15-5:30	18	5	26		48	82	1	131	1	9	4	14	1	53	16	70			264		5:15-5:30
5:30-5:45	20	8	39		49	72	0	121	1	3	1	5	1	55	11	67	72	188			5:30-5:45
5:45-6:00	23	5	25	53	56	73	2	131	0	5	1	6	2	56	21	79		210			5:45-6:00
6:00-6:15	24	2	36		39	64	1	104	0	7	2	9	2	34	22	58		162	233		6:00-6:15
6:15-6:30	16	7	36		47	59	2	108	1	4	0	5	2	42	17	61	64	169	233		6:15-6:30
6:30-6:45	21 14	5	34 30	60 51	38 24	44 38	2	84 62	0	2	1	3	1	44 39	11 17	56 56		140 118	203 174		6:30-6:45
6:45-7:00	14	,	30	31	24	30	U	02	U	4	1) 	U	39	17	36	36	110	174		6:45-7:00
3 Hour Totals	234	61	395	690	568	819	13	1,400	7	50	18	75	15	586	171	772	765	2,172	2,937		
1 Hour	207	01	333	330	330	313	13	1,700	•	30	10	, ,	13	300	171	112	, 00	£,11£	2,337		
Totals																					
4:00-5:00	78	16		1	206	309	4	1	4	11	6	21	4		45	254	245		· · · · · · · · · · · · · · · · · · ·		4:00-5:00
4:15-5:15	77	17			220	326	5	I I	3		8	25	6		47	269		820			4:15-5:15
4:30-5:30	76	20			215	328	5		3		10		6		51	282			•		4:30-5:30
4:45-5:45 5:00-6:00	81 81	24 24		I I	215 214	313 305	4 1	532 523	3 2	20 22	10 8	33 32	5 6	222 222	48 59	275 287	280 266	807 810	1,087 1,076		4:45-5:45 5:00-6:00
5:15-6:15	85	24 20		1	192	291	4	487	2	22 24	٥ و	34	6			267 274	265 265	761	1,076		5:15-6:15
5:30-6:30	83	22			192	268	5	I I	2	19	4	25	7	187	70	265			995		5:30-6:30
5:45-6:45	84	19		234	180	240	7		1	18	4	23	7	176	71	254	257	681	938		5:45-6:45
6:00-7:00	75	21	136	I I	148	205	5	358	1	17	4	22	5	159	67	231	254		843		6:00-7:00
PM Peak																					PM Peak
4:30-5:30	76	20	136	232	215	328	5	548	3	19	10	32	6	225	51	282	264	830	1,094	0.97	4:30-5:30

B-7

Project Name:	Higher Learning
Project Number	: 8455
Location:	Montgomery Co.,MD
Intersection:	Bradley Blvd. & Fernwood Rd.
Weather:	clear
Date:	5/6/2021



Hourly Pedestrian Count

Agan

Surveyor:

			1	2	3	4	5	6	7	8					
		From:	SE	NE	SW	SE	SW	NW	NW	NE	Total	1 & 2	3 & 4	5 & 6	7 & 8
Time	Period	To:	NE	SE	SE	SW	NW	SW	NE	NW					
AM PEAK															
6:30	7:30		0	1	0	0	0	0	0	0	1	1	0	0	0
6:45	7:45		0	1	0	0	0	0	0	0	1	1	0	0	0
7:00	8:00		0	1	0	0	0	0	0	0	1	1	0	0	0
7:15	8:15		0	1	0	0	0	0	0	0	1	1	0	0	0
7:30	8:30		1	0	0	0	0	0	0	0	1	1	0	0	0
7:45	8:45		1	1	0	0	0	0	0	0	2	2	0	0	0
8:00	9:00		2	1	0	0	0	0	0	0	3	3	0	0	0
8:15	9:15		2	1	0	0	0	0	0	0	3	3	0	0	0
8:30	9:30		1	1	0	0	0	0	0	0	2	2	0	0	0
PM PEAK															
16:00	17:00		0	0	0	0	0	0	0	0	0	0	0	0	0
16:15	17:15		0	0	0	0	0	0	0	0	0	0	0	0	0
16:30	17:30		0	0	0	0	0	0	0	0	0	0	0	0	0
16:45	17:45		0	0	0	0	0	0	0	0	0	0	0	0	0
17:00	18:00		0	0	0	0	1	0	0	0	1	0	0	1	0
17:15	18:15		0	0	0	0	1	0	0	0	1	0	0	1	0
17:30	18:30		0	0	0	0	1	0	0	0	1	0	0	1	0
17:45	18:45		0	0	0	0	1	0	0	0	1	0	0	1	0
18:00	19:00		0	0	0	0	0	0	0	0	0	0	0	0	0

B-8 D-92

McLean, Virginia

Existing Traffic Count

PROJECT: Higher Learning DATE: 5/6/2021 SOUTHBOUND ROAD: Fernwood Road W & A JOB NO.: 8455 DAY: Thursday NORTHBOUND ROAD: Fernwood Road INTERSECTION: Bradley Blvd. & Fernwood Rd. WEATHER: clear WESTBOUND ROAD: Bradley Boulevard - 191 LOCATION: Montgomery Co.,MD COUNTED BY: Agan EASTBOUND ROAD: Bradley Boulevard - 191

INTERSECT LOCATION:		•	Blvd. & Fonds	ernwood F MD	Rd.			WEATH COUNTI INPUTE	ED BY:	clear Agan agan				BOUND OUND F BIKE	ROAD:		•	Boulevar Boulevar			
							Turning			ayan				DIILE							
		South	bound			West	bound	Moveme	7115	North	bound			Fastk	oound						
Time			od Road	4	Bra	dley Bou		- 191			od Road		Brad	dley Bou		191	North	East	Total	PHF	Time
Period	1	2	3	<u> </u>	4	5	6		7	8	9		10	11	12	I	& &	&	l		Period
	Right	Thru	Left	Total	Right	Thru	Left	Total	Right	Thru	Left	Total	Right	Thru	Left	Total	South	West			
AM 5:30-6:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		6:30-6:45
6:45-7:00					0	0	0		0	0	9	9	0	2		2	9	2	11		6:45-7:00
':00-7:15	0	0	1		0	0	0	0	0	0	0	0	0	1	0	1	1	1	2		7:00-7:15
':15-7:30	1				0	0	0	0	0	0	0	0	0	1		1		1	2		7:15-7:30
':30-7:45	0				0	0	0	0	0	0	0	0	0	3		3	0	3	3		7:30-7:4
':45-8:00	0			0	0	0	0		0	0	0	0	0	6		6	0	6	6		7:45-8:00
3:00-8:15	0	Ö		0	0	0	0	0	0	0	0	0	0	0	0		0				8:00-8:1
3:15-8:30	1				0	0	0	0	0	0	0	0	0	0	0	0	1		1		8:15-8:30
3:30-8:45	0	0	1		0	0	0	0	0	0	0	0	0	1	0	1	1		2		8:30-8:4
3:45-9:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0			8:45-9:00
9:00-9:15	0	0	2	2	0	0	0	0	0	0	0	0	0	0	0		2		2		9:00-9:15
9:15-9:30	0	0	0	0	0	0	0	0	0	0	0	Ö	0	1	0	1	0	1	1		9:15-9:30
3 Hour																					
Totals	2	0	4	6	0	0	0	0	0	0	9	9	0	15	0	15	15	15	30		
1 Hour			<u> </u>																		
Totals																					
6:30-7:30	1	0	1	2	0	0	0	0	0	0	9	9	0	4	0	4	11	4	15	0.34	6:30-7:30
6:45-7:45	1		1	2	0	0	0	0		0	9	9	0	7	0	7	11	7	18		6:45-7:45
7:00-8:00	1	0	1	2	0	0	0	0		0	0	0	0	11	0	11		11			7:00-8:00
7:15-8:15	1	0	o	$\frac{1}{1}$	0	0	0	0	_	0	0	0	0	10		10		10			7:15-8:1
':30-8:30	1	0	o	1	0	0	0	0	0	0	0	0	0	9	0	9	1	9	10		7:30-8:30
7:45-8:45	1	0	1	2	0	0	0	0	0	0	0	0	0	7	0	7	2	7	9		7:45-8:45
3:00-9:00	1	0	1	2	0	0	0			0	0	0	0	1	0	1	2	1	3		8:00-9:00
3:15-9:15	1	0	3	4	0	0	0	0	0	0	0	0	0	1	0	1	4	1	5	0.63	8:15-9:15
3:30-9:30	0	0	3	3	0	0	0	0	0	0	0	0	0	2	0	2	3	2	5	0.63	8:30-9:30
AM Peak																					AM Peak
6:45-7:45	1	0	1	2	0	0	0	0	0	0	9	9	0	7	0	7	11	7	18	0.41	6:45-7:45
PM																					
4:00-4:15	0	0	1	1	0	0	0	0	0	0	0	0	0	1	0	1	1	1	2		4:00-4:15
4:15-4:30	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	0	1	1		4:15-4:30
4:30-4:45	1	0	1	2	0	2	0	2	0	0	0	0	0	0	0	0	2	2	4		4:30-4:45
4:45-5:00	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	2	0	2	2		4:45-5:00
5:00-5:15	0	0	0	0	1	0	0	1	0	0	0	0	0	1	0	1	0	2	2		5:00-5:15
5:15-5:30	0	0	1	1	0	0	0	0	0	0	0	0	0	2	0	2	1	2	3		5:15-5:30
5:30-5:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		5:30-5:45
5:45-6:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		5:45-6:00
6:00-6:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		6:00-6:15
6:15-6:30	0	0	0	0	1	0	0	1	0	0	0	0	0	0	0	0	0	1	1		6:15-6:30
6:30-6:45	0			1	1	0	1	2	0	_	0	0	0	U	0	0		2	3		6:30-6:4
6:45-7:00	0	0	0	0	2	0	0	2	0	0	0	0	0	0	0	0	0	2	2		6:45-7:00
3 Hour Totals	1	0	4	5	5	2	1	8	0	0	0	0	0	7	0	7	5	15	20		
1 Hour	<u>'</u>	"	4	. <u>3</u>	3		<u> </u>	0	U	<u> </u>	0	0	U	<u> </u>	<u> </u>	 	3	13	20		
Totals																					
:00-5:00	1	0	2	3	0	2	0	2	0	0	<u> </u>	0	0	Δ	0	1	3	6	9	0.56	4:00-5:00
:15-5:15			1	2	1	2) 	3		_		0	0	4	o	4	2	7	9		4:15-5:1
:30-5:30	1		2	2	1	2	0				0	0	0	5	0	5	3	, 8	11		4:30-5:30
:45-5:45	0		1	1	1	0	0	1	0	0	0	0	0	5	0	5	1	6	7		4:45-5:4
:00-6:00				1	1) 	1	o) 	o			3		3		4	5		5:00-6:0
:15-6:15			1	1) n) 0	0	0			2	o	2	1	7	3		5:15-6:1
5:30-6:30		1			1) n) 0	0	0	0	٠ ١	0	0	^		1	3		5:30-6:3
5:45-6:45			1	1	2		1	3	0) n	0	0	0		0	^	1	2	'		5:45-6:4
3:00-7:00	0		1		4	o o	1	5	0	0	0	0	0	0	o o	0	1	5	6		6:00-7:0
PM Peak																					DM Doc
'M Реак :30-5:30	1	0	2	3	1	2	0	3	0	0	0	0	0	5	0	5	3	8	11		PM Peal 4:30-5:3

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McLean, Virginia

Existing Traffic Count

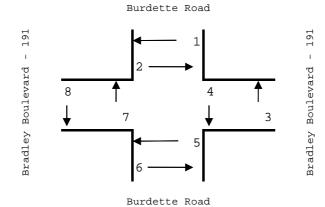
PROJECT: DATE: **Burdette Road** Higher Learning 5/6/2021 SOUTHBOUND ROAD: W & A JOB NO.: DAY: **Burdette Road** 8455 Thursday NORTHBOUND ROAD: INTERSECTION: Bradley Blvd. & Burdette Rd. WEATHER: WESTBOUND ROAD: clear

Bradley Boulevard - 191 Bradley Boulevard - 191 LOCATION: COUNTED BY: Agan Montgomery Co.,MD EASTBOUND ROAD:

LOCATION:		Montgon	nery Co.,N	الا 				INPUTE		agan			EASID	OUND R	.ОAD.		Diauley I	Boulevar	u - 191		
		South	bound				Turning bound	Moveme	nts	North	bound			Faeth	ound						
Time		Burdett	e Road	•	I		ılevard -	191		Burdett	e Road	г		dley Bou	levard -	191	North	East	Total	PHF	Time
Period	1 Right	2 Thru	3 Left	Total	4 Right	5 Thru	6 Left	Total	7 Right	8 Thru	9 Left	Total	10 Right	11 Thru	12 Left	Total	& South	& West			Period
AM	0	4		2	0		4	7	4	0	0	4	0	40	0	4.4	4	24	25		0.20 0.45
6:30-6:45 6:45-7:00	1	1	3	5	1	6 7	1 5	13	1	1	0	2	0	12 18	2	14 20	7	21 33	25 40		6:30-6:45 6:45-7:00
7:00-7:15	7	1	1	9	2	25	5	32	1	2	1	4	0	26	3	29	13	l	74		7:00-7:15
7:15-7:30	5	1	0	6	2	25	1	28	2	2	1	5	4	51	3	58	11	86	97		7:15-7:30
7:30-7:45 7:45-8:00	22	10	1 4	36	3	61 117	9	73 128	1 3	3 5	2	10	7	58 90	1 6	59 103	11 46	132 231	143 277		7:30-7:45 7:45-8:00
8:00-8:15	14		6		7	71	7	85	5	9	4	18		142	20	167	59		311		8:00-8:15
8:15-8:30	19		13		8	68	10	86	2	8	1	11	6	127	22	155			301		8:15-8:30
8:30-8:45 8:45-9:00	16 9	15 8	10	41 25	4	59 53	9	72 61	3	7	2	15 12	5	81 90	15 8	96 103	56 37		224 201		8:30-8:45 8:45-9:00
9:00-9:15	5	3	5	13	5	59	11	75	1	7	1	9	3	61	4	68	22	143	165		9:00-9:15
9:15-9:30	1	4	5	10	2	34	4	40	4	4	1	9	3	50	4	57	19	97	116		9:15-9:30
3 Hour Totals	102	85	58	245	42	585	73	700	30	55	15	100	33	806	90	929	345	1,629	1,974		
1 Hour Totals																					
6:30-7:30	13	4	6	23	5	63	12	80	5	5	2	12	4	107	10	121	35	201	236	0.61	6:30-7:30
6:45-7:45	16		5	27	8		20	146	5	8	2	15		153	9	166			354		6:45-7:45
7:00-8:00 7:15-8:15	37 44			58 90	11 16	228 274	22 24	261 314	7 11	12 19	4 7	23 37	11 16	225 341	13 30	249 387	81 127	510 701	591 828		7:00-8:00 7:15-8:15
7:30-8:30	58		24	133	22	317	33	372	11	25	7	43	18	417	49	484	176				7:30-8:30
7:45-8:45	71			167	23		33		16		9	54	18	440	63	521	221	892	,		7:45-8:45
8:00-9:00 8:15-9:15	58 49		37 36	156 128	23 21	251 239	30 34	304 294	16 12	31 29	9	56 47	16 14	440 359	65 49	521 422	212 175		1,037 891		8:00-9:00 8:15-9:15
8:30-9:30	31	30	28	89	15		28	248	14	25	6	45	11	282	31	324	134		706		8:30-9:30
AM Peak 7:45-8:45	71	63	33	167	23	315	33	371	16	29	9	54	18	440	63	521	221	892	1,113	0.89	AM Peak 7:45-8:45
PM	_						_								_						
4:00-4:15 4:15-4:30	6	3	1	11 16	6 8	70 88	6	83 102	8 4	10 4	4	22 10	2	54 43	9	65 52	33 26		181 180		4:00-4:15 4:15-4:30
4:30-4:45	19	9	6	34	11		11	109	13	9	2	24		52	6	64	58				4:30-4:45
4:45-5:00	7	8	4	19	7	83	7	97	9	5	6	20		63	3	68					4:45-5:00
5:00-5:15 5:15-5:30	11	8	5 5	21 24	9 4	76 89	11 9	96 102	9	6 9	1 6	16 21	3 1	61 60	9	73 63					5:00-5:15 5:15-5:30
5:30-5:45	5	3	4	12	6	82	5	93	5	9	2	16	1	69	9	79	28	172	200		5:30-5:45
5:45-6:00 6:00-6:15	12 12	7	7	26 26	6	92 77	11	109 87	8	3	1	12 19	1	55 45	3	59 47	38 45		206 179		5:45-6:00 6:00-6:15
6:15-6:30	9	6	2	17	8	57	5	70	3	6	3	12	0	57	3	60		134			6:15-6:30
6:30-6:45	8	3	1	12	3	54	5	62	4	4	5	13	0	52	8	60	25	122	147		6:30-6:45
6:45-7:00	1	4	3	8	4	50	3	57	2	12	3	17	0	48	1	49	25	106	131		6:45-7:00
3 Hour Totals	105	73	48	226	77	905	85	1,067	78	86	38	202	18	659	62	739	428	1,806	2,234		
1 Hour	133		1.5			333		.,		33	33			333		- 33		-,	_,,		
Totals 4:00-5:00	39	26	15	80	32	328	31	391	34	28	14	76	12	212	25	249	156	640	796	0 8A	4:00-5:00
4:15-5:15	40	30	20	90	35	334	35	404	35	24	11	70	13	219	25	257	160	661	821		4:15-5:15
4:30-5:30	45				31 26	335	38	404	37	29	15		12	236	20	268					4:30-5:30
4:45-5:45 5:00-6:00	31 36	26 25			26 25	330 339	32 36	388 400	29 28	29 27	15 10			253 245	23 23	283 274	149 148		820 822		4:45-5:45 5:00-6:00
5:15-6:15	40	27	21	88	21	340	30	391	26	30	12	68	3	229	16	248	156	639			5:15-6:15
5:30-6:30	38				25		26		23	27	9	59	2	226	17	245			744		5:30-6:30
5:45-6:45 6:00-7:00	41 30	25 22	15 11	81 63	22 20	280 238	26 18	328 276	22 16	22 31	12 14	56 61	0	209 202	16 14	226 216	137 124		691 616		5:45-6:45 6:00-7:00
PM Peak 4:30-5:30	45	32	21	98	31	335	38	404	37	29	15	81	12	236	20	268	179	672	851	0.92	PM Peak 4:30-5:30

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Project Name:	Higher Learning
Project Number	: 8455
Location:	Montgomery Co.,MD
Intersection:	Bradley Blvd. & Burdette Rd.
Weather:	clear
Date:	5/6/2021



Hourly Pedestrian Count

Agan

Surveyor:

			1	2	3	4	5	6	7	8					
		From:	SE	NE	SW	SE	SW	NW	NW	NE	Total	1 & 2	3 & 4	5 & 6	7 & 8
	Period	To:	NE	SE	SE	SW	NW	SW	NE	NW					
AM PEAK															
6:30	7:30		1	1	0	0	0	0	1	1	4	2	0	0	2
6:45	7:45		1	1	0	0	0	0	0	1	3	2	0	0	1
7:00	8:00		1	1	0	0	0	0	0	0	2	2	0	0	0
7:15	8:15		1	0	0	0	0	0	0	0	1	1	0	0	0
7:30	8:30		0	0	0	0	0	0	0	0	0	0	0	0	0
7:45	8:45		0	0	0	0	0	0	0	0	0	0	0	0	0
8:00	9:00		0	0	0	0	0	0	0	0	0	0	0	0	0
8:15	9:15		0	0	0	0	0	0	0	0	0	0	0	0	0
8:30	9:30		0	0	0	0	0	0	0	0	0	0	0	0	0
PM PEAK															
16:00	17:00		0	0	0	0	0	0	0	0	0	0	0	0	0
16:15	17:15		0	0	0	0	0	0	0	0	0	0	0	0	0
16:30	17:30		0	0	0	0	0	0	0	0	0	0	0	0	0
16:45	17:45		0	0	0	0	0	0	0	0	0	0	0	0	0
17:00	18:00		0	0	0	0	0	0	0	0	0	0	0	0	0
17:15	18:15		0	0	0	0	0	0	0	0	0	0	0	0	0
17:30	18:30		0	0	0	0	0	0	0	0	0	0	0	0	0
17:45	18:45		0	0	0	0	0	0	0	0	0	0	0	0	0
18:00	19:00		0	0	0	0	0	0	0	0	0	0	0	0	0

B-11 D-95

McLean, Virginia

Existing Traffic Count

DATE: PROJECT: 5/6/2021 SOUTHBOUND ROAD: **Burdette Road** Higher Learning DAY: W & A JOB NO.: Thursday NORTHBOUND ROAD: **Burdette Road** 8455 INTERSECTION: Bradley Blvd. & Burdette Rd. WEATHER: Bradlev Boulevard - 191 clear WESTBOUND ROAD:

INTERSECT LOCATION:			Blvd. & B nery Co.,	urdette Ro MD	l.			WEATH	ED BY:				WESTE EASTB	OUND F	ROAD:		•	Boulevar Boulevar			
	ļ.						T	INPUTE		agan				BIKE	5				<u> </u>		Π
		South	bound			\/\act	Turning bound	Moveme	ents	North	bound			Facth	oound						
Time			te Road		Bra	dley Bou		- 191			te Road		Brad		levard -	191	North	East	Total	PHF	Time
Period	1	2	3		4	5	6		7	8	9		10	11	12		&	&			Period
	Right	Thru	Left	Total	Right	Thru	Left	Total	Right	Thru	Left	Total	Right	Thru	Left	Total	South	West			
AM 6:30-6:45	0	1		1	0	0	0	0	0	0	0	0	0	1	0	1	1	1	2		6:30-6:45
6:45-7:00				0	0	9		9		0			0	2	0	2	0	11	11		6:45-7:00
7:00-7:15	0		1		0	1			0	0	0	0	0	1	0	1	1	2	3		7:00-7:15
7:15-7:30	0	0	C	0	0	2	0	2	0	0	0	0	0	1	0	1	0	3	3		7:15-7:30
7:30-7:45	1	C	C	1	0	0	0	0	0	0	0	0	0	2	0	2	1	2	3		7:30-7:4
' :45-8:00	0	C	1	1	0	0	0	0	0	0	0	0	0	1	0	1	1	1	2		7:45-8:00
3:00-8:15	0	C	C	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		8:00-8:1
3:15-8:30	0	C	C	0	0	0	0	0		0	0	0	0	1	0	1	0	1	1		8:15-8:30
3:30-8:45	0	0		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		8:30-8:45
3:45-9:00	0	0		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		8:45-9:00
9:00-9:15	0			0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		9:00-9:15
9:15-9:30	0) 0	U	U	0		U	U	0		Ü	1	0	1	0	1	1		9:15-9:30
3 Hour Totals	1	1	2	2 4	0	12	0	12	0	0	0	0	0	10	0	10	4	22	26		
1 Hour	<u> </u>	•					3	12			—			1.0			-		20		
Totals																					
6:30-7:30	0	1	1	2	0	12	О	12	0	0	0	0	0	5	0	5	2	17	19	0.43	6:30-7:30
6:45-7:45	1	0	1	2	0	12				0	0	0	0	6	0	6	2	18			6:45-7:45
7:00-8:00	1	0	2	2 3	0	3	0	3	0	0	0	0	0	5	0	5	3	8	11		7:00-8:00
7:15-8:15	1	0	1	2	0	2	0	2	0	0	0	0	0	4	0	4	2	6	8		7:15-8:15
7:30-8:30	1	0	1	2	0	0	0	0		0	0	0	0	4	0	4	2	4	6		7:30-8:30
7:45-8:45	0		1	1	0	0	0			_	0	0	0	2	0	2	1	2	3		7:45-8:45
3:00-9:00	0	0	C		0	0	0	0	0	0	0	0	0	1	0	1	0	1	1		8:00-9:00
8:15-9:15 8:30-9:30					0	0	0	0	0	0	0	0	0	1	0	1	0	1	1 1		8:15-9:15
<u> </u>	U	U		, 0	U	U	U	U	U	0	U	U	0	'	U	'	U	'	'	0.23	8:30-9:30
AM Peak																					AM Peak
6:45-7:45	1	0	1	2	0	12	0	12	0	0	0	0	0	6	0	6	2	18	20	0.45	6:45-7:45
PM																					
4:00-4:15	0	0		0	0	0	0	0	0	0	0	0	0	1	0	1	0	1	1		4:00-4:15
4:15-4:30	0	0		0	1	1	0	2	1	0	0	1	0	1	0	1	1	3	4		4:15-4:30
4:30-4:45	0			0	0	0	0	0	0	0	0	0	0	1	0	1	0	1	1		4:30-4:45
4:45-5:00 5:00-5:15					0	0			0	0	0		0	0	0	0		2	2		4:45-5:00 5:00-5:15
5:15-5:30	0				0	0				0			0	2	0	2		2	2		5:15-5:30
5:30-5:45	0			0	0	0		0	0	0	0	0	0	1	1	2	0	2	2		5:30-5:45
5:45-6:00	1			1	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1		5:45-6:00
6:00-6:15	0	0	C	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		6:00-6:15
6:15-6:30	1	0	C	1	0	0	0	0	0	0	1	1	0	0	0	0	2	0	2		6:15-6:30
6:30-6:45	0	0		0	•	0	0	0	•	0	0	0	0	0	0	0	0		0		6:30-6:45
6:45-7:00	0	C	C	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		6:45-7:00
3 Hour					1	4	^		1	^	4		•	0	1	^		44	15		
Totals 1 Hour	2	0	0	2	1	1	0	2	1	0	1	2	0	8	1	9	4	11	15		
Totals																					
1:00-5:00	0	0	C	o	1	1	0	2	1	0	n	1	0	5	0	5	1	7	8	0.50	4:00-5:00
4:15-5:15								2	1	0	0		0	4	0	4		, 6	7		4:15-5:15
1:30-5:30	0	0	_	_	-	0	0				0	0	0	5	0	5	0	5	5		4:30-5:30
1:45-5:45	0	O	C		0	0	0	0		0	0	0	0	5	1	6		6	6		4:45-5:4
5:00-6:00	1	C	C	1	0	0	0	0	0	0	0	0	0	3	1	4	1	4	5		5:00-6:00
5:15-6:15	1	C		1	0	0	0	0	0	0	0	0	0	3	1	4	1	4	5	0.63	5:15-6:1
5:30-6:30	2	0	C	2	0	0	0	0	0	0	1	1	0	1	1	2	3	2	5		5:30-6:30
5:45-6:45	2	0		2	0	0	0	0	0	0	1	1	0	0	0	0	3	0	3		5:45-6:4
6:00-7:00	1	0		1	0	0	0	0	0	0	1 	1 	0	0	0	0	2	0	2	0.25	6:00-7:00
PM Peak																					PM Peal
:00-5:00	0	0	C	0	1	1	0	2	1	0	0	1	0	5	0	5	1	1 7	8	0.50	4:00-5:0

B-12

Street: Burdette Road

Location: between Hillmeade Road and Bradley Boulevard

A study of vehicle traffic was conducted with the device having serial number 400358. The study was done in the NB 358 lane at Burdette Road in Potomac, MD in Montgomery County county. The study began on 05/18/2021 at 12:00 AM and concluded on 05/20/2021 at 12:00 AM, lasting a total of 48.00 hours. Traffic statistics were recorded in 15 minute time periods. The total recorded volume showed 1,422 vehicles passed through the location with a peak volume of 27 on 05/19/2021 at [01:45 PM-02:00 PM] and a minimum volume of 0 on 05/18/2021 at [09:45 PM-10:00 PM]. The AADT count for this study was 711.

SPEED

Chart 1 lists the values of the speed bins and the total traffic volume for each bin. At least half the vehicles were traveling in the 20 - 25 MPH range or lower. The average speed for all classifed vehicles was 21 MPH with 1.35% vehicles exceeding the posted speed of 35 MPH. 0.28% percent of the total vehicles were traveling in excess of 55 MPH. The mode speed for this traffic study was 20MPH and the 85th percentile was 26.35 MPH.

Γ	<	10	15	20	25	30	35	40	45	50	55	60	65	70	75
1	to	to	to	to	to	to	to	to	to	to	to	to	to	to	to
L	9	14	19	24	29	34	39	44	49	54	59	64	69	74	>
Г	77	73	356	628	230	25	6	3	3	3	0	0	1	0	3

CHART 1

CLASSIFICATION

Chart 2 lists the values of the classification bins and the total traffic volume accumulated for each bin.

Most of the vehicles classified during the study were Vans & Pickups. The number of Passenger Vehicles in the study was 628 which represents 45 percent of the total classified vehicles. The number of Vans & Pickups in the study was 657 which represents 47 percent of the total classified vehicles. The number of Busses & Trucks in the study was 87 which represents 6 percent of the total classified vehicles. The number of Tractor Trailers in the study was 36 which represents 3 percent of the total classified vehicles.

<	18	21	24	28	32	38	44				
to 17	to 20	to 23	to 27	to 31	to 37	to 43	to >				
628	432	225	65	16	12	14	16				

CHART 2

HEADWAY

During the peak traffic period, on 05/19/2021 at [01:45 PM-02:00 PM] the average headway between vehicles was 32.143 seconds. During the slowest traffic period, on 05/18/2021 at [09:45 PM-10:00 PM] the average headway between vehicles was 900 seconds.

WEATHER

The roadway surface temperature over the period of the study varied between 57.00 and 127.00 degrees F.

05/20/2021 01:06 AM Page: 1

B-13 D-97

Street: Burdette Road

Location: between Greentree Road and Bradley Boulevard

A study of vehicle traffic was conducted with the device having serial number 400366. The study was done in the NB 366 lane at Burdette Road in Potomac, MD in Montgomery County county. The study began on 05/18/2021 at 12:00 AM and concluded on 05/20/2021 at 12:00 AM, lasting a total of 48.00 hours. Traffic statistics were recorded in 15 minute time periods. The total recorded volume showed 1,598 vehicles passed through the location with a peak volume of 42 on 05/18/2021 at [08:15 AM-08:30 AM] and a minimum volume of 0 on 05/18/2021 at [10:00 PM-10:15 PM]. The AADT count for this study was 799.

SPEED

Chart 1 lists the values of the speed bins and the total traffic volume for each bin. At least half the vehicles were traveling in the 20 - 25 MPH range or lower. The average speed for all classifed vehicles was 20 MPH with 1.41% vehicles exceeding the posted speed of 35 MPH. 0.19% percent of the total vehicles were traveling in excess of 55 MPH. The mode speed for this traffic study was 20MPH and the 85th percentile was 27.61 MPH.

	<	10	15	20	25	30	35	40	45	50	55	60	65	70	75
	to	to	to	to	to	to	to	to	to	to	to	to	to	to	to
	9	14	19	24	29	34	39	44	49	54	59	64	69	74	>
Г	229	68	267	598	299	75	10	5	1	3	1	0	1	0	1

CHART 1

CLASSIFICATION

Chart 2 lists the values of the classification bins and the total traffic volume accumulated for each bin.

Most of the vehicles classified during the study were Passenger Vehicles. The number of Passenger Vehicles in the study was 639 which represents 41 percent of the total classified vehicles. The number of Vans & Pickups in the study was 631 which represents 41 percent of the total classified vehicles. The number of Busses & Trucks in the study was 232 which represents 15 percent of the total classified vehicles. The number of Tractor Trailers in the study was 49 which represents 3 percent of the total classified vehicles.

<	18	21	24	28	32	38	44				
to 17	to 20	to 23	to 27	to 31	to 37	to 43	to >				
639	418	213	119	73	52	10	34				

CHART 2

HEADWAY

During the peak traffic period, on 05/18/2021 at [08:15 AM-08:30 AM] the average headway between vehicles was 20.93 seconds. During the slowest traffic period, on 05/18/2021 at [10:00 PM-10:15 PM] the average headway between vehicles was 900 seconds.

WEATHER

The roadway surface temperature over the period of the study varied between 55.00 and 90.00 degrees F.

05/20/2021 01:03 AM Page: 1

B-14 D-98

Street: Bradley Boulevard
Location: between Burdette Road & Fernwood Road

A study of vehicle traffic was conducted with the device having serial number 400588. The study was done in the EB 588 lane at Bradley Boulevard in Potomac, MD in Montgomery County county. The study began on 05/18/2021 at 12:00 AM and concluded on 05/20/2021 at 12:00 AM, lasting a total of 48.00 hours. Traffic statistics were recorded in 15 minute time periods. The total recorded volume showed 7,767 vehicles passed through the location with a peak volume of 137 on 05/18/2021 at [08:00 AM-08:15 AM] and a minimum volume of 0 on 05/18/2021 at [12:00 AM-12:15 AM]. The AADT count for this study was 3,884.

SPEED

Chart 1 lists the values of the speed bins and the total traffic volume for each bin. At least half the vehicles were traveling in the 35 - 40 MPH range or lower. The average speed for all classifed vehicles was 34 MPH with 50.70% vehicles exceeding the posted speed of 35 MPH. 0.75% percent of the total vehicles were traveling in excess of 55 MPH. The mode speed for this traffic study was 35MPH and the 85th percentile was 41.63 MPH.

Γ	<	10	15	20	25	30	35	40	45	50	55	60	65	70	75
	to	to	to	to	to	to	to	to	to	to	to	to	to	to	to
	9	14	19	24	29	34	39	44	49	54	59	64	69	74	>
Γ	311	52	124	387	861	2083	2395	1093	297	83	27	14	8	2	7

CHART 1

CLASSIFICATION

Chart 2 lists the values of the classification bins and the total traffic volume accumulated for each bin.

Most of the vehicles classified during the study were Vans & Pickups. The number of Passenger Vehicles in the study was 3308 which represents 43 percent of the total classified vehicles. The number of Vans & Pickups in the study was 3778 which represents 49 percent of the total classified vehicles. The number of Busses & Trucks in the study was 499 which represents 6 percent of the total classified vehicles. The number of Tractor Trailers in the study was 145 which represents 2 percent of the total classified vehicles.

<	18	21	24	28	32	38	44				
to 17	to 20	to 23	to 27	to 31	to 37	to 43	to >				
3308	2442	1336	386	76	49	80	67				

CHART 2

HEADWAY

During the peak traffic period, on 05/18/2021 at [08:00 AM-08:15 AM] the average headway between vehicles was 6.522 seconds. During the slowest traffic period, on 05/18/2021 at [12:00 AM-12:15 AM] the average headway between vehicles was 900 seconds.

WEATHER

The roadway surface temperature over the period of the study varied between 55.00 and 115.00 degrees F.

05/20/2021 01:06 AM Page:

B-15 D-99

Street: Bradley Boulevard
Location: between Burdette Road & Fernwood Road

A study of vehicle traffic was conducted with the device having serial number 400589. The study was done in the WB 589 lane at Bradley Boulevard in Potomac, MD in Montgomery County county. The study began on 05/18/2021 at 12:00 AM and concluded on 05/20/2021 at 12:00 AM, lasting a total of 48.00 hours. Traffic statistics were recorded in 15 minute time periods. The total recorded volume showed 7,415 vehicles

passed through the location with a peak volume of 125 on 05/18/2021 at [03:15 PM-03:30 PM] and a minimum volume of 0 on 05/18/2021 at [12:15 AM-12:30 AM]. The AADT count for this study was 3,708.

SPEED

Chart 1 lists the values of the speed bins and the total traffic volume for each bin. At least half the vehicles were traveling in the 30 - 35 MPH range or lower. The average speed for all classifed vehicles was 35 MPH with 44.51% vehicles exceeding the posted speed of 35 MPH. 3.53% percent of the total vehicles were traveling in excess of 55 MPH. The mode speed for this traffic study was 30MPH and the 85th percentile was 40.94 MPH.

<	10	15	20	25	30	35	40	45	50	55	60	65	70	75
to	to	to	to	to	to	to	to	to	to	to	to	to	to	to
9	14	19	24	29	34	39	44	49	54	59	64	69	74	>
269	99	88	406	1049	2101	1888	743	224	108	50	49	34	28	94

CHART 1

CLASSIFICATION

Chart 2 lists the values of the classification bins and the total traffic volume accumulated for each bin.

Most of the vehicles classified during the study were Vans & Pickups. The number of Passenger Vehicles in the study was 2824 which represents 40 percent of the total classified vehicles. The number of Vans & Pickups in the study was 2894 which represents 41 percent of the total classified vehicles. The number of Busses & Trucks in the study was 995 which represents 14 percent of the total classified vehicles. The number of Tractor Trailers in the study was 392 which represents 6 percent of the total classified vehicles.

Γ	'	18	21	24	28	32	38	44				
	to 17	to 20	to 23	to 27	to 31	to 37	to 43	to >				
\vdash	2824	1883	1011	534	295	217	154	312				

CHART 2

HEADWAY

During the peak traffic period, on 05/18/2021 at [03:15 PM-03:30 PM] the average headway between vehicles was 7.143 seconds. During the slowest traffic period, on 05/18/2021 at [12:15 AM-12:30 AM] the average headway between vehicles was 900 seconds.

WEATHER

The roadway surface temperature over the period of the study varied between 55.00 and 122.00 degrees F.

05/20/2021 01:02 AM Page: 1

B-16 D-100

Street: Burdette Road

Location: between Hillmeade Road and Bradley Boulevard

A study of vehicle traffic was conducted with the device having serial number 400641. The study was done in the SB 641 lane at Burdette Road in Potomac, MD in Montgomery County county. The study began on 05/18/2021 at 12:00 AM and concluded on 05/20/2021 at 12:00 AM, lasting a total of 48.00 hours. Traffic statistics were recorded in 15 minute time periods. The total recorded volume showed 1,580 vehicles passed through the location with a peak volume of 37 on 05/18/2021 at [08:00 AM-08:15 AM] and a minimum volume of 0 on 05/18/2021 at [09:00 PM-09:15 PM]. The AADT count for this study was 790.

SPEED

Chart 1 lists the values of the speed bins and the total traffic volume for each bin. At least half the vehicles were traveling in the 20 - 25 MPH range or lower. The average speed for all classifed vehicles was 24 MPH with 1.53% vehicles exceeding the posted speed of 35 MPH. 0.13% percent of the total vehicles were traveling in excess of 55 MPH. The mode speed for this traffic study was 20MPH and the 85th percentile was 28.59 MPH.

ſ	<	10	15	20	25	30	35	40	45	50	55	60	65	70	75
	to	to	to	to	to	to	to	to	to	to	to	to	to	to	to
L	9	14	19	24	29	34	39	44	49	54	59	64	69	74	>
	47	20	183	721	509	68	15	3	2	2	0	1	0	0	1

CHART 1

CLASSIFICATION

Chart 2 lists the values of the classification bins and the total traffic volume accumulated for each bin.

Most of the vehicles classified during the study were Passenger Vehicles. The number of Passenger Vehicles in the study was 909 which represents 58 percent of the total classified vehicles. The number of Vans & Pickups in the study was 575 which represents 37 percent of the total classified vehicles. The number of Busses & Trucks in the study was 58 which represents 4 percent of the total classified vehicles. The number of Tractor Trailers in the study was 30 which represents 2 percent of the total classified vehicles.

ſ	<	18	21	24	28	32	38	44				
	to 17	to 20	to 23	to 27	to 31	to 37	to 43	to >				
ſ	909	423	152	38	9	17	16	8				

CHART 2

HEADWAY

During the peak traffic period, on 05/18/2021 at [08:00 AM-08:15 AM] the average headway between vehicles was 23.684 seconds. During the slowest traffic period, on 05/18/2021 at [09:00 PM-09:15 PM] the average headway between vehicles was 900 seconds.

WEATHER

The roadway surface temperature over the period of the study varied between 55.00 and 115.00 degrees F.

05/20/2021 01:01 AM Page: 1

B-17 D-101

Street: Bradley Boulevard
Location: between Burdette Road & Fernwood Road

A study of vehicle traffic was conducted with the device having serial number 400875. The study was done in the WB 875 lane at Bradley Boulevard in Potomac, MD in Montgomery County county. The study began on 05/18/2021 at 12:00 AM and concluded on 05/20/2021 at 12:00 AM, lasting a total of 48.00 hours. Traffic statistics were recorded in 15 minute time periods. The total recorded volume showed 7,493 vehicles passed through the location with a peak volume of 125 on 05/18/2021 at [03:15 PM-03:30 PM] and a minimum volume of 0 on 05/18/2021 at [12:15 AM-12:30 AM]. The AADT count for this study was 3,747.

SPEED

Chart 1 lists the values of the speed bins and the total traffic volume for each bin. At least half the vehicles were traveling in the 30 - 35 MPH range or lower. The average speed for all classifed vehicles was 33 MPH with 33.61% vehicles exceeding the posted speed of 35 MPH. 2.01% percent of the total vehicles were traveling in excess of 55 MPH. The mode speed for this traffic study was 30MPH and the 85th percentile was 38.98 MPH.

<	10	15	20	25	30	35	40	45	50	55	60	65	70	75
to	to	to	to	to	to	to	to	to	to	to	to	to	to	to
9	14	19	24	29	34	39	44	49	54	59	64	69	74	>
304	106	117	501	1268	2636	1659	497	137	55	41	24	17	15	52

CHART 1

CLASSIFICATION

Chart 2 lists the values of the classification bins and the total traffic volume accumulated for each bin.

Most of the vehicles classified during the study were Passenger Vehicles. The number of Passenger Vehicles in the study was 3026 which represents 41 percent of the total classified vehicles. The number of Vans & Pickups in the study was 2874 which represents 39 percent of the total classified vehicles. The number of Busses & Trucks in the study was 1088 which represents 15 percent of the total classified vehicles. The number of Tractor Trailers in the study was 368 which represents 5 percent of the total classified vehicles.

Γ	<	18	21	24	28	32	38	44				
	to 17	to 20	to 23	to 27	to 31	to 37	to 43	to >				
r	3026	1921	953	521	335	306	149	218				

CHART 2

HEADWAY

During the peak traffic period, on 05/18/2021 at [03:15 PM-03:30 PM] the average headway between vehicles was 7.143 seconds. During the slowest traffic period, on 05/18/2021 at [12:15 AM-12:30 AM] the average headway between vehicles was 900 seconds.

WEATHER

The roadway surface temperature over the period of the study varied between 52.00 and 120.00 degrees F.

05/20/2021 01:04 AM Page: 1

B-18 D-102

Street: Bradley Boulevard
Location: Between Beltway and Redwood Avenue

A study of vehicle traffic was conducted with the device having serial number 136064. The study was done in the EB 6064 lane at Bradley Boulevard in Potomac, MD in Montgomery County county. The study began on 05/18/2021 at 12:00 AM and concluded on 05/20/2021 at 12:00 AM, lasting a total of 48.00 hours. Traffic statistics were recorded in 15 minute time periods. The total recorded volume showed 7,513 vehicles passed through the location with a peak volume of 174 on 05/18/2021 at [08:00 AM-08:15 AM] and a minimum volume of 0 on 05/18/2021 at [01:00 AM-01:15 AM]. The AADT count for this study was 3,757.

SPEED

Chart 1 lists the values of the speed bins and the total traffic volume for each bin. At least half the vehicles were traveling in the 35 - 40 MPH range or lower. The average speed for all classifed vehicles was 40 MPH with 75.80% vehicles exceeding the posted speed of 35 MPH. 6.20% percent of the total vehicles were traveling in excess of 55 MPH. The mode speed for this traffic study was 35MPH and the 85th percentile was 47.18 MPH.

Γ	<	10	15	20	25	30	35	40	45	50	55	60	65	70	75
	to	to	to	to	to	to	to	to	to	to	to	to	to	to	to
	9	14	19	24	29	34	39	44	49	54	59	64	69	74	>
ſ	0	19	67	88	308	1246	2388	1653	685	244	125	71	64	54	129

CHART 1

CLASSIFICATION

Chart 2 lists the values of the classification bins and the total traffic volume accumulated for each bin.

Most of the vehicles classified during the study were Passenger Vehicles. The number of Passenger Vehicles in the study was 5382 which represents 75 percent of the total classified vehicles. The number of Vans & Pickups in the study was 1271 which represents 18 percent of the total classified vehicles. The number of Busses & Trucks in the study was 374 which represents 5 percent of the total classified vehicles. The number of Tractor Trailers in the study was 111 which represents 2 percent of the total classified vehicles.

<	18	21	24	28	32	38	44				
to 17	to 20	to 23	to 27	to 31	to 37	to 43	to >				
5382	960	311	237	75	90	52	34				

CHART 2

HEADWAY

During the peak traffic period, on 05/18/2021 at [08:00 AM-08:15 AM] the average headway between vehicles was 5.143 seconds. During the slowest traffic period, on 05/18/2021 at [01:00 AM-01:15 AM] the average headway between vehicles was 900 seconds.

WEATHER

The roadway surface temperature over the period of the study varied between 61.00 and 129.00 degrees F.

05/20/2021 01:00 AM Page: 1

B-19 D-103

Street: Bradley Boulevard
Location: Between Beltway and Redwood Avenue

A study of vehicle traffic was conducted with the device having serial number 122569. The study was done in the Wb 2569 lane at Bradley Boulevard in Potomac, MD in Montgomery County county. The study began on 05/18/2021 at 12:00 AM and concluded on 05/20/2021 at 12:00 AM, lasting a total of 48.00 hours. Traffic statistics were recorded in 15 minute time periods. The total recorded volume showed 7,144 vehicles passed through the location with a peak volume of 129 on 05/18/2021 at [03:15 PM-03:30 PM] and a minimum volume of 0 on 05/18/2021 at [01:15 AM-01:30 AM]. The AADT count for this study was 3,572.

SPEED

Chart 1 lists the values of the speed bins and the total traffic volume for each bin. At least half the vehicles were traveling in the 35 - 40 MPH range or lower. The average speed for all classifed vehicles was 40 MPH with 74.89% vehicles exceeding the posted speed of 35 MPH. 6.48% percent of the total vehicles were traveling in excess of 55 MPH. The mode speed for this traffic study was 35MPH and the 85th percentile was 47.86 MPH.

<	10	15	20	25	30	35	40	45	50	55	60	65	70	75
to	to	to	to	to	to	to	to	to	to	to	to	to	to	to
9	14	19	24	29	34	39	44	49	54	59	64	69	74	>
2	34	58	141	331	1151	2031	1644	733	270	139	90	48	31	135

CHART 1

CLASSIFICATION

Chart 2 lists the values of the classification bins and the total traffic volume accumulated for each bin.

Most of the vehicles classified during the study were Passenger Vehicles. The number of Passenger Vehicles in the study was 4087 which represents 60 percent of the total classified vehicles. The number of Vans & Pickups in the study was 1974 which represents 29 percent of the total classified vehicles. The number of Busses & Trucks in the study was 621 which represents 9 percent of the total classified vehicles. The number of Tractor Trailers in the study was 156 which represents 2 percent of the total classified vehicles.

<	18	21	24	28	32	38	44				
to 17	to 20	to 23	to 27	to 31	to 37	to 43	to >				
4087	1341	633	389	152	112	78	46				

CHART 2

HEADWAY

During the peak traffic period, on 05/18/2021 at [03:15 PM-03:30 PM] the average headway between vehicles was 6.923 seconds. During the slowest traffic period, on 05/18/2021 at [01:15 AM-01:30 AM] the average headway between vehicles was 900 seconds.

WEATHER

The roadway surface temperature over the period of the study varied between 61.00 and 129.00 degrees F.

05/20/2021 01:00 AM Page: 1

B-20 D-104

Street: Burdette Road

Location: between Hillmeade Road and Bradley Boulevard

A study of vehicle traffic was conducted with the device having serial number 401264. The study was done in the SB 1264 lane at Burdette Road in Potomac, MD in Montgomery County county. The study began on 05/18/2021 at 12:00 AM and concluded on 05/20/2021 at 12:00 AM, lasting a total of 48.00 hours. Traffic statistics were recorded in 15 minute time periods. The total recorded volume showed 1,574 vehicles passed through the location with a peak volume of 37 on 05/18/2021 at [08:00 AM-08:15 AM] and a minimum volume of 0 on 05/18/2021 at [09:00 PM-09:15 PM]. The AADT count for this study was 787.

SPEED

Chart 1 lists the values of the speed bins and the total traffic volume for each bin. At least half the vehicles were traveling in the 20 - 25 MPH range or lower. The average speed for all classifed vehicles was 24 MPH with 1.72% vehicles exceeding the posted speed of 35 MPH. 0.13% percent of the total vehicles were traveling in excess of 55 MPH. The mode speed for this traffic study was 20MPH and the 85th percentile was 28.74 MPH.

	۲ >	10	15 to	20	25	30	35 to	40	45 to	50	55 to	60 to	65 to	70	75
	to 9	to 14	to 19	to 24	to 29	to 34	to 39	to 44	to 49	to 54	to 59	to 64	to 69	to 74	to >
Ī	48	21	198	673	530	75	14	9	2	0	1	0	0	0	1

CHART 1

CLASSIFICATION

Chart 2 lists the values of the classification bins and the total traffic volume accumulated for each bin.

Most of the vehicles classified during the study were Passenger Vehicles. The number of Passenger Vehicles in the study was 776 which represents 49 percent of the total classified vehicles. The number of Vans & Pickups in the study was 708 which represents 45 percent of the total classified vehicles. The number of Busses & Trucks in the study was 60 which represents 4 percent of the total classified vehicles. The number of Tractor Trailers in the study was 28 which represents 2 percent of the total classified vehicles.

<	18	21	24	28	32	38	44				
to 17	to 20	to 23	to 27	to 31	to 37	to 43	to >				
776	510	198	31	20	12	10	15				

CHART 2

HEADWAY

During the peak traffic period, on 05/18/2021 at [08:00 AM-08:15 AM] the average headway between vehicles was 23.684 seconds. During the slowest traffic period, on 05/18/2021 at [09:00 PM-09:15 PM] the average headway between vehicles was 900 seconds.

WEATHER

The roadway surface temperature over the period of the study varied between 59.00 and 102.00 degrees F.

05/20/2021 01:07 AM Page: 1

B-21 D-105

Street: Bradley Boulevard
Location: Between Beltway and Redwood Avenue

A study of vehicle traffic was conducted with the device having serial number 401263. The study was done in the EB 1263 lane at Bradley Boulevard in Potomac, MD in Montgomery County county. The study began on 05/18/2021 at 12:00 AM and concluded on 05/20/2021 at 12:00 AM, lasting a total of 48.00 hours. Traffic statistics were recorded in 15 minute time periods. The total recorded volume showed 7,653 vehicles passed through the location with a peak volume of 175 on 05/18/2021 at [08:00 AM-08:15 AM] and a minimum volume of 0 on 05/18/2021 at [01:00 AM-01:15 AM]. The AADT count for this study was 3,827.

SPEED

Chart 1 lists the values of the speed bins and the total traffic volume for each bin. At least half the vehicles were traveling in the 40 - 45 MPH range or lower. The average speed for all classifed vehicles was 41 MPH with 85.26% vehicles exceeding the posted speed of 35 MPH. 6.86% percent of the total vehicles were traveling in excess of 55 MPH. The mode speed for this traffic study was 40MPH and the 85th percentile was 48.75 MPH.

Г	<	10	15	20	25	30	35	40	45	50	55	60	65	70	75
П	to	to	to	to	to	to	to	to	to	to	to	to	to	to	to
	9	14	19	24	29	34	39	44	49	54	59	64	69	74	>
Г	216	76	52	65	100	594	2048	2347	1099	373	179	96	59	43	136

CHART 1

CLASSIFICATION

Chart 2 lists the values of the classification bins and the total traffic volume accumulated for each bin.

Most of the vehicles classified during the study were Passenger Vehicles. The number of Passenger Vehicles in the study was 4218 which represents 57 percent of the total classified vehicles. The number of Vans & Pickups in the study was 2526 which represents 34 percent of the total classified vehicles. The number of Busses & Trucks in the study was 495 which represents 7 percent of the total classified vehicles. The number of Tractor Trailers in the study was 199 which represents 3 percent of the total classified vehicles.

Γ	<	18	21	24	28	32	38	44				
	to 17	to 20	to 23	to 27	to 31	to 37	to 43	to >				
Γ	4218	1834	692	287	121	116	88	127				

CHART 2

HEADWAY

During the peak traffic period, on 05/18/2021 at [08:00 AM-08:15 AM] the average headway between vehicles was 5.114 seconds. During the slowest traffic period, on 05/18/2021 at [01:00 AM-01:15 AM] the average headway between vehicles was 900 seconds.

WEATHER

The roadway surface temperature over the period of the study varied between 59.00 and 124.00 degrees F.

05/20/2021 01:09 AM Page: 1

B-22 D-106

Street: Burdette Road

Location: between Greentree Road and Bradley Boulevard

A study of vehicle traffic was conducted with the device having serial number 401212. The study was done in the SB 1212 lane at Burdette Road in Potomac, MD in Montgomery County county. The study began on 05/18/2021 at 12:00 AM and concluded on 05/20/2021 at 12:00 AM, lasting a total of 48.00 hours. Traffic statistics were recorded in 15 minute time periods. The total recorded volume showed 1,145 vehicles passed through the location with a peak volume of 50 on 05/18/2021 at [03:00 PM-03:15 PM] and a minimum volume of 0 on 05/18/2021 at [09:45 PM-10:00 PM]. The AADT count for this study was 573.

SPEED

Chart 1 lists the values of the speed bins and the total traffic volume for each bin. At least half the vehicles were traveling in the 20 - 25 MPH range or lower. The average speed for all classifed vehicles was 21 MPH with 0.70% vehicles exceeding the posted speed of 35 MPH. 0.09% percent of the total vehicles were traveling in excess of 55 MPH. The mode speed for this traffic study was 20MPH and the 85th percentile was 25.65 MPH.

<	10	15	20	25	30	35	40	45	50	55	60	65	70	75
to	to	to	to	to	to	to	to	to	to	to	to	to	to	to
9	14	19	24	29	34	39	44	49	54	59	64	69	74	>
63	37	362	488	153	31	4	2	1	0	0	0	0	0	1

CHART 1

CLASSIFICATION

Chart 2 lists the values of the classification bins and the total traffic volume accumulated for each bin.

Most of the vehicles classified during the study were Passenger Vehicles. The number of Passenger Vehicles in the study was 569 which represents 50 percent of the total classified vehicles. The number of Vans & Pickups in the study was 419 which represents 37 percent of the total classified vehicles. The number of Busses & Trucks in the study was 134 which represents 12 percent of the total classified vehicles. The number of Tractor Trailers in the study was 20 which represents 2 percent of the total classified vehicles.

<	18	21	24	28	32	38	44				
to 17	to 20	to 23	to 27	to 31	to 37	to 43	to >				
569	297	122	74	45	19	7	9				

CHART 2

HEADWAY

During the peak traffic period, on 05/18/2021 at [03:00 PM-03:15 PM] the average headway between vehicles was 17.647 seconds. During the slowest traffic period, on 05/18/2021 at [09:45 PM-10:00 PM] the average headway between vehicles was 900 seconds.

WEATHER

The roadway surface temperature over the period of the study varied between 32.00 and 90.00 degrees F.

05/20/2021 01:11 AM Page: 1

B-23 D-107

Street: Bradley Boulevard
Location: Between Beltway and Redwood Avenue

A study of vehicle traffic was conducted with the device having serial number 401202. The study was done in the WB1202 lane at Bradley Boulevard in Potomac, MD in Montgomery County county. The study began on 05/18/2021 at 12:00 AM and concluded on 05/20/2021 at 12:00 AM, lasting a total of 48.00 hours. Traffic statistics were recorded in 15 minute time periods. The total recorded volume showed 7,148 vehicles passed through the location with a peak volume of 127 on 05/18/2021 at [03:15 PM-03:30 PM] and a minimum volume of 0 on 05/18/2021 at [01:15 AM-01:30 AM]. The AADT count for this study was 3,574.

SPEED

Chart 1 lists the values of the speed bins and the total traffic volume for each bin. At least half the vehicles were traveling in the 40 - 45 MPH range or lower. The average speed for all classifed vehicles was 45 MPH with 88.81% vehicles exceeding the posted speed of 35 MPH. 9.93% percent of the total vehicles were traveling in excess of 55 MPH. The mode speed for this traffic study was 40MPH and the 85th percentile was 52.25 MPH.

<	10	15	20	25	30	35	40	45	50	55	60	65	70	75
to	to	to	to	to	to	to	to	to	to	to	to	to	to	to
9	14	19	24	29	34	39	44	49	54	59	64	69	74	>
129	74	46	46	118	369	1048	1982	1737	743	258	146	82	63	145

CHART 1

CLASSIFICATION

Chart 2 lists the values of the classification bins and the total traffic volume accumulated for each bin.

Most of the vehicles classified during the study were Vans & Pickups. The number of Passenger Vehicles in the study was 1969 which represents 28 percent of the total classified vehicles. The number of Vans & Pickups in the study was 3531 which represents 51 percent of the total classified vehicles. The number of Busses & Trucks in the study was 1143 which represents 17 percent of the total classified vehicles. The number of Tractor Trailers in the study was 279 which represents 4 percent of the total classified vehicles.

<	18	21	24	28	32	38	44				
to 17	to 20	to 23	to 27	to 31	to 37	to 43	to >				
1969	1920	1611	785	224	179	86	212				

CHART 2

HEADWAY

During the peak traffic period, on 05/18/2021 at [03:15 PM-03:30 PM] the average headway between vehicles was 7.031 seconds. During the slowest traffic period, on 05/18/2021 at [01:15 AM-01:30 AM] the average headway between vehicles was 900 seconds.

WEATHER

The roadway surface temperature over the period of the study varied between 57.00 and 120.00 degrees F.

05/20/2021 01:03 AM Page: 1

B-24 D-108

MH Corbin Traffic Analyzer Study Computer Generated Summary Report City: Potomac

Street: Burdette Road

Location: between Greentree Road and Bradley Boulevard

A study of vehicle traffic was conducted with the device having serial number 401134. The study was done in the SB 1134 lane at Burdette Road in Potomac, MD in Montgomery County county. The study began on 05/18/2021 at 12:00 AM and concluded on 05/20/2021 at 12:00 AM, lasting a total of 48.00 hours. Traffic statistics were recorded in 15 minute time periods. The total recorded volume showed 1,711 vehicles passed through the location with a peak volume of 49 on 05/18/2021 at [03:00 PM-03:15 PM] and a minimum volume of 0 on 05/18/2021 at [09:45 PM-10:00 PM]. The AADT count for this study was 856.

SPEED

Chart 1 lists the values of the speed bins and the total traffic volume for each bin. At least half the vehicles were traveling in the 20 - 25 MPH range or lower. The average speed for all classifed vehicles was 22 MPH with 1.11% vehicles exceeding the posted speed of 35 MPH. 0.18% percent of the total vehicles were traveling in excess of 55 MPH. The mode speed for this traffic study was 20MPH and the 85th percentile was 27.31 MPH.

Г	<	10	15	20	25	30	35	40	45	50	55	60	65	70	75
	to	to	to	to	to	to	to	to	to	to	to	to	to	to	to
	9	14	19	24	29	34	39	44	49	54	59	64	69	74	>
	88	62	435	701	359	46	12	3	0	1	1	0	0	0	2

CHART 1

CLASSIFICATION

Chart 2 lists the values of the classification bins and the total traffic volume accumulated for each bin.

Most of the vehicles classified during the study were Passenger Vehicles. The number of Passenger Vehicles in the study was 855 which represents 50 percent of the total classified vehicles. The number of Vans & Pickups in the study was 766 which represents 45 percent of the total classified vehicles. The number of Busses & Trucks in the study was 64 which represents 4 percent of the total classified vehicles. The number of Tractor Trailers in the study was 24 which represents 1 percent of the total classified vehicles.

<	18	21	24	28	32	38	44				
to 17	to 20	to 23	to 27	to 31	to 37	to 43	to >				
855	564	202	42	11	18	10	8				

CHART 2

HEADWAY

During the peak traffic period, on 05/18/2021 at [03:00 PM-03:15 PM] the average headway between vehicles was 18 seconds. During the slowest traffic period, on 05/18/2021 at [09:45 PM-10:00 PM] the average headway between vehicles was 900 seconds.

WEATHER

The roadway surface temperature over the period of the study varied between 57.00 and 82.00 degrees F.

05/20/2021 01:08 AM Page: 1

B-25 D-109

MH Corbin Traffic Analyzer Study Computer Generated Summary Report City: Potomac

Street: Burdette Road

Location: between Greentree Road and Bradley Boulevard

A study of vehicle traffic was conducted with the device having serial number 400876. The study was done in the NB 876 lane at Burdette Road in Potomac, MD in Montgomery County county. The study began on 05/18/2021 at 12:00 AM and concluded on 05/20/2021 at 12:00 AM, lasting a total of 48.00 hours. Traffic statistics were recorded in 15 minute time periods. The total recorded volume showed 1,597 vehicles passed through the location with a peak volume of 42 on 05/18/2021 at [08:15 AM-08:30 AM] and a minimum volume of 0 on 05/18/2021 at [10:00 PM-10:15 PM]. The AADT count for this study was 799.

SPEED

Chart 1 lists the values of the speed bins and the total traffic volume for each bin. At least half the vehicles were traveling in the 20 - 25 MPH range or lower. The average speed for all classifed vehicles was 22 MPH with 1.19% vehicles exceeding the posted speed of 35 MPH. 0.50% percent of the total vehicles were traveling in excess of 55 MPH. The mode speed for this traffic study was 20MPH and the 85th percentile was 27.29 MPH.

٧	10	15	20	25	30	35	40	45	50	55	60	65	70	75
to 9	to 14	to 19	to 24	to 29	to 34	to 39	to 44	to 49	to 54	to 59	to 64	to 69	to 74	to >
75	62	330	752	293	63	9	2	0	0	2	0	2	1	3

CHART 1

CLASSIFICATION

Chart 2 lists the values of the classification bins and the total traffic volume accumulated for each bin.

Most of the vehicles classified during the study were Vans & Pickups. The number of Passenger Vehicles in the study was 710 which represents 45 percent of the total classified vehicles. The number of Vans & Pickups in the study was 726 which represents 46 percent of the total classified vehicles. The number of Busses & Trucks in the study was 126 which represents 8 percent of the total classified vehicles. The number of Tractor Trailers in the study was 30 which represents 2 percent of the total classified vehicles.

<	18	21	24	28	32	38	44				
to 17	to 20	to 23	to 27	to 31	to 37	to 43	to >				
710	487	239	92	27	10	14	15				

CHART 2

HEADWAY

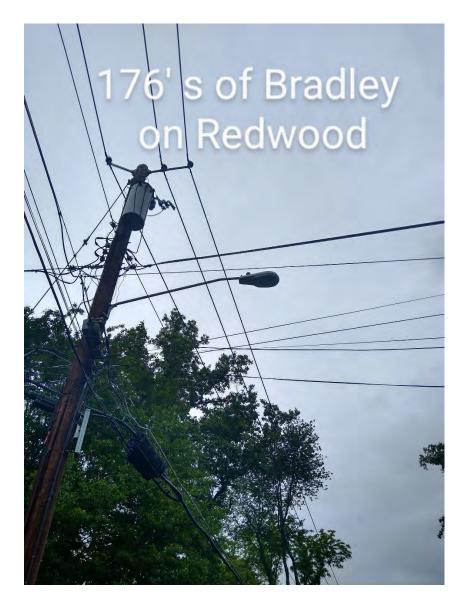
During the peak traffic period, on 05/18/2021 at [08:15 AM-08:30 AM] the average headway between vehicles was 20.93 seconds. During the slowest traffic period, on 05/18/2021 at [10:00 PM-10:15 PM] the average headway between vehicles was 900 seconds.

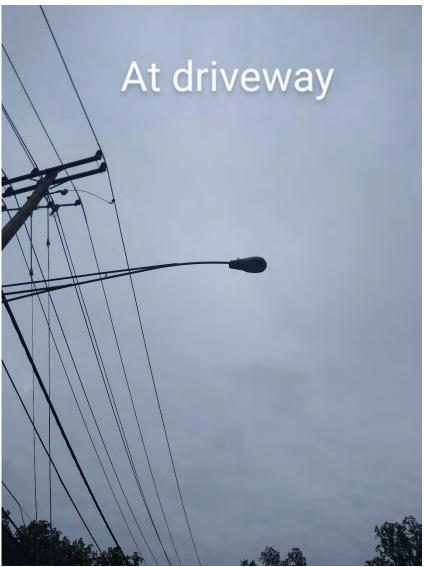
WEATHER

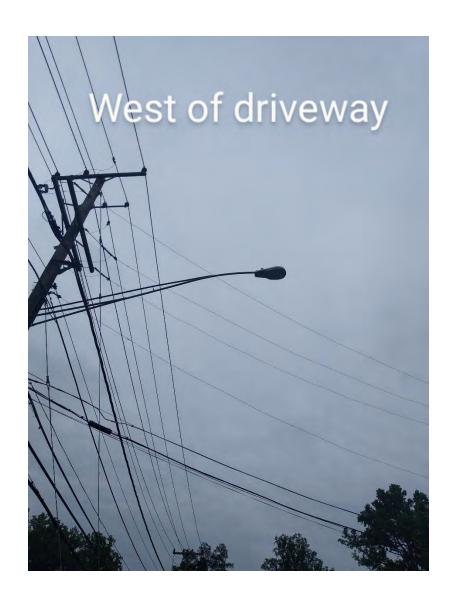
The roadway surface temperature over the period of the study varied between 57.00 and 95.00 degrees F.

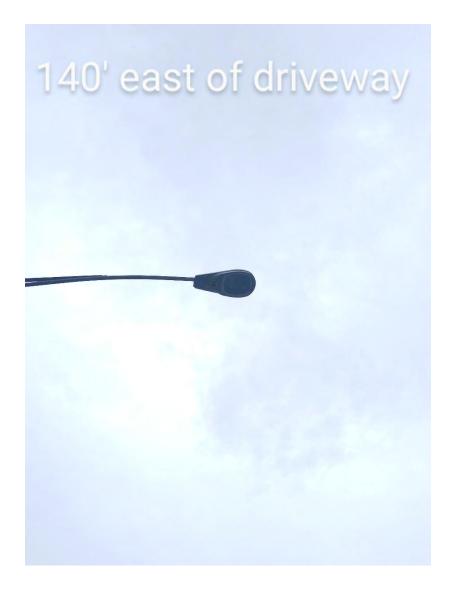
05/20/2021 01:02 AM Page: 1

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B-29 D-113









B-30 D-114









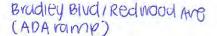








B-33 D-117



Montgomery Planning

PEDESTRIAN LEVEL OF COMFORT METHODOLOGY, VERSION 1.2

Pathway Condition

Research indicates that pathway condition affects pedestrian comfort and this variable is included in other leading pedestrian comfort indices. ^{7 8 9} Montgomery County is currently collecting information about pathway condition throughout the county including cross slope (helps drain water and prevent pooling), tripping hazards, cracks, severe spalling (surface peeling or cracking of concrete), obstructions (to be accounted for in the Accessibility Evaluation), and missing sections. A sample survey is provided below.

			нот	BUTTONS			
Ra	imps	Path	nways		Crosswalks	Bus par	ds
Detectable Warning Surface (DWS)	(Yes/No	Surface Type	Concrete; Asphalt or Other	Slope		Minimum Size (5' x 8')	Yes/No
DWS Type	Cast in Place, Nail Down or Other	Width	In Feet	Marking Type	Solid, Standard, Continental, Dashed, Zebra, Ladder, None	Bus Stop Connected to Pathway (100' of pathway or nearest intersection)	Yes/No
DWS Color	(Red, Yellow, Gray or Other	Cross Slope (2% or less but greater than 0)	Yes (No)	Centered with Ramp	Yes/No	Midblock	Yes/No
DWS Size	In Feet	Trip Hazard 1/4" or greater	Yes/No)	Pedestrian Signal	Yes/No		
Ramp Width	In Feet	Multiple Cracks in one section	Yes/No	Pushbutton	Yes/No		
Ramp Slope (8.33% or less)	Yes/No	Severe Spalling	Yes/No	Pushbutton	Heights, Distance from Pathway, Raised Tactile, Tone, Audible Indication, Actuated Indicator		
Ramp Landing area (2% or less)	Yes/No	Obstructions (less than 36" opening)	Yes No				
Ramp Landing Area (5' x 5')	Yes/No	Obstruction Type	Utility, Vegetation, Sign or Other				
		Missing Sections Lengths	In Feet				

Montgomery County Pathway Condition Survey

B-34

⁷ Clifton, Kelly J., Andrea D. Livi Smith, and Daniel Rodriguez. 2007. "The development and testing of an audit for the pedestrian environment." Landscape and Urban Planning; 95-110.

⁸ San Francisco Department of Public Health, 2012.

⁹ Oregon DOT. 2018. "Multimodal Analysis." Chap. 14 in Analysis Procedure Manual.

Higher Ground Education, Inc July 2021

APPENDIX C SIGNAL TIMING WORKSHEETS

Page 1 May 5, 2021

PHASE IN U	JS	E/F	PEI	D_												
PHASE	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
IN USE		Х		Х		Х		Х								
EXCLUSIVE PED																

am pm

PHASE	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
MIN GRN	0	10	0	10	0	10	0	10	0	0	0	0	0	0	0	0
BK MGRN	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
CS MGRN	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
DLY GRN	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
WALK	0	0	0	7	0	0	0	0	0	0	0	0	0	0	0	0
WALK2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
WLK MAX	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
PED CLR	0	0	0	10	0	0	0	0	0	0	0	0	0	0	0	0
PD CLR2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
PC MAX	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
PED CO	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
VEH EXT	0.0	3.0	0.0	3.0	0.0	3.0	0.0	3.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
VH EXT2	0.0	4.0	0.0	4.0	0.0	4.0	0.0	4.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
MAX1	0	50	0	30	0	50	0	30	0	0	0	0	0	0	0	0
MAX2	0	50	0	60	0	50	0	60	0	0	0	0	0	0	0	0
MAX3	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
DYM MAX	0	70	0	60	0	70	0	60	0	0	0	0	0	0	0	0
DYM STP	0.0	20.0	0.0	15.0	0.0	20.0	0.0	15.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
YELLOW	0.0	4.0	0.0	4.0	0.0	4.0	0.0	4.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
RED CLR	0.0	1.0	0.0	1.5	0.0	1.0	0.0	1.5	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
RED MAX	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
RED RVT	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
ACT B4	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
SEC/ACT	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
MAX INT	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
TIME B4	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
CARS WT	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
STPTDUC	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
TTREDUC	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
MIN GAP	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

PLAN 1

, ., .																
PHASE	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
LOCK DET		Χ		Х		Χ		Χ								
VE RCALL																
PD RCALL		Х				Х										
MX RCALL																
SF RCALL																
NO REST																
AI CALC																

NextEdit

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COORDINATOR PATTERN 98

0
0
0
NONE
534
0

TIMING PLAN	0
SEQUENCE	0
ACTION PLAN	0
FORCE OFF	NONE
VEH PERM 1	0
VEH PERM 2	0
VEH PERM 2 - DISP	0
XART PTRN.	0
	-

RING CONFIG

			_										
RING	1	2	3	4	RING	1	2	3	4	RING	1	2	3
SPLT EXT	0	0	0	0	SPLIT DEMAND PTRN.	0	0			RING DISP		0	0

SPLIT PREF PHASES

PHASE	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
PREF 1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
PREF 2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

PHASE MODES

Phase	1	2	3	4	5	6	7	8
COORD								
PHASE MODE	NONE							

PHASE MODES

Phase	9	10	11	12	13	14	15	16
COORD								
PHASE MODE	NONE							

\mathbf{a}		0		-	
•	_	,,			
•		$\mathbf{\mathcal{L}}$	u	, ,	

PHASE	1	2	3	4	5	6	7	8
SF OUT								

COORDINATOR PATTERN 99

USE SPLIT PATTERN	0
CYCLE	0
OFFSET VAL	0
ACTUATED COORD	
ACT WALK REST	
PHASE RESERVICE	
MAX SELECT	NONE
STD (COS)	544
DWELL/ADD TIME	0

TIMING PLAN	0
SEQUENCE	0
ACTION PLAN	0
FORCE OFF	NONE
VEH PERM 1	0
VEH PERM 2	0
VEH PERM 2 - DISP	0
XART PTRN.	0

RING CONFIG

RING	1	2	3	4	RING	1	2	3	4	RING	1	2	3	4
SPLTEXT	0	0	0	0	SPLIT DEMAND PTRN	0	0			RING DISP		0	0	0

NextEdit

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SPLI	T F	PRI	ΕF	Pŀ	1A	SE	S									
PHASE	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
PREF 1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
PREF 2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

PHASE MODES									
Phase	1	2	3	4	5	6	7	8	
COORD									
PHASE MODE	NONE								

PHASE M	ODE	S						
Phase	9	10	11	12	13	14	15	16
COORD								
PHASE MODE	NONE							

SF OUT										
PHASE	1	2	3	4	5	6	7	8		
SF OUT										

COORDINATOR PA	TTERN 100
USE SPLIT PATTERN	0
CYCLE	0
OFFSET VAL	0
ACTUATED COORD	
ACT WALK REST	
PHASE RESERVICE	
MAX SELECT	NONE
STD (COS)	554
DWELL/ADD TIME	0

TIMING PLAN	0
SEQUENCE	0
ACTION PLAN	0
FORCE OFF	NONE
VEH PERM 1	0
VEH PERM 2	0
VEH PERM 2 - DISP	0
XART PTRN.	0

RING C	RING CONFIG													
RING	1	2	3	4	RING	1	2	3	4	RING	1	2	3	4
SPLT EXT	0	0	0	0	SPLIT DEMAND PTRN.	0	0			RING DISP		0	0	0

SPLIT PREF PHASES																
PHASE	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
PREF 1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
PREF 2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

PHASE MODES													
Phase	1	2	3	4	5	6	7	8					
COORD													
PHASE MODE	NONE												

PHASE MODES												
Phase	9	10	11	12	13	14	15	16				
COORD												
PHASE MODE	NONE											

SF OUT											
PHASE	1	2	3	4	5	6	7	8			
SF OUT											

NextEdit

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Page 4 May 5, 2021

am

ACTION PLAN 98

PATTERN	FREE
TIMING PLAN	1
VEH DET PLAN	0
FLASH	
VEH DET DIAG PLN	0
DIMMING ENABLE	

0
0
0

PED PR RETURN	
QUEUE DELAY	
PMT COND DELAY	

PHASE TABLE

PHASE	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
PED RCL																
WALK 2																
VEX 2		Χ		Χ		Χ		Χ								
VEH RCL																
MAX RCL																
MAX 2																
MAX 3																
CS INH																
OMIT																
SPC FCT																
AUX FCT																

LP TABLE

LP Statement	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15
LP 1-15															
LP 16-30															
LP 31-45															
LP 46-60															
LP 61-75															
LP 76-90															
LP 91-100															

ACTION PLAN 99

FREE		SYS OVERRIDE	
1		SEQUENCE	1
0		DET LOG	0
		RED REST	
0		PED DET DIAG PLN	0
		PRIORITY RETURN	
	1	1 0	1 SEQUENCE 0 DET LOG RED REST 0 PED DET DIAG PLN

PED PR RETURN	
QUEUE DELAY	
PMT COND DELAY	

PHASE TABLE

PHASE	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
PED RCL																
WALK 2																
VEX 2																
VEH RCL																
MAX RCL																

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PHASI	E 7	ΓΑΙ	BL	E												
PHASE	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
MAX 2																
MAX 3																
CS INH																
OMIT																
SPC FCT																

pm

LP	TA	BL	E
----	----	----	---

LP Statement	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15
LP 1-15															
LP 16-30															
LP 31-45															
LP 46-60															
LP 61-75															
LP 76-90															
LP 91-100															

ACTION PLAN 100

PATTERN	FLSH
TIMING PLAN	1
VEH DET PLAN	0
FLASH	X
VEH DET DIAG PLN	0

DIMMING ENABLE	
SYS OVERRIDE	
SEQUENCE	1
DET LOG	0
RED REST	

PED DET DIAG PLN	0
PRIORITY RETURN	
PED PR RETURN	
QUEUE DELAY	
PMT COND DELAY	

PHASE TABLE

PHASE	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
PED RCL																
WALK 2																
VEX 2																
VEH RCL																
MAX RCL																
MAX 2																
MAX 3																
CS INH																
OMIT																
SPC FCT																
AUX FCT																

LP TABLE

LP Statement	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15
LP 1-15															
LP 16-30															
LP 31-45															
LP 46-60															
LP 61-75															

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PHASE IN U	JSI	E/F	PEI	D												
PHASE	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
IN USE		Χ		Χ		Χ		Χ								
EXCLUSIVE PED																

PLAN 1	1															
PHASE	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
MIN GRN	0	10	0	5	0	10	0	5	0	0	0	0	0	0	0	0
BK MGRN	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
CS MGRN	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
DLY GRN	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
WALK	0	5	0	0	0	5	0	6	0	0	0	0	0	0	0	0
WALK2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
WLK MAX	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
PED CLR	0	5	0	0	0	5	0	6	0	0	0	0	0	0	0	0
PD CLR2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
PC MAX	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
PED CO	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
VEH EXT	0.0	6.0	0.0	3.0	0.0	6.0	0.0	3.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
VH EXT2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
MAX1	0	60	0	30	0	60	0	60	0	0	0	0	0	0	0	0
MAX2	0	60	0	40	0	60	0	40	0	0	0	0	0	0	0	0
MAX3	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
DYM MAX	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
DYM STP	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
YELLOW	0.0	4.0	0.0	3.5	0.0	4.0	0.0	3.5	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
RED CLR	0.0	1.0	0.0	1.5	0.0	1.0	0.0	1.5	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
RED MAX	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
RED RVT	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
ACT B4	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
SEC/ACT	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
MAX INT	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
TIME B4	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
CARS WT	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
STPTDUC	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
TTREDUC	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
MIN GAP	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

PLAN 1	PLAN 1															
PHASE	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
LOCK DET																
VE RCALL																
PD RCALL		Х				Χ										
MX RCALL		Х				Χ										
SF RCALL																
NO REST																
AI CALC																

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COORDINATOR PATTERN 99

USE SPLIT PATTERN	0
CYCLE	0
OFFSET VAL	0
ACTUATED COORD	
ACT WALK REST	
PHASE RESERVICE	
MAX SELECT	NONE
STD (COS)	544
DWELL/ADD TIME	0

TIMING PLAN	0
SEQUENCE	0
ACTION PLAN	0
FORCE OFF	NONE
VEH PERM 1	0
VEH PERM 2	0
VEH PERM 2 - DISP	0
XART PTRN.	0

RING CONFIG

RING	1	2	3	4	RING	1	2	3	4
SPLT EXT	0	0	0	0	SPLIT DEMAND PTRN.	0	0		

RING	1	2	3	4
RING DISP		0	0	0

SPLIT PREF PHASES

PHASE	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
PREF 1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
PREF 2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

PHASE MODES

Phase	1	2	3	4	5	6	7	8
COORD								
PHASE MODE	NONE							

PHASE I	
	<i>(///)) \</i>
FIIASEI	NODLO

Phase	9	10	11	12	13	14	15	16
COORD								
PHASE MODE	NONE							

PHASE	1	2	3	4	5	6	7	8
SF OUT								

ACTION PLAN 99

PATTERN	FREE	SYS OVERRIDE	
TIMING PLAN	1	SEQUENCE	1
VEH DET PLAN	0	DET LOG	0
FLASH		RED REST	
VEH DET DIAG PLN	0	PED DET DIAG PLN	0
DIMMING ENABLE		PRIORITY RETURN	

PED PR RETURN	
QUEUE DELAY	
PMT COND DELAY	

PHASE TABLE

PHASE	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
PED RCL																
WALK 2																
VEX 2																
VEH RCL																
MAX RCL																
MAX 2																

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PHAS	E 1	ΊΑ	BL	E												
PHASE	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
MAX 3																
CS INH																
OMIT																
SPC FCT																
AUX FCT																

LP IABL	.E														
LP Statement	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15
LP 1-15															
LP 16-30															
LP 31-45															
LP 46-60															
LP 61-75															
LP 76-90															
LP 91-100															

Day Plan 1																									
EVENT	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25
ACTION PLAN	99	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - HH	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - MM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Day Plan 1																									
EVENT	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50
ACTION PLAN	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - HH	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - MM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Day Plan 2																									
EVENT	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25
ACTION PLAN	99	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - HH	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - MM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Day Plan 2																									
EVENT	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50
ACTION PLAN	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - HH	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - MM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Day Plan 3																									
EVENT	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25
ACTION PLAN	99	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - HH	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

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EVENT 1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16 17 18 19 20 21 22 23 24 25																										iay
START TIME - MM O O O O O O O O O	Day Plan 3																									
START TIME - MM O O O O O O O O O	EVENT	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25
EVENT 26 27 28 29 30 31 32 33 34 35 36 37 38 39 40 41 42 43 44 45 46 47 48 49 50 ACTION PLAN 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	START TIME - MM	0	0	0	0	0	0	0	0	0																
ACTION PLAN	Day Plan 3																									
START TIME - HH 0 0 0 0 0 0 0 0 0	EVENT	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50
START TIME - MM 0 0 0 0 0 0 0 0 0	ACTION PLAN	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Day Plan 4	START TIME - HH	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
EVENT 1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16 17 18 19 20 21 22 23 24 25 ACTION PLAN 99 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	START TIME - MM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
ACTION PLAN 99 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Day Plan 4																									
START TIME - HH 0 0 0 0 0 0 0 0 0	EVENT	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25
START TIME - MM O O O O O O O O O	ACTION PLAN	99	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
EVENT	START TIME - HH	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
EVENT	START TIME - MM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
ACTION PLAN 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Day Plan 4																									
START TIME - HH 0 0 0 0 0 0 0 0 0	EVENT	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50
START TIME - MM 0 0 0 0 0 0 0 0 0	ACTION PLAN	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
EVENT	START TIME - HH	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
EVENT 1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16 17 18 19 20 21 22 23 24 25 ACTION PLAN 99 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	START TIME - MM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
ACTION PLAN 99 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Day Plan 5																									
START TIME - HH 0 0 0 0 0 0 0 0 0	EVENT	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25
START TIME - MM 0 0 0 0 0 0 0 0 0	ACTION PLAN	99	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Day Plan 5 EVENT 26 27 28 29 30 31 32 33 34 35 36 37 38 39 40 41 42 43 44 45 46 47 48 49 50 ACTION PLAN 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	START TIME - HH	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
EVENT	START TIME - MM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
ACTION PLAN 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Day Plan 5																									
START TIME - HH 0 0 0 0 0 0 0 0 0	EVENT	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50
START TIME - MM 0 0 0 0 0 0 0 0 0	ACTION PLAN	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Day Plan 6 EVENT 1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16 17 18 19 20 21 22 23 24 25 ACTION PLAN 99 0	START TIME - HH	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
EVENT 1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16 17 18 19 20 21 22 23 24 25 ACTION PLAN 99 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	START TIME - MM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
ACTION PLAN 99 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Day Plan 6																									
START TIME - HH 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	EVENT	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25
START TIME - MM 0 0 <th< td=""><td>ACTION PLAN</td><td>99</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td></th<>	ACTION PLAN	99	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Day Plan 6 EVENT 26 27 28 29 30 31 32 33 34 35 36 37 38 39 40 41 42 43 44 45 46 47 48 49 50 ACTION PLAN 0	START TIME - HH	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
EVENT 26 27 28 29 30 31 32 33 34 35 36 37 38 39 40 41 42 43 44 45 46 47 48 49 50 ACTION PLAN 0	START TIME - MM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
ACTION PLAN 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Day Plan 6																									
START TIME - HH 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	EVENT	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50
	ACTION PLAN	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - MM 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	START TIME - HH	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	START TIME - MM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

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Day Plan 7																									
EVENT	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25
ACTION PLAN	99	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - HH	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - MM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Day Plan	7																									
EVENT		26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50
ACTION PLA	٨N	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME -	- HH	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME -	MM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

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LP TABL	E														
LP Statement	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15
LP 76-90															
LP 91-100															

Day Plan 1				am			pm																
									1	ı	l	I	ı	l	I					1		l	
EVENT	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23
ACTION PLAN	99	100	99	98	99	98	99	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - HH	0	0	5	6	9	15	18	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - MM	0	30	30	30	30	0	30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
									-								-	-			-		

Day Plan 1																											
EVENT	24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50
ACTION PLAN	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - HH	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - MM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Day Plan 2																							
EVENT	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23
ACTION PLAN	99	100	99	98	99	98	99	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - HH	0	0	5	6	9	15	18	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - MM	0	30	30	30	30	0	30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Day Plan 2																											
EVENT	24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50
ACTION PLAN	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - HH	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - MM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Day Plan 3																							
EVENT	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23
ACTION PLAN	99	100	99	98	99	98	99	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - HH	0	0	5	6	9	15	18	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - MM	0	30	30	30	30	0	30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Day Plan 3																											
EVENT	24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50
ACTION PLAN	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - HH	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - MM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Day Plan 4																							
EVENT	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23
ACTION PLAN	99	100	99	98	99	98	99	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - HH	0	0	5	6	9	15	18	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - MM	0	30	30	30	30	0	30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

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Day Plan 4																											
EVENT	24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50
ACTION PLAN	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - HH	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - MM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Day Plan 5																							
EVENT	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23
ACTION PLAN	99	100	99	98	99	98	99	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - HH	0	0	5	6	9	15	18	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - MM	0	30	30	30	30	0	30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Day Plan 5																											
EVENT	24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50
ACTION PLAN	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - HH	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - MM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Day Plan 6																							
EVENT	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23
ACTION PLAN	99	100	99	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - HH	0	0	5	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - MM	0	30	30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Day Plan 6																											
EVENT	24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50
ACTION PLAN	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - HH	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - MM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Day Plan 7																							
EVENT	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23
ACTION PLAN	99	100	99	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - HH	0	0	5	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - MM	0	30	30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Day Plan 7																											
EVENT	24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50
ACTION PLAN	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - HH	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - MM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

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PHASE IN U	JS	E/F	PEI	D												
PHASE	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
IN USE	Χ	Χ	Χ	Χ		Х		Х								
EXCLUSIVE PED																

PHASE	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
MIN GRN	5	10	5	5	0	10	0	5	0	0	0	0	0	0	0	0
BK MGRN	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
CS MGRN	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
DLY GRN	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
WALK	0	7	0	7	0	7	0	7	0	0	0	0	0	0	0	0
WALK2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
WLK MAX	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
PED CLR	0	14	0	15	0	14	0	15	0	0	0	0	0	0	0	0
PD CLR2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
PC MAX	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
PED CO	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
VEH EXT	3.0	0.0	3.0	3.0	0.0	0.0	0.0	3.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
VH EXT2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
MAX1	15	40	15	30	0	40	0	30	0	0	0	0	0	0	0	0
MAX2	40	60	30	40	0	60	0	60	0	0	0	0	0	0	0	0
MAX3	40	0	0	20	0	0	0	60	0	0	0	0	0	0	0	0
DYM MAX	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
DYM STP	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
YELLOW	3.5	4.0	3.5	4.0	3.0	4.0	3.0	4.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
RED CLR	2.5	2.0	2.5	2.0	0.0	2.0	0.0	2.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
RED MAX	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
RED RVT	5.0	2.0	5.0	5.0	5.0	2.0	5.0	5.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
ACT B4	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
SEC/ACT	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
MAX INT	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
TIME B4	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
CARS WT	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
STPTDUC	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
TTREDUC	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
MIN GAP	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

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PLAN 1

, ., .,																
PHASE	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
LOCK DET								Х								
VE RCALL																
PD RCALL		Χ				Х										
MX RCALL		Χ				Χ										
SF RCALL																
NO REST																
AI CALC																

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COORDINATOR OPTIONS

MANUAL PATTERN	AUTO
SYSTEM SOURCE	SYS
SPLITS IN	SECONDS
TRANSITION	SMOOTH
DWELL/ADD TIME	255
DLY COORD WK-LZ	
OFFSET REF	LAG
PED RECALL	Х
LOCAL ZERO OVRD	
RE-SYNC COUNT	1

ECPI COORD	Χ
SYSTEM FORMAT	STD
OFFSET IN	SECONDS
MAX SELECT	MAX2
FORCE OFF	FIXED
CAL USE PED TM	Χ
PED RESERVE	
FO ADD INI GRN	
MULTISYNC	
·	

COORDINATOR PATTERN 1

USE SPLIT PATTERN	1
CYCLE	150
OFFSET VAL	143
ACTUATED COORD	
ACT WALK REST	
PHASE RESERVICE	
MAX SELECT	NONE
STD (COS)	111
DWELL/ADD TIME	0

	_
TIMING PLAN	1
SEQUENCE	1
ACTION PLAN	0
FORCE OFF	NONE
VEH PERM 1	0
VEH PERM 2	0
VEH PERM 2 - DISP	0
XART PTRN.	0
	•

RING CONFIG

RI	NG	1	2	3	4	RING	1	2	3	4	RING	1	2	3	4
SPL	EXT	0	0	0	0	SPLIT DEMAND PTRN.	0	0			RING DISP		0	0	0

SPLIT PREF PHASES

PHASE	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
PREF 1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
PREF 2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

PHASE MODES

Phase	1	2	3	4	5	6	7	8
COORD		Х				Х		
PHASE MODE	NONE							

PHASE MODES

Phase	9	10	11	12	13	14	15	16
COORD								
PHASE MODE	NONE							

SF OUT												
PHASE	1	2	3	4	5	6	7	8				
SF OUT												

COORDINATOR PATTERN 2

USE SPLIT PATTERN	2
CYCLE	120
OFFSET VAL	0

ACTUATED COORD	
ACT WALK REST	
PHASE RESERVICE	

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COORDINATOR PATTERN 2

MAX SELECT	NONE
STD (COS)	121
DWELL/ADD TIME	0
TIMING PLAN	1
SEQUENCE	1
ACTION PLAN	0

FORCE OFF	NONE
VEH PERM 1	0
VEH PERM 2	0
VEH PERM 2 - DISP	0
XART PTRN.	0
	•

RING CONFIG

RING	1	2	3	4	RING	1	2	3	4	RING	1	2	3	4
SPLT EXT	0	0	0	0	SPLIT DEMAND PTRN.	0	0			RING DISP		0	0	0

SPLIT PREF PHASES

PHASE	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
PREF 1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
PREF 2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

PHASE MODES

Phase	1	2	3	4	5	6	7	8
COORD		Х				X		
PHASE MODE	NONE							

PHASE MODES

Phase	9	10	11	12	13	14	15	16
COORD								
PHASE MODE	NONE							

SF O	SF OUT											
PHASE	1	2	3	4	5	6	7	8				
SF OUT												

COORDINATOR PATTERN 3

USE SPLIT PATTERN	3
CYCLE	150
OFFSET VAL	20
ACTUATED COORD	
ACT WALK REST	
PHASE RESERVICE	
MAX SELECT	NONE
STD (COS)	131
DWELL/ADD TIME	0

TIMING PLAN	1
SEQUENCE	1
ACTION PLAN	0
FORCE OFF	NONE
VEH PERM 1	0
VEH PERM 2	0
VEH PERM 2 - DISP	0
XART PTRN.	0

RING CONFIG

RING	1	2	3	4	RING	1	2	3	4	RING	1	2	3	4
SPLT EXT	0	0	0	0	SPLIT DEMAND PTRN.	0	0			RING DISP		0	0	0

SPLIT PREF PHASES

PHASE	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
PREF 1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
PREF 2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

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PHASE MODES

Phase	1	2	3	4	5	6	7	8
COORD		Х				Х		
PHASE MODE	NONE							

SF OUT

31	V	<u> </u>							
PHAS	SE	1	2	3	4	5	6	7	8
SF O	UT								

PHASE MODES

Phase	9	10	11	12	13	14	15	16
COORD								
PHASE MODE	NONE							

COORDINATOR PATTERN 4

USE SPLIT PATTERN	4
CYCLE	110
OFFSET VAL	0
ACTUATED COORD	
ACT WALK REST	
PHASE RESERVICE	
MAX SELECT	NONE
STD (COS)	141
DWELL/ADD TIME	0

TIMING PLAN	1
SEQUENCE	1
ACTION PLAN	0
FORCE OFF	NONE
VEH PERM 1	0
VEH PERM 2	0
VEH PERM 2 - DISP	0
XART PTRN.	0

RING CONFIG

RING	1	2	3	4	RING	1	2	3	4	RING	1	2	3	4
SPLT EXT	0	0	0	0	SPLIT DEMAND PTRN.	0	0			RING DISP		0	0	0

SPLIT PREF PHASES

PHASE	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
PREF 1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
PREF 2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

PHASE MODES

Phase	1	2	3	4	5	6	7	8
COORD		Х				Х		
PHASE MODE	NONE							

PHASE MODES

Phase	9	10	11	12	13	14	15	16
COORD								
PHASE MODE	NONE							

SF OUT									
PHASE	1	2	3	4	5	6	7	8	
SF OUT									

COORDINATOR PATTERN 30

USE SPLIT PATTERN	30
CYCLE	150
OFFSET VAL	36
ACTUATED COORD	
ACT WALK REST	

PHASE RESERVICE	
MAX SELECT	NONE
STD (COS)	252
DWELL/ADD TIME	0
TIMING PLAN	1

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COORDINATOR PATTERN 30

SEQUENCE	1	
ACTION PLAN	0	
FORCE OFF	NONE	
VEH PERM 1	0	

VEH PERM 2	0
VEH PERM 2 - DISP	0
XART PTRN.	0

RING CONFIG

RING	1	2	3	4	RING	1	2	3	4	RING	1	2	3	4
SPLT EXT	0	0	0	0	SPLIT DEMAND PTRN.	0	0			RING DISP		0	0	0

SPLIT PREF PHASES

PHASE	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
PREF 1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
PREF 2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

PHASE MODES

	Phase	1	2	3	4	5	6	7	8
	COORD		Х				Х		
F	PHASE MODE	NONE							

PHASE MODES

Phase	9	10	11	12	13	14	15	16
COORD								
PHASE MODE	NONE							

SF	OUT	

31 0	<u> </u>							
PHASE	1	2	3	4	5	6	7	8
SF OUT								

Split 1

PHASE	1	2	3	4	5	6	7	8
SPLIT	43	61	16	30	0	104	0	46
COORD		X				X		
PHASE MODE	NONE							

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Split 1

PHASE	9	10	11	12	13	14	15	16
SPLIT	0	0	0	0	0	0	0	0
COORD								
PHASE MODE	NONE							

Split 2

-								
PHASE	1	2	3	4	5	6	7	8
SPLIT	18	42	24	36	0	60	0	60
COORD		X				Χ		
PHASE MODE	NONE							

Split 2

Opiic																	
PHASE	9	10	11	12	13	14	15	16	PHASE	9	10	11	12	13	14	15	16
SPLIT	0	0	n	n	0	n	n	0	COORD								

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Split 2								
PHASE	9	10	11	12	13	14	15	16
PHASE MODE	NONE							

Split 3								
PHASE	1	2	3	4	5	6	7	8
SPLIT	13	88	21	28	0	101	0	49
COORD		Χ				Χ		
PHASE MODE	NONE							

Split 3								
PHASE	9	10	11	12	13	14	15	16
SPLIT	0	0	0	0	0	0	0	0
COORD								
PHASE MODE	NONE							

Split 4								
PHASE	1	2	3	4	5	6	7	8
SPLIT	17	42	19	32	0	59	0	51
COORD		Х				X		
PHASE MODE	NONE							

Split 4								
PHASE	9	10	11	12	13	14	15	16
SPLIT	0	0	0	0	0	0	0	0
COORD								
PHASE MODE	NONE							

Split 30								
PHASE	1	2	3	4	5	6	7	8
SPLIT	23	62	31	34	0	85	0	65
COORD		Х				X		
PHASE MODE	NONE							

Split 30								
PHASE	9	10	11	12	13	14	15	16
SPLIT	0	0	0	0	0	0	0	0
COORD								
PHASE MODE	NONE							

ACTION PLAN	V 1			
PATTERN	1	DIMMING ENABLE		PED DET DIAG PLN 0
TIMING PLAN	1	SYS OVERRIDE		PRIORITY RETURN
VEH DET PLAN	0	SEQUENCE	1	PED PR RETURN
FLASH		DET LOG	0	QUEUE DELAY
VEH DET DIAG PLN	0	RED REST		PMT COND DELAY
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PHASE	PHASE TABLE															
PHASE	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
PED RCL																
WALK 2																
VEX 2																
VEH RCL																
MAX RCL																
MAX 2																
MAX 3	Χ															
CS INH																
OMIT																
SPC FCT																
AUX FCT																

LP TABLE

LP Statement	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15
LP 1-15															
LP 16-30															
LP 31-45															
LP 46-60															
LP 61-75															
LP 76-90															
LP 91-100															

ACTION PLAN 2

PATTERN	2	SYS OVERRIDE
TIMING PLAN	1	SEQUENCE
VEH DET PLAN	0	DET LOG
FLASH		RED REST
VEH DET DIAG PLN	0	PED DET DIAG PLN
DIMMING ENABLE		PRIORITY RETURN

PED PR RETURN	
QUEUE DELAY	
PMT COND DELAY	

PHASE	= 7	AL	3L	E												
PHASE	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
PED RCL																
WALK 2																
VEX 2																
VEH RCL																
MAX RCL																
MAX 2																
MAX 3				Χ												
CS INH																
OMIT																
SPC FCT																
AUX FCT																

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pm

LP TABL	LP TABLE														
LP Statement	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15
LP 1-15															
LP 16-30															
LP 31-45															
LP 46-60															
LP 61-75															
LP 76-90															
LP 91-100															

ACTION	I PLAN 3
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3
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0

PED PR RETURN	
QUEUE DELAY	
PMT COND DELAY	

PHASE TABLE

PHASE	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
PED RCL																
WALK 2																
VEX 2																
VEH RCL																
MAX RCL																
MAX 2																
MAX 3																
CS INH																
OMIT																
SPC FCT																
AUX FCT																

LP TABLE

LP Statement	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15
LP 1-15															
LP 16-30															
LP 31-45															
LP 46-60															
LP 61-75															
LP 76-90															
LP 91-100															

ACTION PLAN 4

PATTERN	4
TIMING PLAN	1
VEH DET PLAN	0
FLASH	

VEH DET DIAG PLN	0
DIMMING ENABLE	
SYS OVERRIDE	
SEQUENCE	1

DET LOG	0
RED REST	
PED DET DIAG PLN	0
PRIORITY RETURN	

PED PR RETURN	
QUEUE DELAY	
PMT COND DELAY	

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PHASE TABLE																
PHASE	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
PED RCL																
WALK 2																
VEX 2																
VEH RCL																
MAX RCL																
MAX 2																
MAX 3				Χ												
CS INH																
OMIT																
SPC FCT																
AUX FCT																

LP TABL	Ε														
LP Statement	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15
LP 1-15															
LP 16-30															
LP 31-45															
LP 46-60															
LP 61-75															
LP 76-90															
LP 91-100															

ACTION PLAN	30			
PATTERN	30	SYS OVERRIDE		PED PR RETURN
TIMING PLAN	1	SEQUENCE	1	QUEUE DELAY
VEH DET PLAN	0	DET LOG	0	PMT COND DELAY
FLASH		RED REST		
VEH DET DIAG PLN	0	PED DET DIAG PLN	0	
DIMMING ENABLE		PRIORITY RETURN		

PHASE TABLE																
PHASE	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
PED RCL																
WALK 2																
VEX 2																
VEH RCL																
MAX RCL																
MAX 2																
MAX 3																
CS INH																
OMIT																
SPC FCT																
AUX FCT																

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LP TABL	Ε														
LP Statement	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15
LP 1-15															
LP 16-30															
LP 31-45															
LP 46-60															
LP 61-75															
LP 76-90															
LD 01 100															

ACTION PLAN 99

FREE
1
0
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SYS OVERRIDE	
SEQUENCE	1
DET LOG	0
RED REST	
PED DET DIAG PLN	0
PRIORITY RETURN	

PED PR RETURN	
QUEUE DELAY	
PMT COND DELAY	

PHASE TABLE

PHASE	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
PED RCL																
WALK 2																
VEX 2																
VEH RCL																
MAX RCL																
MAX 2																
MAX 3																
CS INH																
OMIT																
SPC FCT																
AUX FCT																

LP TABLE

LP Statement	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15
LP 1-15															
LP 16-30															
LP 31-45															
LP 46-60															
LP 61-75															
LP 76-90															
LP 91-100															

ACTION PLAN 100

PATTERN	FLSH
TIMING PLAN	1
VEH DET PLAN	0
FLASH	Х

VEH DET DIAG PLN	0
DIMMING ENABLE	
SYS OVERRIDE	
SEQUENCE	1

DET LOG	0
RED REST	
PED DET DIAG PLN	0
PRIORITY RETURN	

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ACTIO	N F	L	4٨	1 1	00																					10	iay	0, 2	202 1
PED PR	RET	UR	N				Q	UEL	JE C	EL	AY					PM ⁻	T CC	DND I	DEL	AY									
PHASE	RCL																												
PHASE	PR RETURN QUEUE DELAY PMT COND DELAY SE TABLE																												
PED RCL																													
WALK 2																													
VEX 2																													
VEH RCL																													
MAX RCL																													
MAX 2																													
MAX 3																													
CS INH																	_												
OMIT																													
SPC FCT																_	_												
AUX FCT																													
LP TAI	BLE	=																											
LP Stateme	ent	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	5												
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LP 16-30) .																_												
LP 31-45	5 .																_												
LP 46-60) .																4												
LP 61-75	5 .	_ .															4												
LP 76-90		_ .															4												
LP 91-10	0 .			.																									
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START TIN	ЛЕ - I	MM	0	3	0	30	0	30	0		15	0	0	0	()	0	0	0	0	0	0	()	0	0	0	0	0
Dav Pl	an	1						•																					
		_	24	25	26	5 2 ⁻	7 28	3 29	30	3 3	1 3	2 3	3 34	4 35	5 3	6 3	37 3	38 39	9 40	41	42	43	44	45	46	47	48	49 5	50
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START TI			0	0	0	0	0	0	0	0	-	_	0	0	0) (0	0	0	0	0	0	0	0	0	0	0 0	
START TIM				0	0	0	0	0	0	0	-		_	0	0				0	0	_		0	0	0	0	0	0 0	_
Day Pla	an i	2																											
EVE	NT		1	_	2	3	4	5	(6	7	8	9	10	0	11	12	13	14	15	16	17	7	18	19	20	21	22	23
ACTION	PLA	N	99	1	00	99	1	2	30		3	4	99	0	()	0	0	0	0	0	0	-)	0	0	0	0	0
START TI	ME -	НН	0	0	-	5	6	9	14	4	16	19	22	0	(0	0	0	0	0	0	(0	0	0	0	0
START TIM	⁄Е - I	MM	0	3	0	30	0	30	0		15	0	0	0	()	0	0	0	0	0	0	()	0	0	0	0	0

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Day Plan 2																											
EVENT	24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50
ACTION PLAN	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - HH	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - MM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Day Plan 3																							
EVENT	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23
ACTION PLAN	99	100	99	1	2	30	3	4	99	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - HH	0	0	5	6	9	14	16	19	22	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - MM	0	30	30	0	30	0	15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Day Plan 3																											
EVENT	24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50
ACTION PLAN	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - HH	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - MM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Day Plan 4																							
EVENT	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23
ACTION PLAN	99	100	99	1	2	30	3	4	99	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - HH	0	0	5	6	9	14	16	19	22	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - MM	0	30	30	0	30	0	15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Day Plan 4																											
EVENT	24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50
ACTION PLAN	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - HH	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - MM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Day Plan 5																							
EVENT	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23
ACTION PLAN	99	100	99	1	2	30	3	4	99	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - HH	0	0	5	6	9	14	16	19	22	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - MM	0	30	30	0	30	0	15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Day Plan 5																											
EVENT	24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50
ACTION PLAN	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - HH	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - MM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Day Plan 6																							
EVENT	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23
ACTION PLAN	99	100	99	2	99	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - HH	0	0	5	9	22	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

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Day Plan 6																							
EVENT	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23
START TIME - MM	0	30	30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Day Plan 6																											
EVENT	24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50
ACTION PLAN	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - HH	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - MM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Day Plan 7																							
EVENT	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23
ACTION PLAN	99	100	99	2	99	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - HH	0	0	5	9	22	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - MM	0	30	30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Day Plan 7																											
EVENT	24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50
ACTION PLAN	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - HH	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
START TIME - MM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	o

NextEdit

C-25 D-144

INT.# (Cabinet Typ		+							RSEC									
409	TS1	TSS	1,	_			Brac	lley E	lvd	MD.1	91/	Fern	WOO	d R	oad				
					WB Bradley Blvd MD.191		Fernwood Road		Bradley Blvd MD.191		Fernwood Road								
Z-1 CC		R TIMING DATA	100		3		S B				g R								1
PHASE	PLAN 1		15		- 4		BET			7570	(FA)		F.	10.5	VI.		316		70年月
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	M GREEN			10		5	10	10		1	50								
	E MINIMUM GF																		
_		E MINIMUM GREEN															1	1	
DELAYE	D GREEN						YU.										1	1	-
211			4	ST		No.	58	File	au	MELL	56	5.00	200	- 1	The	935	a me	000	S. S. S. S.
WALK				2		7*	T	2	T	T	T		T	T			1	720-50	1
WALK 2							1		+	1	1	1					1	+	-
WALKM	ΑX								-				1				+	+	+
PEDEST	RIAN CLEARAI	NCE		2		8	0	2	1	1	1	-	+	-			-	+	-
PEDEST	RIAN CLEARAI	NCE 2		5		1			1	-	+	-	+	-		-	-	+	+
PEDEST	RIAN CLEARAI	NCE MAX					1		1	-	1	+	-	-	-		1	-	-
PEDEST	RIAN CARRY C	OVER	l lane	Gran									1	1					
VEHICLE	EXTENSION		-	3.0	T	3.0	T	3.0	T	121		137	1		200	251	000	ARIS	-
VEHICLE	EXTENSION 2		1	4.0	-	4.0	-	4.0	+	3.0		-	+	-	_				_
- Trie	Note: Ne	- 8 - S - S M	(Zame	110	-	14.0	- 34	9.0	-	4,0	1								
VIAX1			-	50	1	30		T 50	-	1 40	-	9.0	-	-	C. (F)		Con.	Same	Sep.
JAX2				50	-	60	-	50	-	10	+	+	_	-					
ЛАХ3			-	30	-	00		50	-	15	-	-		1					
YNAMIC	MAX		-	70	-	100	-	70	_	1									
	MAX STEP			20.0		15.0	-	70	_	60	-	-							
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ED MAX				1.0		1.0		1.0		1.0									
ED REVE	:PT									-									
LED IVE	411		5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	2.0	2.0	2.	0 2	2.0	2.0	2.0	2.0	2.0
CTHATIC	NE DEFORE A	AD DEDUCTION	1007.0		N2FJV	V.		-	- 4	4	66	3 1				45		ķ+	
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INIMUM G	AP											-	-	-			_		

TOD (Late) Flash Spec. Action Plan Spec. Action Plan	00:30 - 05:30 7 Days AP 98 Coll	S VEXT 2 for all	phones 6:30-9:30 f 15:00-18:30	- W (763)
Submitted by / Date:		Checked by / Date	Approved by / Date MK 8/18/201	- 912-11

Appendix EN Sign of Firo per North of the test est

TRAFFIC OPERATIONS SECTION
DIVISION OF TRAFFIC ENGINEERING
MONTGOMERY COUNTY MARYLAND

Attachmend D

NO. 409-B

PHASING

INTERSECTION: Bradley Blvd. (MD 191) FFER Wood 12d.

		SIGN	IAL HEAD	INDICAT	IONS	
SIGNAL NO.	1-4			5-9		
TOTAL:	4			5		
LEGEND OPTICALLY LIMITED R - RED Y - YELLOW G - GREEN ARROW F - FLASHING	(R) (Y) (G)	(R) (>) (G) (G)	(R) (Y) (Y) (G) (G) (G)	(R) (S) (S) (S) (S) (S) (S) (S) (S) (S) (S		9" or 12"

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2	G	Y	R	R	/R	R	-			-	-	-	+	-	-	+	11
3	G	Y	R	13	R	R	1	-		-	-	+	+	-	-	+	14
4	G	Y	R	R	R	R			-	-	-	-	-	-	+	+	Y
5	R	R	R	G	Y	R			1			-	-	+	-	+	R
6	19	R	R	G	7	R	-	_			-	+	-	-	-	-	R
7	R	R	R	G	Y	R					-	-	-	-	-	-	_
8	R	R	R	G	Y	R						-	-		-	-	R
9	R	R	R	G	V	R						-	-	-		-	R
10					-	1					-	-	-	-	-	-	A
11					-												-
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NOTES:

CHADGE TO B' IS FOR CABINET REPLACEMENT

SUBMITTED:	CR	12-1-94	CHECKED:	GAN	12/2/94	APPROVED: Buy 12/2/94
IN SERVIC	E BY:		DATE:	-/-	110	TIME:

T.#	Cabinet Type	System								Vork							Atta	_
64	TS1	TSS				Brac	lley	Blvd.	(Md	.191)	- Se	ven l	Lock	s Rd			_	G
_	CONTROLLER 1	TIMING DATA	SBLT Seven Locks Rd.	NB Seven Locks Rd.	EBLT Bradlev Blvd,	WB Bradley Blvd, (Md.191)		SB Seven Locks Bd.		EB Bradley Blvd. (Md.191)		Silvi en la companya de la companya		·				
_	NG PLAN 1		200	T .	L	NA STATE	100	I c	1 -	1 .	9	10	11	12	13	14	15	16
PHA			1	2	3	4	5	6	7	8	1 9	10	111	12	10	-	1	1.0
-	MUM GREEN		5	10	.5	5	_	10	-	5	-	-	-	-	1	-	-	-
BICY	CLE MINIMUM GREE	N		_					-	-	_	-		-	-	-	-	-
CON	IDITIONAL SERVICE N	MINIMUM GREEN			_			-		-		-	_	-	-	-	-	-
DEL	AYED GREEN																1	1
53	SALE MALE VI		127				701	5-17	200						- /		1.0	-
WAL	K			7		7		7		7								-
WAL	К2																	
WAL	K MAX																	
PED	ESTRIAN CLEARANCI	E		14		15		14		15								
PEDE	ESTRIAN CLEARANCE	E 2							-									-
_	ESTRIAN CLEARANCI													P				
_	ESTRIAN CARRY OVE						inis	Syria S	100	Lim.						VG-F		
VEHI	CLE EXTENSION		3,0		3.0	3.0		T		3.0		T						
-	CLE EXTENSION 2			-				-										
The same	CEE EXTENDION E	Stantin -	27.	and and	0.752	200	430	100	-	200	Call S			13	1	-	9	
MAX:			15	40	15	30		40		30								
MAX2			40	60	30	80	-	60	-	80								
-			70	-		0.0	_	-	_	-								
MAX			-															
-	AMIC MAX		_					-		-	_	-	-					
DYNA	AMIC MAX STEP		-	_	-			SHEEP I	150			_		-	200		- 07	
			firmali,		4.5	-	-	1.0		4.0		_						1
	OW CHANGE		3.5	4.0	3.5	4.0	_	4.0	_	_	_				-			
	CLEARANCE		2.5	2.0	2.5	2.0		2,0		2.0						-		
RED I								- 5				0.0	0.0	0.0	2.0	2.0	2.0	2.0
REDI	REVERT		5.0	2.0	5.0	5.0	5.0	20	5.0	5.0	2.0	2.0	2.0	2.0	2.0	3.0	2.0	2.0
190		I I I	7		136	-						_						
-	JATIONS BEFORE GA		_					-							-		-	
	NDS PER ACTIONS A								-	_						-		
	MUM ADDED INITIAL																	_
TIME	BEFORE GAP REDUC	TION			\											-	-	_
CARS	WAITING BEFORE G	AP REDUCTION																
STEP	TO REDUCE							1										
TIME	TO REDUCE TO MINI	MUM																
MINUNA	IUM GAP		(-1		1 - 1										1.37		

THINNING PURPOSE	SONE 1			m - n ud-	7-6-2017
TOD (Late) Flash	00:30 - 05:30 7 Days	* ADD	Bailing	President	
Spec. Action Plan					
Spec. Action Plan					
Submitted by / Date:	, ,	Checked by / Date	* KHamud	9/6/11 Approve	oby/Date KHAMMUN 9/6/11
In Service by / Date/	Time CR 12K15H	1A 6/14	4/12 at	1130	

SEQUENCE OF OPERATION SHEET

Appendix C STOTER ATTIMING WORKSTREETS DIVISION OF TRAFFIC ENGINEERING MONTGOMERY COUNTY, MARYLAND

NO. Attachmend D.

PHASING

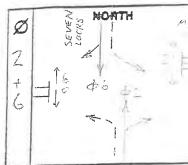
INTERSECTION: Bradley Blud. (MD 191) 4 Seven Locks Red.

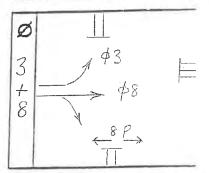
	' SIGNAL HEAD INDICATIONS										
SIGNAL NO.	2-6.8	1,7				9-16					
TOTAL	6	2		,		8					
DOPTICALLY LIMITED R. RED Y. YELLOW G. GREEN ARROW F. FLASHING	(R) (G) 12*	(R) (Y) (Y) (G) (G)	(R) (Y) (Y) (G) (G) (G)	(R) (Y) (G) (J)	(R) (Y) (Y) (G) (G) 12" 8"	16" PEDESTRIAN COUNTOWN SIGNAL HEAD "LED" TYPE A FS PUSE B a HCM					

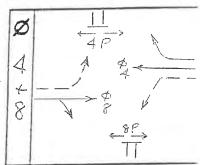
					5	EQU	ENC		_		RATI	ION					FL
SCHAL								STE	MAYT					-		-	4 5
NC.	1	2	3	4	5	6	7	8	9	10	111	112	13	14	15	18	LE
1	R	R	R	R	-06	475	1.G	G	G	G	14	IR	R	R	R		1
2	R	R	R	R	6	G	G	G	G	(F	ΙΫ́	R	R				1
3	R	R	R	R	R	R	R	G	16		14	IR	R	R			1
4	R	R	R	R	R	R	18	G	5	G	1	TR	R	R	12		
5	G	G	TY	R	R	18	R	R	R	17	1 R	R	R	R			
6	G	G	Y	R	R	R	R	IR	R	R	1.2	IR	R	IR	R		
7	6	G	4	R	R	R	R	R	R	R	18	1 A	+6C	_			,
8	G	G	Y	R	13	R	R	R	R	R	R	IR	G	6	G		
9	DW	DW	DW	DW	DW	DW	DW	W	DWF	DW	347	(U)	DW	DI	DW		_
10	Du	NW	DIN	DW) W	DW	DW	W	DW=	DW	A UL	DW.	DW	DW	DW		-
17	DW	Die	DW	DW	W	W	W	W	DWF	DLU	DUC	10m	DW	17m	-		-
12	DW	DIU	DW	DW	W	W	W	W	DWF	DIM	1000	DIM	DW	DW	DW		
13	W	DWF	Du	SW	DW	DIM	DW	DW	DW	MU	DW	Dut	W	W	14/		_
14	W	DWF	NW	BID	Did	DW	MILL	DW	DW	OW	រាមរ		W	W	W		AP-
15	W	DWF	MG	DW	DW	DW	DW	DIN	DIN	DW (MI	D.M	DW	DW	DW		
16	W	DWF	DW	Du	DW	DIN	DU	DW	DIU	DUI	NWI	DW	心见	DU	DW		
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18						1 4											
19																	
20						C.E.											
21																	
22							5										
23 .																	
24																	
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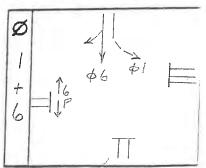
HOTES: "G" adds CD Pand A Logs.

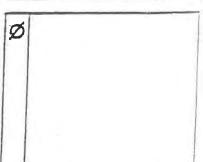
Adds \$8 PED siles Some soles.











SUBMITTED: CR 9-6-11	CHECKED: Elfanged 9/6/11	APPROVED: Kthimud 9/6/11
IN SERVICE BY: 789	DATE: 6/14/12	TIME: ()-37

C-29

Appendix C: Signal Timing Worksheets

IT.# Cabinet Type System								ECTI								TS
99 TS1 TSS						Blve	IM) k	0.191) and	d Bui	rdett	e Rd				A.
Pole Mount		Hub	bett	e:	EF						123	13				70
											1117					
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			1			D.			1	40	4118	10		1 -		m ²
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		O Post	5	Burdotto		Bradley		Burdette			1			1 = 1	100	
		1	8	1 2	3	2			150	100	1		e.	1	1 3	
		0,40	5					9						-	/ //E	
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IMING PLAN 1	27500									10		140	解音器	SALE.		
PHASE	1	-	-	4	5	-	7	8	9	10	11	12	13	14	15	16
MINIMUM GREEN	+	10	-	5	+	10	+	5	+	-	-	+	+	-	+	+
BICYCLE MINIMUM GREEN CONDITIONAL SERVICE MINIMUM GREEN	+	+	+	+	+-	+	-	-	+	+	+	-	-	-	+	+
DELAYED GREEN	+	-	+	-	+	+	+	+-	-	-	-	-	-	+-	+	+
DELATED GREEN		SUPER GO	WE WAR	SE DIV	NOT THE			ALTERNA .	section 100	100 2000屋			il stuSke	-	Senore	調賞
VALK	(日本)(大)(日本)	5	PACIFICA	T	NAME OF STREET	5	Unit 25 To	6		S STREET	THE PERSON NAMED IN	25 97	No.	SHEET BYTE	200	GHIMOS SOON
VALK 2	1	+	1	1	-	+	1	1	1							
VALK MAX	1	1		1	1		1				1				+	
EDESTRIAN CLEARANCE		5				5	1	6							1	
EDESTRIAN CLEARANCE 2																
EDESTRIAN CLEARANCE MAX				1									Y T			
EDESTRIAN CARRY OVER																
				有题		型馬2				150/00			1770			
EHICLE EXTENSION		6.0		3.0		6.0		6.0								
HICLE EXTENSION 2																
	2 Alle		结拟	\$413°		THE REAL PROPERTY.	, the	於首都	10000			de la		et e	1010	M
XX1		60		30		60		60								
X2	-	60		40		60		40								
4X3	-														-	\vdash
NAMIC MAX	-		-			-									-	
'NAMIC MAX STEP		100 M	2000 NATO	10 CO 12	- BENDER	E ASIE	2012/40	1	AUGUSTAS.	Ver. 80.04	900 1555	182 F 160	Lo seller le	01 41 de	100000	MULTINA.
	Tocobs.	里到型		中国				SE THE		178	200	m de Tar				
LLOW CHANGE D CLEARANCE	\vdash	1.0		3.5 1.5	-	1.0		3.5 1.5	-					-	-	-
D MAX	-	1.0	-	1.9	-	1.0		1.5	-							
D REVERT	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
	in although	diam's	L A FRO	能建特	国 原	397			With the state of		10/11920		F 550/A			
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XIMUM ADDED INITIAL GREEN																
E DECORE CAR REDUCTION													7 1,			
E BEFORE GAP REDUCTION	1		- X						- 1							
S WAITING BEFORE GAP REDUCTION																1. 6
		7				_										
RS WAITING BEFORE GAP REDUCTION																
S WAITING BEFORE GAP REDUCTION P TO REDUCE										- 1						

HATEL CIPERS HIMS SELECT

DIVISION OF TRAFFIC ENGINEERING MONTGOMERY COUNTY, MARYLAND NO.

INTERSECTION: BEADLEY BLUD. (MD. 191) - BURDETTE RD.

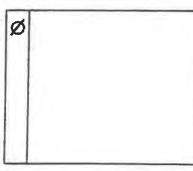
PHASING

		SIGN	AL HEAD	INDICATI	ONS	
SIGNAL NO.	2-5			1,6-9		
TOTAL:	4			5		
LEGEND OPTICALLY LIMITED R RED Y YELLOW G GREEN ARROW F FLASHING	(R) (Y) (G)	(R) (Y) (G) (G) (2 ^t)	(R) (Y) (G) (G)	(R) (Y) (G) 8"		9" or 12"

Ø	K 1	ORTH
2		
7-	1	052 A
6	500	->
6	BENDLEY BLUD.	,
	BLUD.	TAB
	le:	20

		SEQUENCE OF OPERATION															F
SIGNA		_						INTE	RVAL								ŝ
NO.	1	2	3	4	5	6	7	8	8	10	11	12	13	14	15	16	lH
1	G	Y	R	R	R	R	1										IY
2	G	Y	R	R	R	R											Y
3	G	Y	R	R	R	R											У
4	G	Y	R	R	R	R											Y
5	S	y	R	R	R	R	1										Y
6	R	R	R	G	У	R											R
7	R	R	R	6	y	R											R
8 .	R	R	R	6	Y	R	1					1	T				R
9	R	R	R	G	Y	R											R
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11.												1					
12																	
13																	
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24																	
HASE	\$2+	6	CC.	44	+8	RED			-								

Ø 8



NOTES:

SUBMITTED: TR 3/8/90 CHECKED: 82M 3/8/90 DATE: 4/14/90' IN SERVICE BY: SHA 10 Am TIME: C-31

APPROVED: 3/8/80

Higher Ground Education, Inc July 2021

APPENDIX D EXISTING INTERSECTION DELAYS, CLVs, and HCM WORKSHEETS

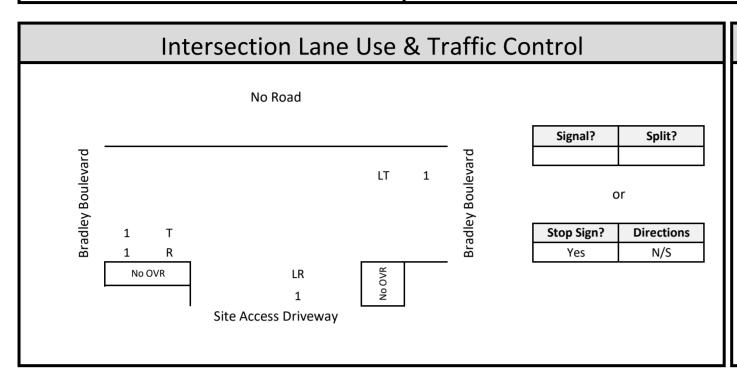
Critical Lane Volume and Level of Service Calculations

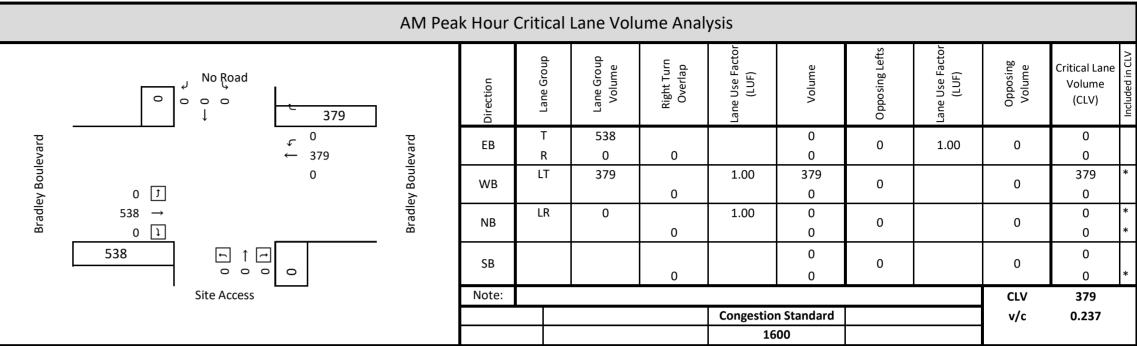
Intersection: Bradley Boulevard / Site Access Driveway

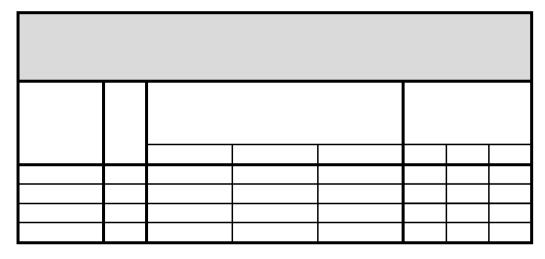
Jurisdiction: Montgomery County, MD

Scenario/Design Year: 2021 Adjusted Existing

Computed by: W+A

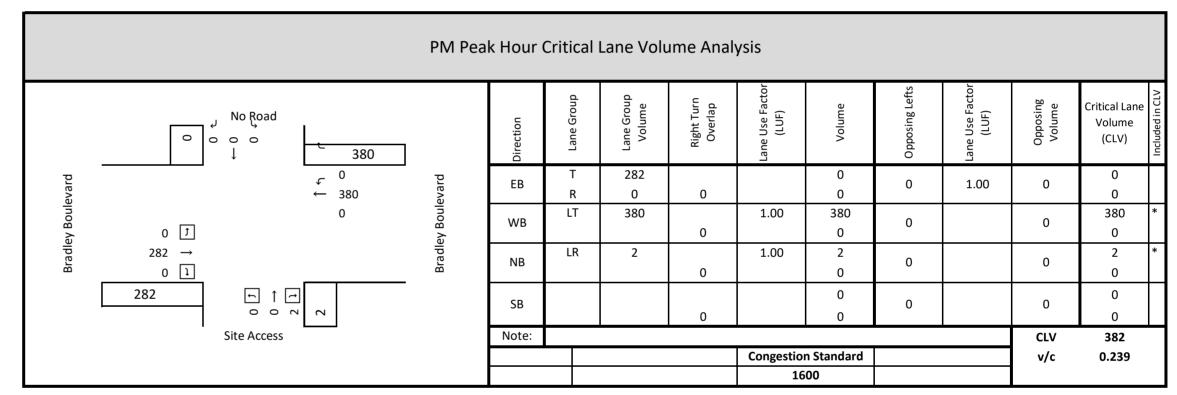






	Right Turn Overlap												
	I. Right		Right	t Vol.		Ad	djacent O	verlap V	ol.	Overlap			
Approach	Excl.	AM	PM		LUF	AM	PM		LUF	AM	PM		
Eastbound	No	n/a	n/a		n/a	n/a	n/a		n/a	0	0		
Westbound	No	n/a	n/a		n/a	n/a	n/a		n/a	0	0		
Northbound	No	n/a	n/a		n/a	n/a	n/a		n/a	0	0		
Southbound	No	n/a	n/a		n/a	n/a	n/a		n/a	0	0		

Montgomery County LATR												
	Number	Left Turn	Through									
	of Lanes	LUF	LUF									
	1	1	1.00									
	2	0.53	0.53									
	3	0.37	0.37									
	4		0.30									
	5		0.25									



·						

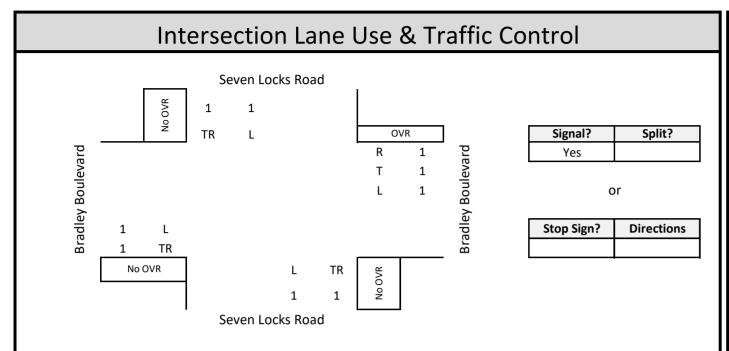
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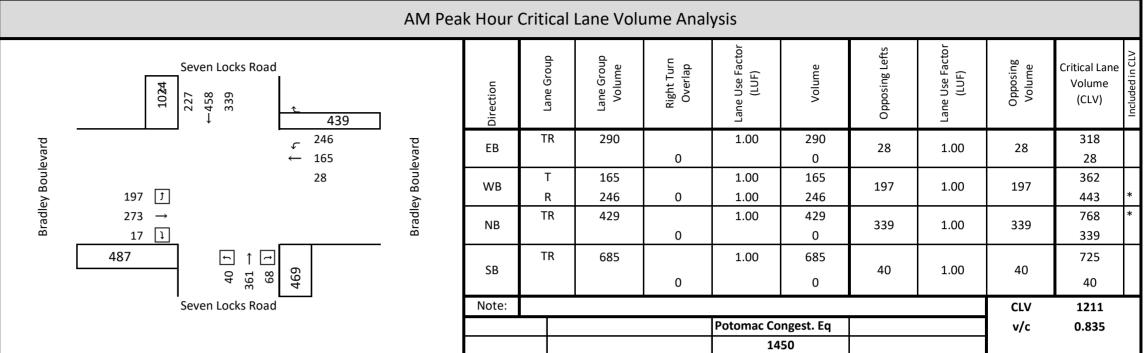
Critical Lane Volume and Level of Service Calculations Intersection: Bradley Boulevard / Seven Locks Road

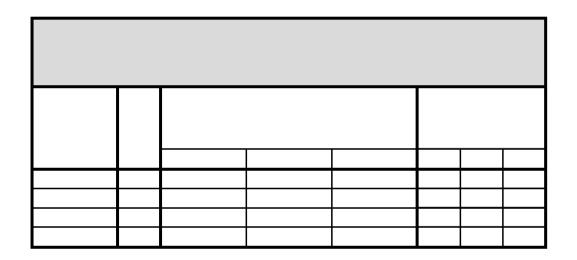
Jurisdiction: Montgomery County, MD

Scenario/Design Year: 2021 Adjusted Existing

Computed by: W+A







			R	ight	Tur	n Ov	/erla	р					
Right Vol. Adjacent Overlap Vol. Overlap													
Approach	Approach AM PM LUF AM PM LUF AM PM												
Eastbound	No	n/a	n/a		n/a	n/a	n/a		n/a	0	0		
Westbound	Yes	246	255		1.00	339	154		0.00	0	0		
Northbound	Northbound No n/a n/a n/a n/a n/a n/a 0 0												
Southbound	No	n/a	n/a		n/a	n/a	n/a		n/a	0	0		

M	ontgome	ry Coun	ty LATR											
Lane Use Factors														
	Number Left Turn Through of Lanes LUF LUF													
	of Lanes													
	1	1	1.00											
	2	0.53												
	3	0.37	0.37											
	4		0.30											
	5 0.25													

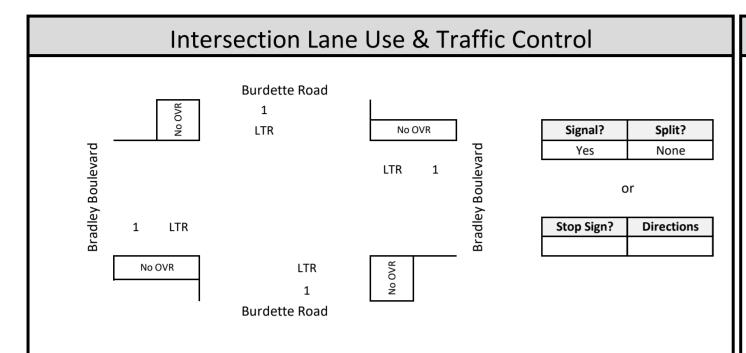
			PM Pea	k Hour (Critical	Lane Volu	ıme Analı	ysis					
	5985 15.95 15.47 Seveu Focks Board	455		Direction	Lane Group	Lane Group Volume	Right Turn Overlap	Lane Use Factor (LUF)	Volume	Opposing Lefts	Lane Use Factor (LUF)	Opposing Volume	Critical Lane Volume (CLV)
evard		<i>Ç</i> 255	evard	EB	TR	121	0	1.00	121 0	44	1.00	44	165 44
Bradley Boulevard	194 J	44	Bradley Boulevard	WB	T R	156 255	0	1.00 1.00	156 255	194	1.00	194	350 449 *
Bradle	110 → 11 1		Bradle	NB	TR	514	0	1.00	514 0	154	1.00	154	668 * 154
	315 t t t t t t t t t t t t t t t t t t t	525		SB	TR	441	0	1.00	441 0	11	1.00	11	452 11
	Seven Locks Road			Note:								CLV	1117
								Potomac Co	ngest. Eq			v/c	0.770
								14	50				

·						

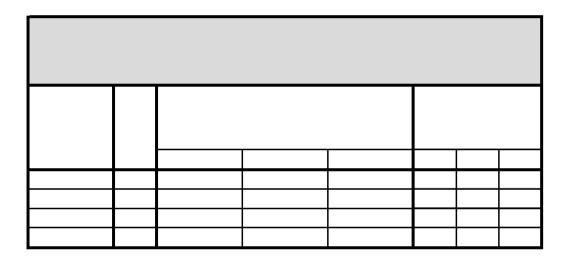
Critical Lane Volume

and **Level of Service Calculations**

Intersection: Bradley Boulevard / Burdette Road Jurisdiction: Montgomery County, MD Scenario/Design Year: 2021 Adjusted Existing Computed by: W+A



		AM Pe	ak Hour	Critical	Lane Volu	ıme Anal	ysis					
	Burdette Road 35 35 35	397	Direction	Lane Group	Lane Group Volume	Right Turn Overlap	Lane Use Factor (LUF)	Volume	Opposing Lefts	Lane Use Factor (LUF)	Opposing Volume	Critical Lane Volume (CLV)
evard	<u></u>	<u>~</u>	ЕВ	LTR	557	0	1.00	557 0	35	1.00	35	592 * 35
Bradley Boulevard	67 J	35 Inog Aa	WB	LTR	397	0	1.00	397 0	67	1.00	67	464 67
Bradle	471 → 19 l	Bradley I	NB	LTR	58	0	1.00	58 0	35	1.00	35	93 35
	557 01 12 282		SB	LTR	178	0	1.00	178 0	10	1.00	10	188 * 10
	Burdette Road		Note:								CLV	780
							Congestio	n Standard			v/c	0.488
					·	·	16	00				



			R	ight	Tur	n Ov	/erla	р					
	문 Right Vol. Adjacent Overlap Vol. Overlap												
Approach	Excl.	AM	PM LUF AM PM LUF AM PM										
Eastbound	No	n/a	n/a		n/a	n/a	n/a		n/a	0	0		
Westbound	No	n/a	n/a		n/a	n/a	n/a		n/a	0	0		
Northbound No n/a n/a n/a n/a n/a n/a 0 0													
Southbound													

Мо	Montgomery County LATR													
		Lane Use Factors	S											
	Number	Left Turn	Through											
	of Lanes	LUF	LUF											
	1	1	1.00											
	2	0.53	0.53											
	3	0.37	0.37											
	4		0.30											
5 0.25														
0.25														

			PM Peal	k Hour (Critical	Lane Volu	ıme Analı	ysis					
	Burdette Road 7 2 7 21 7	404		Direction	Lane Group	Lane Group Volume	Right Turn Overlap	Lane Use Factor (LUF)	Volume	Opposing Lefts	Lane Use Factor (LUF)	Opposing Volume	Critical Lane Volume (CLV)
evard		<i>⊊</i> 31	evard	EB	LTR	268	0	1.00	268 0	38	1.00	38	306 38
Bradley Boulevard	20 🗇	38	Bradley Boulevard	WB	LTR	404	0	1.00	404 0	20	1.00	20	424 * 20
Bradle	236 → 12 1		Bradle	NB	LTR	81	0	1.00	81 0	21	1.00	21	102 21
	268	81		SB	LTR	98	0	1.00	98 0	15	1.00	15	113 * 15
	Burdette Road	<u></u>		Note:								CLV	537
								Congestion	n Standard		·	v/c	0.336
i								16	00				

·						

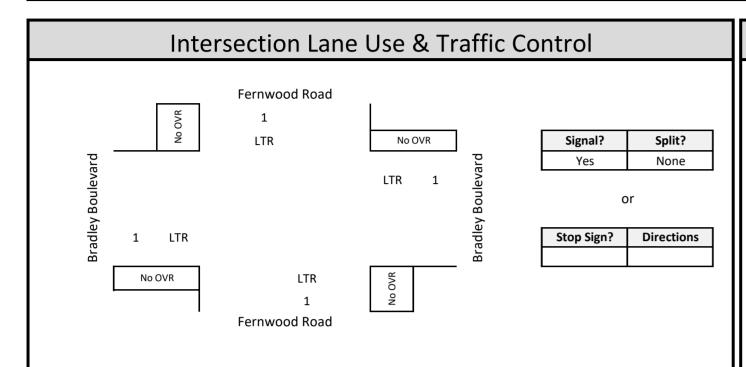
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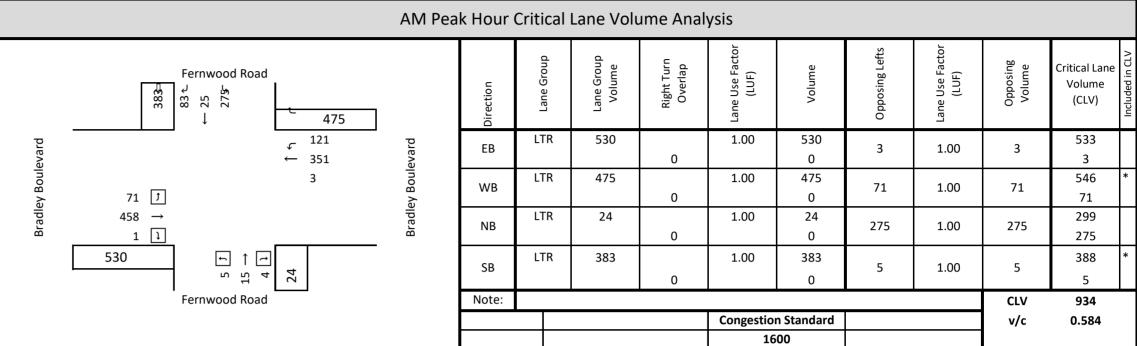
Critical Lane Volume and Level of Service Calculations Intersection: Bradley Boulevard / Fernwood Road

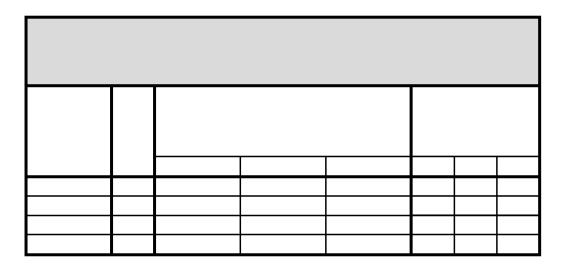
Jurisdiction: Montgomery County, MD

Scenario/Design Year: 2021 Adjusted Existing

Computed by: W+A

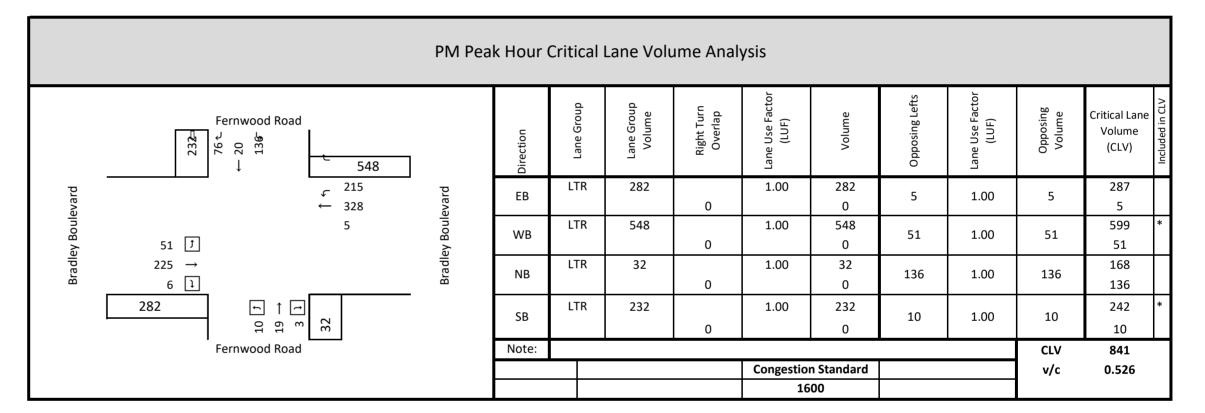






			R	ight	Tur	n O\	/erla	р						
	문 Right Vol. Adjacent Overlap Vol. Overlap													
Approach	Excl.	AM	PM LUF AM PM LUF AM PM											
Eastbound	No	n/a	n/a		n/a	n/a	n/a		n/a	0	0			
Westbound	No	n/a	n/a		n/a	n/a	n/a		n/a	0	0			
Northbound	Northbound No n/a n/a n/a n/a n/a n/a 0 0													
Southbound	No	n/a	n/a		n/a	n/a	n/a		n/a	0	0			

Mo	ntgome	ry Coun	ty LATR									
		Lane Use Factors	S									
	Number	Left Turn	Through									
	of Lanes	LUF	LUF									
	1	1	1.00									
	2	0.53	0.53									
	3	0.37	0.37									
	4		0.30									
5 0.25												



l						

	-	\rightarrow	•	←	•	~
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	†	1		4	W	
Traffic Volume (veh/h)	538	0	0	379	0	0
Future Volume (Veh/h)	538	0	0	379	0	0
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	598	0	0	421	0	0
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			598		1019	598
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			598		1019	598
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			100		100	100
cM capacity (veh/h)			979		263	502
	ED 1	ED 3		ND 1		
Direction, Lane #	EB 1	EB 2	WB 1	NB 1		
Volume Total	598	0	421	0		
Volume Left	0	0	0	0		
Volume Right	1700	1700	0	0		
cSH	1700	1700	979	1700		
Volume to Capacity	0.35	0.00	0.00	0.00		
Queue Length 95th (ft)	0	0	0	0		
Control Delay (s)	0.0	0.0	0.0	0.0		
Lane LOS	0.0		0.0	A		
Approach Delay (s)	0.0		0.0	0.0		
Approach LOS				А		
Intersection Summary						
Average Delay			0.0			
Intersection Capacity Utiliza	ation		31.6%	IC	U Level c	f Service
Analysis Period (min)			15			

3: Burdette Road & Bradley Boulevard #191

06/08/2021

	۶	→	•	•	+	•	1	†	~	/	+	✓
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (vph)	67	471	19	35	337	25	10	31	17	35	67	76
Future Volume (vph)	67	471	19	35	337	25	10	31	17	35	67	76
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	12	12	12	12	12	12	12	13	12	12	14	12
Total Lost time (s)		5.0			5.0			5.0			5.0	
Lane Util. Factor		1.00			1.00			1.00			1.00	
Frt		1.00			0.99			0.96			0.94	
Flt Protected		0.99			1.00			0.99			0.99	
Satd. Flow (prot)		1843			1839			1833			1854	
Flt Permitted		0.90			0.92			0.93			0.92	
Satd. Flow (perm)		1675			1704			1719			1724	
Peak-hour factor, PHF	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89
Adj. Flow (vph)	75	529	21	39	379	28	11	35	19	39	75	85
RTOR Reduction (vph)	0	1	0	0	1	0	0	16	0	0	34	0
Lane Group Flow (vph)	0	624	0	0	445	0	0	49	0	0	165	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		6			2			8			4	
Permitted Phases	6			2			8			4		
Actuated Green, G (s)		55.1			55.1			12.6			12.6	
Effective Green, g (s)		55.1			55.1			12.6			12.6	
Actuated g/C Ratio		0.71			0.71			0.16			0.16	
Clearance Time (s)		5.0			5.0			5.0			5.0	
Vehicle Extension (s)		6.0			6.0			6.0			3.0	
Lane Grp Cap (vph)		1187			1208			278			279	
v/s Ratio Prot												
v/s Ratio Perm		c0.37			0.26			0.03			c0.10	
v/c Ratio		0.53			0.37			0.18			0.59	
Uniform Delay, d1		5.2			4.4			28.1			30.2	
Progression Factor		1.00			1.00			1.00			1.00	
Incremental Delay, d2		1.1			0.9			0.9			3.3	
Delay (s)		6.3			5.3			28.9			33.5	
Level of Service		A			A			C			C	
Approach Delay (s)		6.3			5.3			28.9			33.5	
Approach LOS		А			Α			С			С	
Intersection Summary												
HCM 2000 Control Delay			11.1	H	CM 2000	Level of	Service		В			
HCM 2000 Volume to Capacity	y ratio		0.54									
Actuated Cycle Length (s)			77.7		um of los				10.0			
Intersection Capacity Utilization	n		65.4%	IC	U Level	of Service			С			
Analysis Period (min)			15									
c Critical Lane Group												

4: Fernwood Road & Bradley Boulevard #191

06/08/2021

	۶	→	•	•	+	•	1	†	~	/	+	✓
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (vph)	71	458	1	3	351	121	5	15	4	275	25	83
Future Volume (vph)	71	458	1	3	351	121	5	15	4	275	25	83
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	12	11	12	12	11	12	12	12	12	12	12	12
Total Lost time (s)		5.0			5.0			5.5			5.5	
Lane Util. Factor		1.00			1.00			1.00			1.00	
Frt		1.00			0.97			0.98			0.97	
Flt Protected		0.99			1.00			0.99			0.97	
Satd. Flow (prot)		1788			1738			1804			1746	
Flt Permitted		0.88			1.00			0.93			0.77	
Satd. Flow (perm)		1583			1735			1694			1395	
Peak-hour factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Adj. Flow (vph)	78	503	1	3	386	133	5	16	4	302	27	91
RTOR Reduction (vph)	0	0	0	0	9	0	0	3	0	0	12	0
Lane Group Flow (vph)	0	582	0	0	513	0	0	22	0	0	408	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		6			2			8			4	
Permitted Phases	6			2			8			4		
Actuated Green, G (s)		45.5			45.5			30.1			30.1	
Effective Green, g (s)		45.5			45.5			30.1			30.1	
Actuated g/C Ratio		0.53			0.53			0.35			0.35	
Clearance Time (s)		5.0			5.0			5.5			5.5	
Vehicle Extension (s)		3.0			3.0			3.0			3.0	
Lane Grp Cap (vph)		836			916			592			487	
v/s Ratio Prot												
v/s Ratio Perm		c0.37			0.30			0.01			c0.29	
v/c Ratio		0.70			0.56			0.04			0.84	
Uniform Delay, d1		15.1			13.6			18.5			25.8	
Progression Factor		1.00			1.00			1.00			1.00	
Incremental Delay, d2		2.5			2.5			0.0			12.0	
Delay (s)		17.7			16.1			18.5			37.7	
Level of Service		В			В			B			D	
Approach Delay (s)		17.7			16.1			18.5			37.7	
Approach LOS		В			В			В			D	
Intersection Summary												
HCM 2000 Control Delay			22.6	Н	CM 2000	Level of	Service		С			
HCM 2000 Volume to Capacity	/ ratio		0.75									
Actuated Cycle Length (s)			86.1		um of los				10.5			
Intersection Capacity Utilization	n		95.3%	IC	CU Level	of Service			F			
Analysis Period (min)			15									
c Critical Lane Group												

	→	•	•	←	•	<i>></i>
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<u> </u>	7		4	¥	
Traffic Volume (veh/h)	282	0	0	380	0	2
Future Volume (Veh/h)	282	0	0	380	0	2
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94
Hourly flow rate (vph)	300	0	0	404	0	2
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			300		704	300
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			300		704	300
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			100		100	100
cM capacity (veh/h)			1261		403	740
Direction, Lane #	EB 1	EB 2	WB 1	NB 1		
Volume Total	300	0	404	2		
Volume Left	0	0	0	0		
Volume Right	0	0	0	2		
cSH	1700	1700	1261	740		
Volume to Capacity	0.18	0.00	0.00	0.00		
Queue Length 95th (ft)	0	0	0	0		
Control Delay (s)	0.0	0.0	0.0	9.9		
Lane LOS				Α		
Approach Delay (s)	0.0		0.0	9.9		
Approach LOS				А		
Intersection Summary						
Average Delay			0.0			
Intersection Capacity Utilizat	tion		30.0%	IC	U Level c	f Service
Analysis Period (min)			15	10	2 200010	

3: Burdette Road & Bradley Boulevard #191

06/08/2021

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (vph)	20	236	12	38	335	31	15	29	37	21	32	45
Future Volume (vph)	20	236	12	38	335	31	15	29	37	21	32	45
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	12	12	12	12	12	12	12	13	12	12	14	12
Total Lost time (s)		5.0			5.0			5.0			5.0	
Lane Util. Factor		1.00			1.00			1.00			1.00	
Frt		0.99			0.99			0.94			0.94	
Flt Protected		1.00			1.00			0.99			0.99	
Satd. Flow (prot)		1845			1835			1790			1844	
Flt Permitted		0.96			0.95			0.93			0.93	
Satd. Flow (perm)		1777			1757			1688			1727	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	22	257	13	41	364	34	16	32	40	23	35	49
RTOR Reduction (vph)	0	1	0	0	2	0	0	35	0	0	42	0
Lane Group Flow (vph)	0	291	0	0	437	0	0	53	0	0	65	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		6			2			8			4	
Permitted Phases	6			2			8			4		
Actuated Green, G (s)		55.6			55.6			8.6			8.6	
Effective Green, g (s)		55.6			55.6			8.6			8.6	
Actuated g/C Ratio		0.75			0.75			0.12			0.12	
Clearance Time (s)		5.0			5.0			5.0			5.0	
Vehicle Extension (s)		6.0			6.0			6.0			3.0	
Lane Grp Cap (vph)		1331			1316			195			200	
v/s Ratio Prot												
v/s Ratio Perm		0.16			c0.25			0.03			c0.04	
v/c Ratio		0.22			0.33			0.27			0.33	
Uniform Delay, d1		2.8			3.1			29.9			30.1	
Progression Factor		1.00			1.00			1.00			1.00	
Incremental Delay, d2		0.2			0.7			2.1			1.0	
Delay (s)		3.0			3.8			32.0			31.1	
Level of Service		Α			Α			С			С	
Approach Delay (s)		3.0			3.8			32.0			31.1	
Approach LOS		А			А			С			С	
Intersection Summary												
HCM 2000 Control Delay			9.4	H	CM 2000	Level of S	Service		Α			
HCM 2000 Volume to Capacity	y ratio		0.33									
Actuated Cycle Length (s)			74.2	Sı	um of lost	time (s)			10.0			
Intersection Capacity Utilizatio	n		46.3%	IC	CU Level	of Service	1		А			
Analysis Period (min)			15									
c Critical Lane Group												

4: Fernwood Road & Bradley Boulevard #191

06/08/2021

	۶	→	•	•	←	•	•	†	~	\		√
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (vph)	51	225	6	5	328	215	10	19	3	136	20	76
Future Volume (vph)	51	225	6	5	328	215	10	19	3	136	20	76
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	12	11	12	12	11	12	12	12	12	12	12	12
Total Lost time (s)		5.0			5.0			5.5			5.5	
Lane Util. Factor		1.00			1.00			1.00			1.00	
Frt		1.00			0.95			0.99			0.96	
Flt Protected		0.99			1.00			0.99			0.97	
Satd. Flow (prot)		1779			1704			1812			1730	
Flt Permitted		0.85			1.00			0.90			0.80	
Satd. Flow (perm)		1533			1701			1662			1426	
Peak-hour factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Adj. Flow (vph)	53	232	6	5	338	222	10	20	3	140	21	78
RTOR Reduction (vph)	0	0	0	0	13	0	0	20	0	0	24	0
Lane Group Flow (vph)	0	291	0	0	552	0	0	31	0	0	215	0
. , ,		NA	U	Perm	NA	U		NA	U	Perm	NA	0
	Perm			Pellii	NA 2		Perm			Pellii		
Protected Phases Permitted Phases	/	6		2	2		0	8		1	4	
	6	45.0		2	45.0		8	157		4	157	
Actuated Green, G (s)		45.2			45.2			15.7			15.7	
Effective Green, g (s)		45.2			45.2			15.7			15.7	
Actuated g/C Ratio		0.63			0.63			0.22			0.22	
Clearance Time (s)		5.0			5.0			5.5			5.5	
Vehicle Extension (s)		3.0			3.0			3.0			3.0	
Lane Grp Cap (vph)		970			1076			365			313	
v/s Ratio Prot												
v/s Ratio Perm		0.19			c0.32			0.02			c0.15	
v/c Ratio		0.30			0.51			0.08			0.69	
Uniform Delay, d1		5.9			7.1			22.1			25.6	
Progression Factor		1.00			1.00			1.00			1.00	
Incremental Delay, d2		0.2			1.7			0.1			6.1	
Delay (s)		6.1			8.9			22.2			31.7	
Level of Service		Α			Α			С			С	
Approach Delay (s)		6.1			8.9			22.2			31.7	
Approach LOS		Α			Α			С			С	
Intersection Summary												
HCM 2000 Control Delay			13.4	H	CM 2000	Level of	Service		В			
HCM 2000 Volume to Capacity	ratio		0.56									
Actuated Cycle Length (s)			71.4	Sı	um of lost	time (s)			10.5			
Intersection Capacity Utilization			76.5%		CU Level of		;		D			
Analysis Period (min)			15									
c Critical Lane Group												

Higher Ground Education, Inc July 2021

APPENDIX E TRAFFIC FORECASTING WORKSHEETS

		1	. Bradley Bou	levard / S	Site Acces	s Drivew	ay									
				Sc	outhboun	d	W	estbound/	ł	١	Iorthbou	nd	E	astboun	d	
					No Road SBR SBT SBL V		Bradl	ey Boule	/ard	Site A	ccess Dri	veway	Brad	ley Boule	evard	
Component	Period	Inbound	Outbound	SBR	SBT	SBL	WBR	WBT	WBL	NBR	NBT	NBL	EBR	EBT	EBL	Total
Pipeline Trip Distributions																
1. WMAL Property - Residential		(+) Pos	(-) Neg					-3%						3%		1
2. Andrus Property - Residential		(+) Pos	(-) Neg					-3%						3%		
Site Trip Distribution																
Daycare Site Trips									60%	-60%		-40%	40%			

		1	Bradley Bo	ulevard /	Site Acce	ss Drivew	vay									
				S	outhbour	nd	V	Vestboun	d	١	Northbou	ınd		Eastboun	d	
					No Road		+	lley Boule		1	Access Dr		1	lley Boule		4
Component	Period	IN	OUT	SBR	SBT	SBL	WBR	WBT	WBL	NBR	NBT	NBL	EBR	EBT	EBL	Total
Existing 2021 Traffic Volumes	AM			0	0	0	0	354	0	0	0	0	0	503	0	857
	PM			0	0	0	0	380	0	0	0	0	0	282	0	662
Adjustments to Existing	AM		.07	0	0	0	0	25	0	0	0	0	0	35	0	60
, ,	PM	1.00 (afte	er 4:30pm)	0	0	0	0	0	0	0	0	0	0	0	0	0
Adjusted Existing	AM			0	0	0	0	379	0	0	0	0	0	538	0	917
,	PM			0	0	0	0	380	0	0	0	0	0	282	0	662
Pipeline Development			1.00				1 -			T .						
1. WMAL Property - Residential	AM	46	163	0	0	0	0	5	0	0	0	0	0	1 -	0	6
2 A. J. Brand Britania	PM	176	95	0	0	0	0	3	0	0	0	0	0	5	0	8
2. Andrus Property - Residential	AM	7	10	0	0	0	0	0	0	0	0	0	0	0	0	0
Binolina Cultatal	PM		4	0	0	0	0	0	0	0	0	0	0	0	0	0
Pipeline Subtotal	AM PM	50 183	173 99	0	0	0	0	5 3	0	0	0	0	0	1 5	0	6 8
Evicting Trin Credits (ANA Book Hour Only)	AM		22	0	0	0	0		21		0		14			57
Existing Trip Credits (AM Peak Hour Only)	PM	35 0	0		0	0		0	0	13	0	9	0	0	0	0
Background Future Forecast	AM	U	U	0 0	0	0	0	384	21	13	0	9	14	5 39	0	980
Background Future Forecast	PM			0	0	0	0	383	0	0	0	0	0	287	0	670
Daycare Site Traffic	AM	76	63	0	0	0	0	0	46	38	0	25	30	0	0	139
Daycare Site Traffic	PM	53	60	0	0	0	0	0	32	36	0	24	21	0	0	113
Total Future Traffic Forecasts	AM	33	00	0	0	0	0	384	46	38	0	25	30	539	0	1062
Total ruture frame rolecasts	PM			0	0	0	0	383	32	36	0	24	21	287	0	783
	1 141			 				303	<u> </u>	30				207		0
																0
																0
																0
Figure 7				0	0	0	0	5	21	13	0	9	14	1	0	63
				0	0	0	0	3	0	0	0	0	0	5	0	8
																0
																0
																0
																0
1998 Traffic Volumes	AM			0	0	0	0	306	20	16	0	6	15	870	0	1233
	PM			0	0	0	0	531	3	7	0	2	1	258	0	802
2021 vs 1998	AM			0	0	0	0	73	-20	-16	0	-6	-15	-332	0	-316
	PM			0	0	0	0	-151	-3	-7	0	-2	-1	24	0	-140

		2. Bradley	Boulevard /	Seven I	Locks Ro	ad										
				So	uthbour	nd	W	estbour/	nd	No	rthbou	nd	Е	astboun	d	
						Brad	ey Boul	evard	Sever	Locks	Road	Bradl	ey Boule	evard		
Component	Period	Inbound	Outbound	SBR	SBT	SBL	WBR	WBT	WBL	NBR	NBT	NBL	EBR	EBT	EBL	Total
Pipeline Trip Distributions																
1. WMAL Property - Residential		(+) Pos	(-) Neg			1%	-1%	-1%	-1%	1%				1%		
2. Andrus Property - Residential		(+) Pos	(-) Neg			1%	-1%	-1%	-1%	1%				1%		
Daycara Sita Trins		(+) Pos	() Nog			15%	-15%	-15%	-10%	10%				15%		
Daycare Site Trips		(+) POS	(-) Neg			1370	-13%	-13%	-10%	10%				1370		<u> </u>

		2. Bradley	Boulevard /	/ Seven	Locks Ro	oad										
				Sc	uthbou	nd	W	estbou/	nd	No	rthbou	nd	E	astbour	nd	
					n Locks			ey Boul			1 Locks			ley Boul		
Component	Period	IN	OUT	SBR	SBT	SBL	WBR	WBT	WBL	NBR	NBT	NBL	EBR	EBT	EBL	Total
Existing 2021 Traffic Volumes	AM			212	428	317	230	154	26	64	337	37	16	255	184	2260
	PM			145	267	144	238	146	41	38	442	10	10	103	181	1765
Adjustments to Existing	AM		07	15	30	22	16	11	2	4	24	3	1	18	13	159
	PM	1.07 (beto	re 4:30pm)	10	19	10	17	10	3	3	31	1	1	7	13	125
Adjusted Existing	AM			227	458	339	246	165	28	68	361	40	17	273	197	2419
	PM			155	286	154	255	156	44	41	473	11	11	110	194	1890
Pipeline Development		1	4.00					_					•		_	
1. WMAL Property - Residential	AM	46	163	0	0	0	2	2	1	0	0	0	0	1	0	6
2 Andrew Brancher Besidential	PM	176	95	0	0	2	1	1	1	1	0	0	0	2	0	8
2. Andrus Property - Residential	AM	4	10	0	0	0	0	0	0	0	0	0	0	0	0	0
Direction College	PM	7	4	0	0	0	0	0	0	0	0	0	0	0	0	0
Pipeline Subtotal	AM	50	173 99	0	0	0	2	2	1	0	0	0	0	1	0	6
Existing Trip Credits (ANA Book Hour Only)	PM	183 35	22	0	0	<u>2</u> 5	3	3	1	1 4	0	0	0	<u>2</u> 5	0	8 23
Existing Trip Credits (AM Peak Hour Only)	AM PM	0	0	0	0	0	0	0	3 0	0	0	0	0	0	0	0
Background Future Forecast	AM	U	U	227	458	344	251	170	32	72	361	40	17	279	197	2448
Background Future Forecast	PM			155	286	156	256	157	45	42	473	11	11	112	194	1898
Daycare Site Traffic	AM	76	63	0	0	11	9	9	6	8	0	0	0	11	0	54
	PM	53	60	0	0	8	9	9	6	5	0	0	0	8	0	45
Total Future Traffic Forecasts	AM			227	458	350	257	176	35	76	361	40	17	285	197	2479
	PM			155	286	164	265	166	51	47	473	11	11	120	194	1943
																0
																0
																0
																0
				0	0	5	5	5	4	4	0	0	0	6	0	29
				0	0	2	1	1	1	1	0	0	0	2	0	8
																0
																0
																0
																0
1998 Traffic Volumes	AM			160	687	565	165	122	22	69	303	18	27	311	192	2641
	PM			134	392	152	372	175	53	37	937	18	9	100	129	2508
2021 vs 1998	AM			67	-229	-226	81	43	6	-1	58	22	-10	-38	5	-222
	PM			21	-106	2	-117	-19	-9	4	-464	-7	2	10	65	-618

		3. Br	adley Boulev	ard / Bu	rdette R	oad										
				Sc	outhbour	ıd	W	'estbour	nd	No	rthbour	nd	Е	astboun	d	
				Burdette Road Bradley Boulevard E		Bur	dette Ro	oad	Bradl	ley Boule	evard					
Component	Period	Inbound	Outbound	SBR	SBT	SBL	WBR	WBT	WBL	NBR	NBT	NBL	EBR	EBT	EBL	Total
Pipeline Trip Distributions																
1. WMAL Property - Residential		(+) Pos	(-) Neg	-3%		-3%	3%								3%	
2. Andrus Property - Residential		(+) Pos	(-) Neg	-3%		-3%	3%								3%	
Daycare Site Trips		(+) Pos	(-) Neg					60%						-60%		

		3. Bra	adley Boulev	ard / Bu	ırdette R	oad										
				Sc	outhbou	nd	W	'estbour	nd	No	orthbou	nd	Е	astboun	t	
				Bui	rdette Ro	oad	Bradl	ey Boule	evard	Bur	rdette R	oad	Brad	ley Boule	vard	
Component	Period	IN	OUT	SBR	SBT	SBL	WBR	WBT	WBL	NBR	NBT	NBL	EBR	EBT	EBL	Total
Existing 2021 Traffic Volumes	AM			71	63	33	23	315	33	16	29	9	18	440	63	1113
Existing 2021 Traine volumes	PM			45	32	21	31	335	38	37	29	15	12	236	20	851
Adjustments to Existing	AM	1.0	07	5	4	2	2	22	2	1	2	1	1	31	4	77
Adjustifients to Existing	PM	1.00 (at/aft	er 4:30pm)	0	0	0	0	0	0	0	0	0	0	0	0	0
Adjusted Existing	AM			76	67	35	25	337	35	17	31	10	19	471	67	1190
Aujusteu Existing	PM			45	32	21	31	335	38	37	29	15	12	236	20	851
Pipeline Development																
1. WMAL Property - Residential	AM	46	163	5	0	5	1	0	0	0	0	0	0	0	1	12
	PM	176	95	3	0	3	5	0	0	0	0	0	0	0	5	16
2. Andrus Property - Residential	AM	4	10	0	0	0	0	0	0	0	0	0	0	0	0	0
	PM	7	4	0	0	0	0	0	0	0	0	0	0	0	0	0
Pipeline Subtotal	AM	50	173	5	0	5	1	0	0	0	0	0	0	0	1	12
	PM	183	99	3	0	3	5	0	0	0	0	0	0	0	5	16
Existing Trip Credits (AM Peak Hour Only)	AM	35	22	0	0	0	0	21	0	0	0	0	0	13	0	34
	PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Background Future Forecast	AM			81	67	40	26	358	35	17	31	10	19	484	68	1236
	PM			48	32	24	36	335	38	37	29	15	12	236	25	867
Daycare Site Traffic	AM	76	63	0	0	0	0	46	0	0	0	0	0	38	0	84
	PM	53	60	0	0	0	0	32	0	0	0	0	0	36	0	68
Total Future Traffic Forecasts	AM			81	67	40	26	383	35	17	31	10	19	509	68	1286
	PM			48	32	24	36	367	38	37	29	15	12	272	25	935
																0
																0
																0
																0
				5	0	5	1	21	0	0	0	0	0	13	1	46
				3	0	3	5	0	0	0	0	0	0	0	5	16
																0
																0
																0
																0
1998 Traffic Volumes	AM			45	43	54	13	257	28	25	38	14	8	870	56	1451
	PM			42	29	19	28	540	25	48	108	23	3	253	22	1140
2021 vs 1998	AM			31	24	-19	12	80	7	-8	-7	-4	11	-399	11	-261
	PM			3	3	2	3	-205	13	-11	-79	-8	9	-17	-2	-289

		4. E	Bradley Boule	evard / F	ernwoo	4. Bradley Boulevard / Fernwood Road														
				Southbound		١	<i>N</i> estbour	nd	No	orthboun	d	E	astbound	d						
				Fern	Fernwood Road		Brac	dley Boul	evard	Fern	wood Ro	ad	Bradl	ey Boule	vard					
Component	Period	Inbound	Outbound	SBR	SBT	SBL	WBR	WBT	WBL	NBR	NBT	NBL	EBR	EBT	EBL	Total				
Pipeline Trip Distributions																				
1. WMAL Property - Residential		(+) Pos	(-) Neg			-10%	10%	3%						-3%						
2. Andrus Property - Residential		(+) Pos	(-) Neg			-10%	10%	3%						-3%						
Daycare Site Trips		(+) Pos	(-) Neg					60%						-60%						

		4. E	Bradley Boul	evard / F	ernwoo	d Road										
				Sc	outhbou	nd	1	Westbou	nd	N	orthbour	nd	E	astboun	d	
				-	nwood F			dley Boul			nwood R			ley Boule		
Component	Period	IN	OUT	SBR	SBT	SBL	WBR	WBT	WBL	NBR	NBT	NBL	EBR	EBT	EBL	Total
Existing 2021 Traffic Volumes	AM			78	23	257	113	328	3	4	14	5	1	428	66	1320
0	PM			76	20	136	215	328	5	3	19	10	6	225	51	1094
Adjustments to Existing	AM	1.07		5	2	18	8	23	0	0	1	0	0	30	5	92
, , , , , , , , , , , , , , , , , , , ,	PM	1.00 (at/after	r 4:30pm)	0	0	0	0	0	0	0	0	0	0	0	0	0
Adjusted Existing	AM			83	25	275	121	351	3	4	15	5	1	458	71	1412
	PM			76	20	136	215	328	5	3	19	10	6	225	51	1094
Pipeline Development	0.04	4.6	1.62	Ι ο		1.0	I -						I 0			27
1. WMAL Property - Residential	AM	46	163	0	0	16	5	1	0	0	0	0	0	5	0	27
2 Andrus Proporty Posidontial	PM	176	95 10	0	0	10	18	5	0	0	0	0	0	3	0	36
2. Andrus Property - Residential	AM PM	7	4	0	0	1	0	0	0	0	0	0	0	0	0	1
Pipeline Subtotal	AM	50	173	0	0	0 17	5	0	0	-	0	0	0	5	0	28
Pipeline Subtotal	PM	183	99	0	0	10	19	5	0	0	0	0	0	3	0	37
Existing Trip Credits (AM Peak Hour Only)	AM	35	22	0	0	0	0	21	0	0	0	0	0	13	0	34
LXISTING THE CIEUTES (AND FEAR HOU! Only)	PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Background Future Forecast	AM	0		83	25	292	126	373	3	4	15	5	1	476	71	1474
Background rature rorecast	PM			76	20	146	234	333	5	3	19	10	6	228	51	1131
Daycare Site Traffic	AM	76	63	0	0	0	0	46	0	0	0	0	0	38	0	84
bayeare site maine	PM	53	60	0	0	0	0	32	0	0	0	0	0	36	0	68
Total Future Traffic Forecasts	AM	33		83	25	292	126	398	3	4	15	5	1	501	71	1524
Total ratal of manner of course	PM			76	20	146	234	365	5	3	19	10	6	264	51	1199
				1												0
																0
																0
																0
				0	0	17	5	22	0	0	0	0	0	18	0	62
				0	0	10	19	5	0	0	0	0	0	3	0	37
																0
																0
																0
																0
1998 Traffic Volumes	AM			85	33	351	203	246	1	0	53	0	0	668	115	1755
	PM			90	45	320	302	453	3	3	30	1	0	231	79	1557
2021 vs 1998	AM			-2	-8	-76	-82	105	2	4	-38	5	1	-210	-44	-343
	PM			-14	-25	-184	-87	-125	2	0	-11	9	6	-6	-28	-463

Higher Ground Education, Inc July 2021

APPENDIX F BACKGROUND INTERSECTION DELAYS, CLVs, and HCM WORKSHEETS

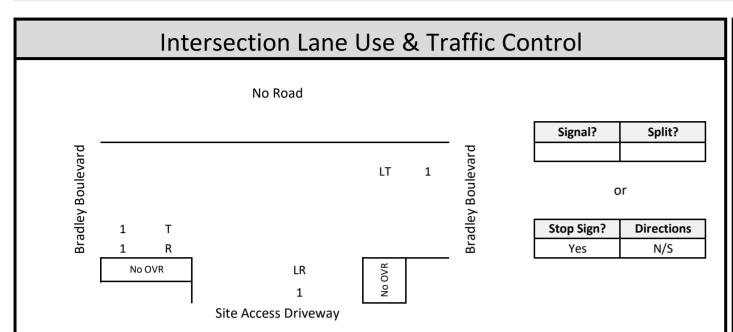
Critical Lane Volume
and
Level of Service Calculations

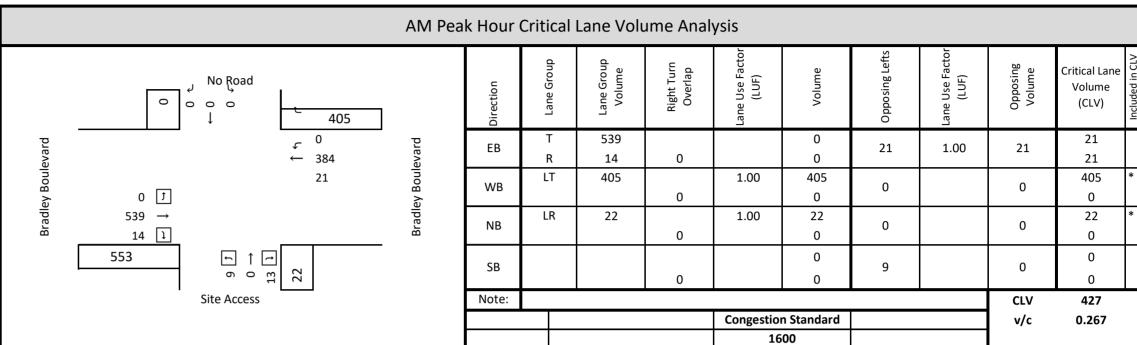
Intersection: Bradley Boulevard / Site Access Driveway

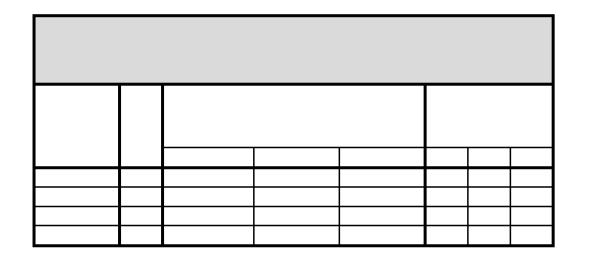
Jurisdiction: Montgomery County, MD

Scenario/Design Year: Background Future

Computed by: W+A

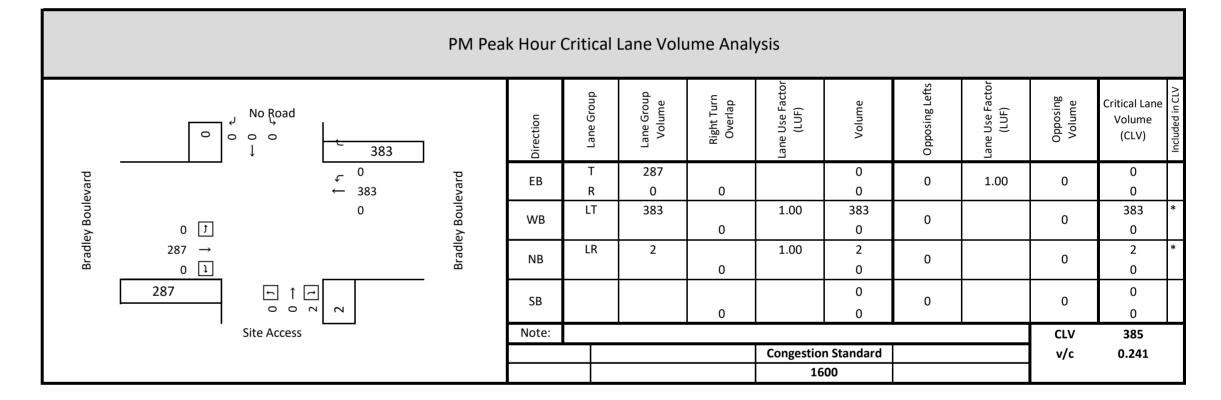






	Right Turn Overlap														
	:l. Right		Right	Vol.		Ad	djacent O	verlap Vo	ol.		Overlap				
Approach	Excl.	AM PM LUF				AM	PM		LUF	AM	PM				
Eastbound	No	n/a	n/a		n/a	n/a	n/a		n/a	0	0				
Westbound	No	n/a	n/a		n/a	n/a	n/a		n/a	0	0				
Northbound	No	n/a	n/a		n/a	n/a	n/a		n/a	0	0				
Southbound	No	n/a	n/a		n/a	n/a	n/a		n/a	0	0				

Мо	ntgome	ry Coun	ty LATR		
		Lane Use Factor	S		
	Number	Left Turn	Through		
	of Lanes	LUF	LUF		
	1	1	1.00		
	2	0.53	0.53		
	3	0.37	0.37		
	4		0.30		
	5		0.25		



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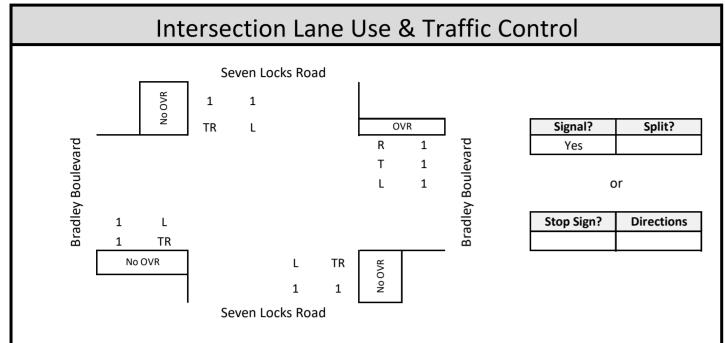
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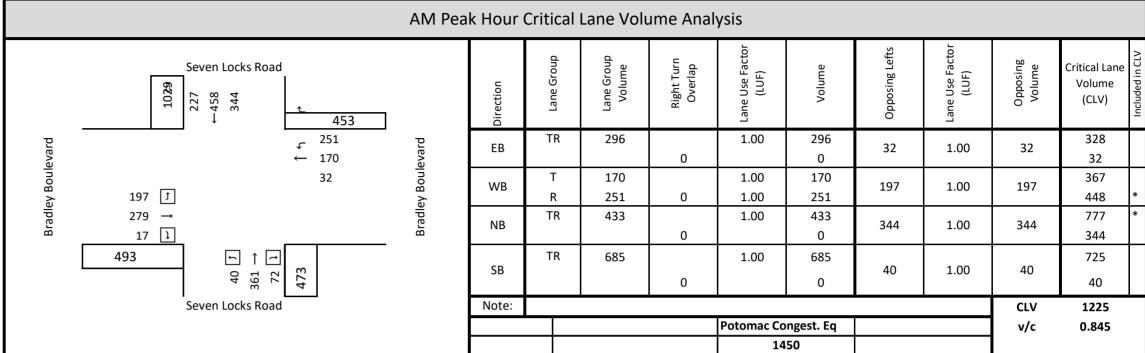
Critical Lane Volume and Level of Service Calculations Intersection: Bradley Boulevard / Seven Locks Road

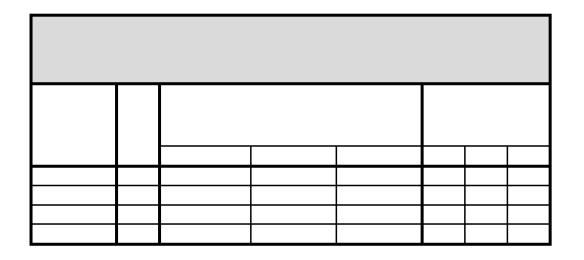
Jurisdiction: Montgomery County, MD

Scenario/Design Year: Background Future

Computed by: W+A







		Right Turn Overlap													
	Excl. Right		Right	Vol.		Ad	djacent O	verlap V	ol.		Overlap				
Approach	Exc	AM				AM	PM		LUF	AM	PM				
Eastbound	No	n/a	n/a		n/a	n/a	n/a		n/a	0	0				
Westbound	Yes	251	256		1.00	344	156		0.00	0	0				
Northbound	No	n/a	n/a		n/a	n/a	n/a		n/a	0	0				
Southbound	No	n/a	n/a		n/a	n/a	n/a		n/a	0	0				

Mo	ntgome	ry Count	ty LATR		
		Lane Use Factor	s		
	Number	Left Turn	Through		
	of Lanes	LUF	LUF		
	1	1	1.00		
	2	0.53	0.53		
	3	0.37	0.37		
	4		0.30		
	5		0.25		

			PM Pea	k Hour (Critical	Lane Volu	ıme Anal	ysis						
	Seven Tocks Boar	d 458		Direction	Lane Group	Lane Group Volume	Right Turn Overlap	Lane Use Factor (LUF)	Volume	Opposing Lefts	Lane Use Factor (LUF)	Opposing Volume	Critical Lane Volume (CLV)	Included in CLV
evard		<i>⊊</i> 256 ← 157	evard	EB	TR	123	0	1.00	123 0	45	1.00	45	168 45	
ey Boule	Bradley Boulevard 312 11 1 473 473 11 15 1156 1156	45	Bradley Boulevard	WB	T R	157 256	0	1.00 1.00	157 256	194	1.00	194	351 450	*
Bradle			Bradle	NB	TR	515	0	1.00	515 0	156	1.00	156	671 156	*
		1 (0 1		SB	TR	441	0	1.00	441 0	11	1.00	11	452 11	
	Seven Locks Road	d		Note:		•						CLV	1121	_
								Potomac Co				v/c	0.773	
								14	50					

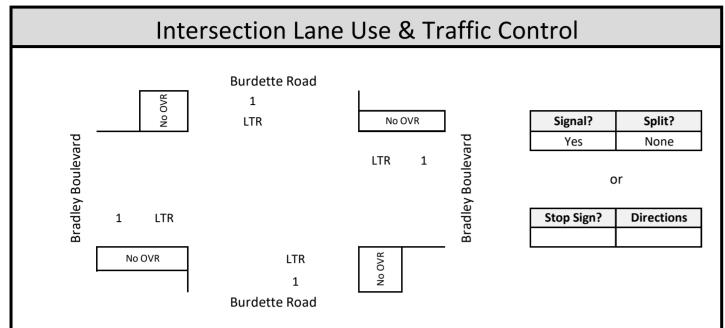
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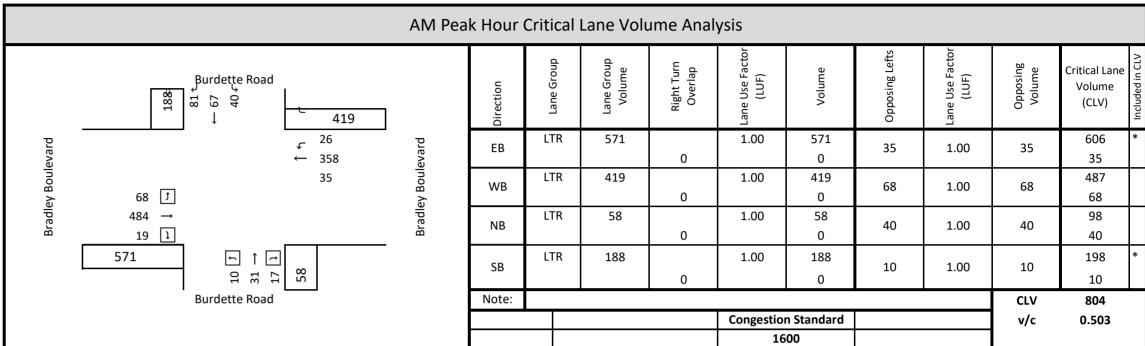
Critical Lane Volume and Level of Service Calculations Intersection: Bradley Boulevard / Burdette Road

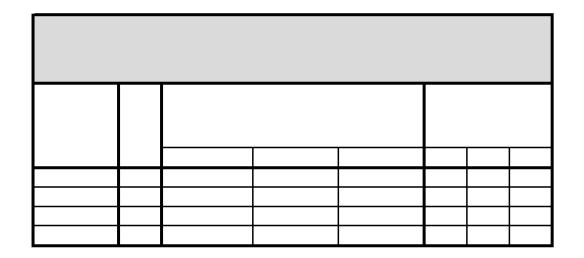
Jurisdiction: Montgomery County, MD

Scenario/Design Year: Background Future

Computed by: W+A







	Right Turn Overlap														
	:l. Right		Right Vol.				djacent O	verlap V	ol.		Overlap				
Approach	Excl.	AM					PM		LUF	AM	PM				
Eastbound	No	n/a	n/a		n/a	n/a	n/a		n/a	0	0				
Westbound	No	n/a	n/a		n/a	n/a	n/a		n/a	0	0				
Northbound	No	n/a	n/a		n/a	n/a	n/a		n/a	0	0				
Southbound	No	n/a	n/a		n/a	n/a	n/a		n/a	0	0				

Mo	ntgome	ry Count	ty LATR		
		Lane Use Factor	S		
	Number	Left Turn	Through		
	of Lanes	LUF	LUF		
	1	1	1.00		
	2	0.53	0.53		
	3	0.37	0.37		
	4		0.30		
	5		0.25		

			PM Pea	k Hour (Critical	Lane Volu	ıme Anal	ysis						
	104 104 48 ← 32 24 € Parter Board	409		Direction	Lane Group	Lane Group Volume	Right Turn Overlap	Lane Use Factor (LUF)	Volume	Opposing Lefts	Lane Use Factor (LUF)	Opposing Volume	Critical Lane Volume (CLV)	Included in CLV
evard		√ 36 ← 335	evard	EB	LTR	273	0	1.00	273 0	38	1.00	38	311 38	
Bradley Boulevard	25]	38	Bradley Boulevard	WB	LTR	409	0	1.00	409 0	25	1.00	25	434 25	*
Bradle	236 → 12 1		Bradle	NB	LTR	81	0	1.00	81 0	24	1.00	24	105 24	
		81		SB	LTR	104	0	1.00	104 0	15	1.00	15	119 15	*
	Burdette Road	1		Note:								CLV	553	
									n Standard			v/c	0.346	
								16	000					

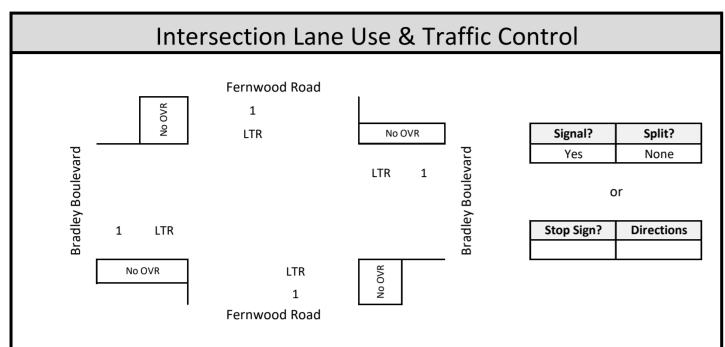
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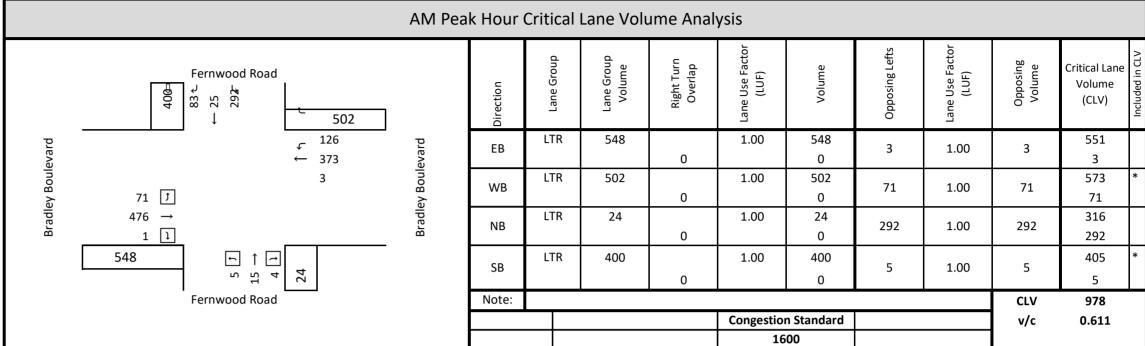
Critical Lane Volume and Level of Service Calculations Intersection: Bradley Boulevard / Fernwood Road

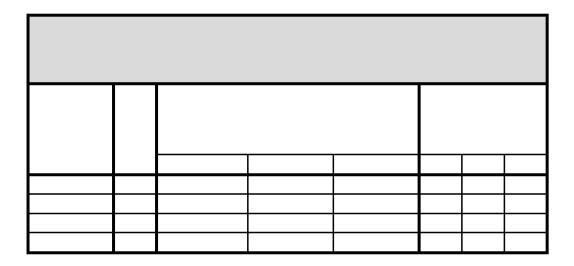
Jurisdiction: Montgomery County, MD

Scenario/Design Year: Background Future

Computed by: W+A







			Ri	ight	Turi	n Ov	erla	р					
	Right Vol. Adjacent Overlap Vol. Overlap												
Approach													
Eastbound	No	n/a	n/a		n/a	n/a	n/a		n/a	0	0		
Westbound	No	n/a	n/a		n/a	n/a	n/a		n/a	0	0		
Northbound	No	n/a	n/a		n/a	n/a	n/a		n/a	0	0		
Southbound	No	n/a	n/a		n/a	n/a	n/a		n/a	0	0		

Мо	ntgome	ry Count	ty LATR										
		Lane Use Factor	S										
	Number Left Turn Through of Lanes LUF LUF												
	of Lanes	LUF											
	1	1.00											
	2	0.53	0.53										
	3	0.37	0.37										
	4		0.30										
	5		0.25										

			PM Pea	k Hour (Critical	Lane Volu	ıme Anal	ysis						
	242 242 76 ← 146 Second proof	572		Direction	Lane Group	Lane Group Volume	Right Turn Overlap	Lane Use Factor (LUF)	Volume	Opposing Lefts	Lane Use Factor (LUF)	Opposing Volume	Critical Lane Volume (CLV)	Included in CLV
evard		<i>⊊</i> 234 ← 333	evard	EB	LTR	285	0	1.00	285 0	5	1.00	5	290 5	
Bradley Boulevard	51 J	5	Bradley Boulevard	WB	LTR	572	0	1.00	572 0	51	1.00	51	623 51	*
Bradle	228 → 6 1		Bradle	NB	LTR	32	0	1.00	32 0	146	1.00	146	178 146	
	285			SB	LTR	242	0	1.00	242 0	10	1.00	10	252 10	*
	Fernwood Road			Note:								CLV	875	
								Congestion				v/c	0.547	
								16	00					

· · · · · · · · · · · · · · · · · · ·						

	→	•	•	←	•	~
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<u></u>	Į,	1100	<u>₩Ы</u>	¥	HUIT
Traffic Volume (veh/h)	539	14	21	384	9	13
Future Volume (Veh/h)	539	14	21	384	9	13
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	599	16	23	427	10	14
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			615		1072	599
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			615		1072	599
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			98		96	97
cM capacity (veh/h)			965		238	502
Direction, Lane #	EB 1	EB 2	WB 1	NB 1		
Volume Total	599	16	450	24		
Volume Left	0	0	23	10		
Volume Right	0	16	0	14		
cSH	1700	1700	965	343		
Volume to Capacity	0.35	0.01	0.02	0.07		
Queue Length 95th (ft)	0	0	2	6		
Control Delay (s)	0.0	0.0	0.7	16.3		
Lane LOS			А	С		
Approach Delay (s)	0.0		0.7	16.3		
Approach LOS				С		
Intersection Summary						
Average Delay			0.7			
Intersection Capacity Utiliz	zation		47.4%	IC	U Level o	f Service
Analysis Period (min)			15			

3: Burdette Road & Bradley Boulevard #191

06/08/2021

	ၨ	→	•	•	←	•	4	†	1	\	ļ	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (vph)	68	484	19	35	358	26	10	31	17	40	67	81
Future Volume (vph)	68	484	19	35	358	26	10	31	17	40	67	81
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	12	12	12	12	12	12	12	13	12	12	14	12
Total Lost time (s)		5.0			5.0			5.0			5.0	
Lane Util. Factor		1.00			1.00			1.00			1.00	
Frt		1.00			0.99			0.96			0.94	
Flt Protected		0.99			1.00			0.99			0.99	
Satd. Flow (prot)		1844			1840			1833			1852	
Flt Permitted		0.90			0.92			0.93			0.91	
Satd. Flow (perm)		1669			1708			1715			1707	
Peak-hour factor, PHF	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89
Adj. Flow (vph)	76	544	21	39	402	29	11	35	19	45	75	91
RTOR Reduction (vph)	0	1	0	0	1	0	0	16	0	0	35	0
Lane Group Flow (vph)	0	640	0	0	469	0	0	49	0	0	176	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		6			2			8			4	
Permitted Phases	6			2			8			4		
Actuated Green, G (s)		55.1			55.1			13.1			13.1	
Effective Green, g (s)		55.1			55.1			13.1			13.1	
Actuated g/C Ratio		0.70			0.70			0.17			0.17	
Clearance Time (s)		5.0			5.0			5.0			5.0	
Vehicle Extension (s)		6.0			6.0			6.0			3.0	
Lane Grp Cap (vph)		1175			1203			287			285	
v/s Ratio Prot												
v/s Ratio Perm		c0.38			0.27			0.03			c0.10	
v/c Ratio		0.54			0.39			0.17			0.62	
Uniform Delay, d1		5.5			4.7			27.9			30.2	
Progression Factor		1.00			1.00			1.00			1.00	
Incremental Delay, d2		1.2			1.0			0.8			3.9	
Delay (s)		6.7			5.7			28.7			34.2	
Level of Service		Α			Α			С			С	
Approach Delay (s)		6.7			5.7			28.7			34.2	
Approach LOS		Α			Α			С			С	
Intersection Summary												
HCM 2000 Control Delay			11.6	H	CM 2000	Level of	Service		В			
HCM 2000 Volume to Capacity	ratio		0.56									
Actuated Cycle Length (s)			78.2		um of los				10.0			
Intersection Capacity Utilization	1		68.2%	IC	CU Level	of Service	:		С			
Analysis Period (min)			15									
c Critical Lane Group												

4: Fernwood Road & Bradley Boulevard #191

06/08/2021

	۶	→	•	•	+	•	1	†	~	/	+	✓
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (vph)	71	476	1	3	373	126	5	15	4	292	25	83
Future Volume (vph)	71	476	1	3	373	126	5	15	4	292	25	83
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	12	11	12	12	11	12	12	12	12	12	12	12
Total Lost time (s)		5.0			5.0			5.5			5.5	
Lane Util. Factor		1.00			1.00			1.00			1.00	
Frt		1.00			0.97			0.98			0.97	
Flt Protected		0.99			1.00			0.99			0.96	
Satd. Flow (prot)		1789			1739			1804			1747	
Flt Permitted		0.87			1.00			0.93			0.77	
Satd. Flow (perm)		1574			1736			1693			1391	
Peak-hour factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Adj. Flow (vph)	78	523	1	3	410	138	5	16	4	321	27	91
RTOR Reduction (vph)	0	0	0	0	9	0	0	3	0	0	11	0
Lane Group Flow (vph)	0	602	0	0	542	0	0	22	0	0	428	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		6			2			8			4	
Permitted Phases	6			2			8			4		
Actuated Green, G (s)		45.6			45.6			31.9			31.9	
Effective Green, g (s)		45.6			45.6			31.9			31.9	
Actuated g/C Ratio		0.52			0.52			0.36			0.36	
Clearance Time (s)		5.0			5.0			5.5			5.5	
Vehicle Extension (s)		3.0			3.0			3.0			3.0	
Lane Grp Cap (vph)		815			899			613			504	
v/s Ratio Prot												
v/s Ratio Perm		c0.38			0.31			0.01			c0.31	
v/c Ratio		0.74			0.60			0.04			0.85	
Uniform Delay, d1		16.5			14.9			18.1			25.8	
Progression Factor		1.00			1.00			1.00			1.00	
Incremental Delay, d2		3.5			3.0			0.0			12.6	
Delay (s)		20.1			17.8			18.1			38.5	
Level of Service		C			B			B			D	
Approach Delay (s)		20.1			17.8			18.1			38.5	
Approach LOS		С			В			В			D	
Intersection Summary												
HCM 2000 Control Delay			24.3	Н	CM 2000	Level of	Service		С			
HCM 2000 Volume to Capacity	/ ratio		0.78									
Actuated Cycle Length (s)			88.0		um of los				10.5			
Intersection Capacity Utilization	n		98.6%	IC	:U Level	of Service	!		F			
Analysis Period (min)			15									
c Critical Lane Group												

	-	•	•	←	•	<i>></i>
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<u> </u>	7		4	¥	
Traffic Volume (veh/h)	287	0	0	383	0	2
Future Volume (Veh/h)	287	0	0	383	0	2
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94
Hourly flow rate (vph)	305	0	0	407	0	2
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			305		712	305
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			305		712	305
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			100		100	100
cM capacity (veh/h)			1256		399	735
Direction, Lane #	EB 1	EB 2	WB 1	NB 1		
Volume Total	305	0	407	2		
Volume Left	0	0	0	0		
Volume Right	0	0	0	2		
cSH	1700	1700	1256	735		
Volume to Capacity	0.18	0.00	0.00	0.00		
Queue Length 95th (ft)	0.10	0.00	0.00	0.00		
Control Delay (s)	0.0	0.0	0.0	9.9		
Lane LOS	0.0	0.0	0.0	7.7 A		
Approach Delay (s)	0.0		0.0	9.9		
Approach LOS	0.0		0.0	7. 7 A		
••						
Intersection Summary						
Average Delay			0.0			
Intersection Capacity Utiliz	zation		30.2%	IC	U Level c	of Service
Analysis Period (min)			15			

3: Burdette Road & Bradley Boulevard #191

06/08/2021

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (vph)	25	236	12	38	335	36	15	29	37	24	32	48
Future Volume (vph)	25	236	12	38	335	36	15	29	37	24	32	48
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	12	12	12	12	12	12	12	13	12	12	14	12
Total Lost time (s)		5.0			5.0			5.0			5.0	
Lane Util. Factor		1.00			1.00			1.00			1.00	
Frt		0.99			0.99			0.94			0.94	
Flt Protected		1.00			1.00			0.99			0.99	
Satd. Flow (prot)		1843			1832			1790			1842	
Flt Permitted		0.95			0.95			0.93			0.92	
Satd. Flow (perm)		1756			1754			1678			1717	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	27	257	13	41	364	39	16	32	40	26	35	52
RTOR Reduction (vph)	0	1	0	0	2	0	0	35	0	0	42	0
Lane Group Flow (vph)	0	296	0	0	442	0	0	53	0	0	71	0
	Perm	NA	0	Perm	NA	U	Perm	NA	0	Perm	NA	0
Protected Phases	reiiii	6		reiiii	2		reiiii	8		reiiii	4	
Permitted Phases	4	U		2	Z		8	0		4	4	
Actuated Green, G (s)	6	55.5		Z	55.5		0	8.6		4	8.6	
Effective Green, g (s)		55.5			55.5			8.6			8.6	
		0.75			0.75			0.12			0.12	
Actuated g/C Ratio Clearance Time (s)		5.0			5.0			5.0			5.0	
		6.0			6.0			6.0			3.0	
Vehicle Extension (s)												
Lane Grp Cap (vph)		1315			1313			194			199	
v/s Ratio Prot		0.47			0.05			0.00			0.04	
v/s Ratio Perm		0.17			c0.25			0.03			c0.04	
v/c Ratio		0.23			0.34			0.27			0.36	
Uniform Delay, d1		2.8			3.1			29.9			30.2	
Progression Factor		1.00			1.00			1.00			1.00	
Incremental Delay, d2		0.2			0.7			2.1			1.1	
Delay (s)		3.1			3.8			32.0			31.3	
Level of Service		Α			Α			С			С	
Approach Delay (s)		3.1			3.8			32.0			31.3	
Approach LOS		А			Α			С			С	
Intersection Summary												
HCM 2000 Control Delay			9.5	Н	CM 2000	Level of	Service		Α			
HCM 2000 Volume to Capacity I	ratio		0.34									
Actuated Cycle Length (s)			74.1		um of lost				10.0			
Intersection Capacity Utilization			45.9%	IC	CU Level of	of Service	;		Α			
Analysis Period (min)			15									
c Critical Lane Group												

4: Fernwood Road & Bradley Boulevard #191

06/08/2021

	۶	→	•	•	—	•	•	†	~	\	↓	√
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (vph)	51	228	6	5	333	234	10	19	3	146	20	76
Future Volume (vph)	51	228	6	5	333	234	10	19	3	146	20	76
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	12	11	12	12	11	12	12	12	12	12	12	12
Total Lost time (s)		5.0			5.0			5.5			5.5	
Lane Util. Factor		1.00			1.00			1.00			1.00	
Frt		1.00			0.94			0.99			0.96	
Flt Protected		0.99			1.00			0.99			0.97	
Satd. Flow (prot)		1780			1700			1812			1732	
Flt Permitted		0.85			1.00			0.90			0.80	
Satd. Flow (perm)		1526			1698			1658			1420	
Peak-hour factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Adj. Flow (vph)	53	235	6	5	343	241	10	20	3	151	21	78
RTOR Reduction (vph)	0	0	0	0	14	0	0	2	0	0	22	0
Lane Group Flow (vph)	0	294	0	0	575	0	0	31	0	0	228	0
	Perm	NA	0	Perm	NA	0	Perm	NA		Perm	NA	
Protected Phases	r Cilli	6		r Cilli	2		r Cilli	8		r Cilli	4	
Permitted Phases	6	Ü		2	2		8	0		4	4	
Actuated Green, G (s)	0	45.2		Z	45.2		0	16.5		4	16.5	
Effective Green, g (s)		45.2			45.2			16.5			16.5	
Actuated g/C Ratio		0.63			0.63			0.23			0.23	
Clearance Time (s)		5.0			5.0			5.5			5.5	
Vehicle Extension (s)		3.0			3.0			3.0			3.0	
` '												
Lane Grp Cap (vph)		955			1063			378			324	
v/s Ratio Prot		0.10			-0.24			0.00			-0.17	
v/s Ratio Perm		0.19			c0.34			0.02			c0.16	
v/c Ratio		0.31			0.54			0.08			0.70	
Uniform Delay, d1		6.3			7.6			21.9			25.6	
Progression Factor		1.00			1.00			1.00			1.00	
Incremental Delay, d2		0.2			2.0			0.1			6.7	
Delay (s)		6.4			9.6			22.0			32.3	
Level of Service		Α			A			С			С	
Approach Delay (s)		6.4			9.6			22.0			32.3	
Approach LOS		Α			Α			С			С	
Intersection Summary												
HCM 2000 Control Delay			14.0	Н	CM 2000	Level of	Service		В			
HCM 2000 Volume to Capacity	ratio		0.58									
Actuated Cycle Length (s)			72.2		um of lost				10.5			
Intersection Capacity Utilization			77.5%	IC	U Level	of Service	!		D			
Analysis Period (min)			15									
c Critical Lane Group												

Higher Ground Education, Inc July 2021

APPENDIX G TOTAL FUTURE INTERSECTION DELAYS, CLVs, and HCM WORKSHEETS

G-1

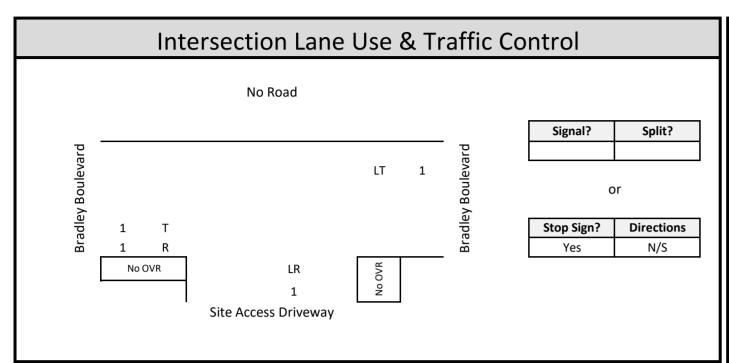
Critical Lane Volume and Level of Service Calculations

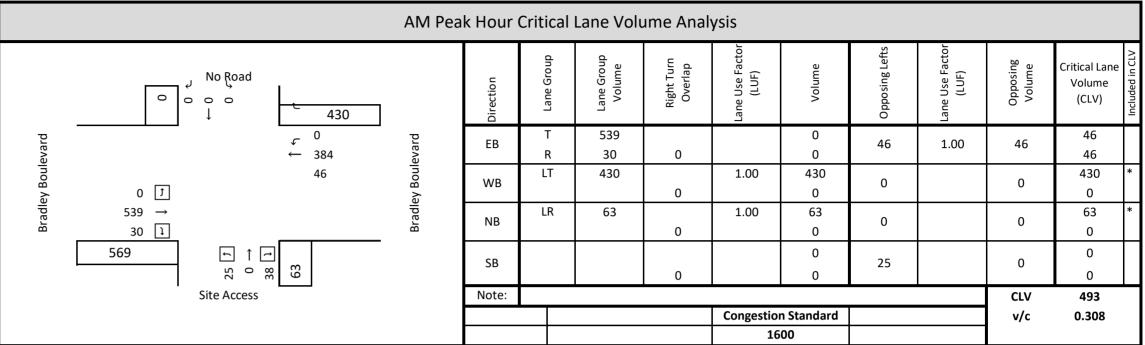
Intersection: Bradley Boulevard / Site Access Driveway

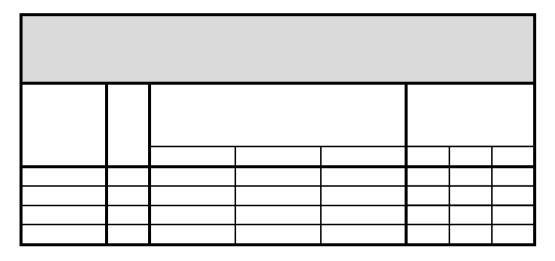
Jurisdiction: Montgomery County, MD

Scenario/Design Year: Background Future

Computed by: W+A

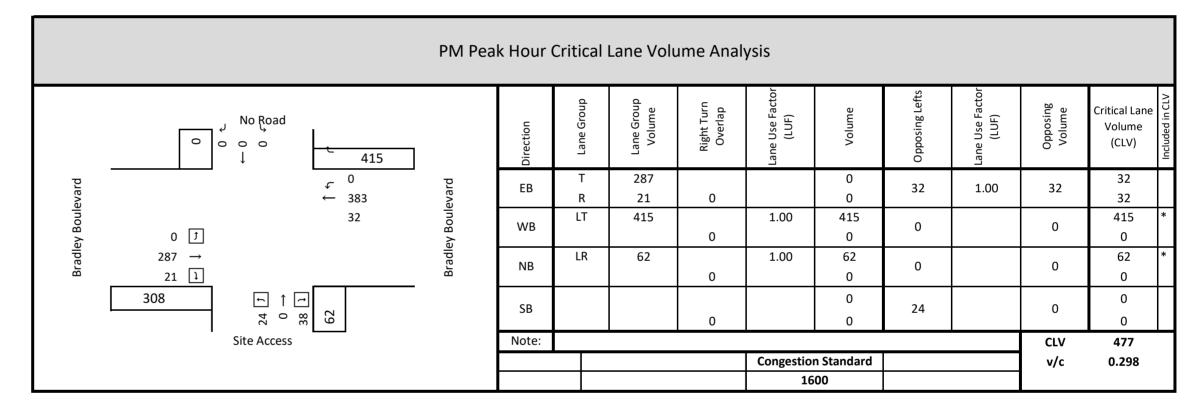






			R	ight	Tur	n Ov	/erla	р						
	Right Vol. Adjacent Overlap Vol. Overlap													
Approach	Excl.	AM	AM PM LUF AM PM LUF AM PM											
Eastbound	No	n/a	n/a		n/a	n/a	n/a		n/a	0	0			
Westbound	No	n/a	n/a		n/a	n/a	n/a		n/a	0	0			
Northbound	No	n/a	n/a		n/a	n/a	n/a	·	n/a	0	0			
Southbound														

Montgomery County LATR											
		Lane Use Factors									
	Number	Left Turn	Through	1							
	of Lanes	LUF	LUF								
	1	1	1.00								
	2	0.53	0.53								
	3	0.37	0.37								
	4		0.30								
	5		0.25								



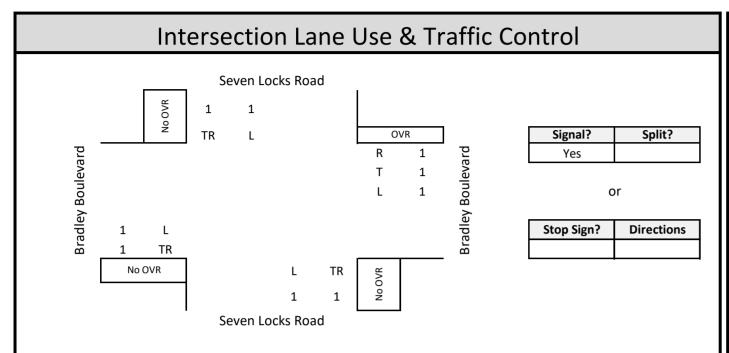
2

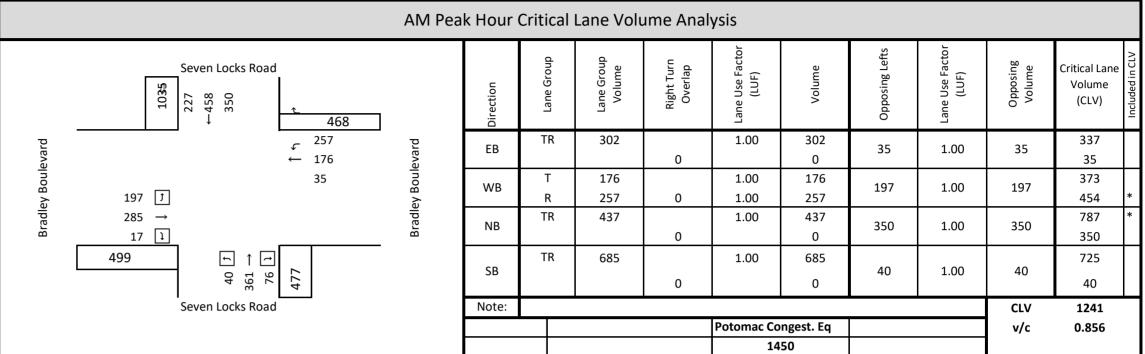
Critical Lane Volume and Level of Service Calculations Intersection: Bradley Boulevard / Seven Locks Road

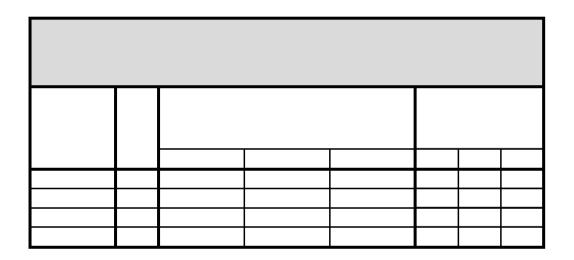
Jurisdiction: Montgomery County, MD

Scenario/Design Year: Background Future

Computed by: W+A







Right Turn Overlap												
	il. Right	Right Vol.				Adjacent Overlap Vol.				Overlap		
Approach	Excl.	AM	PM		LUF	AM	PM		LUF	AM	PM	
Eastbound	No	n/a	n/a		n/a	n/a	n/a		n/a	0	0	
Westbound	Yes	257	265		1.00	350	164		0.00	0	0	
Northbound	No	n/a	n/a		n/a	n/a	n/a		n/a	0	0	
Southbound	No	n/a	n/a		n/a	n/a	n/a		n/a	0	0	

Montgomery County LATR											
		Lane Use Factors									
	Number	Left Turn	Through								
	of Lanes	LUF	LUF								
	1	1	1.00								
	2	0.53	0.53								
	3	0.37	0.37								
	4		0.30								
	5		0.25								

	PM Peak Hour Critical Lane Volume Analysis												
	608 1592 286 Seven Tocks Boa	482		Direction	Lane Group	Lane Group Volume	Right Turn Overlap	Lane Use Factor (LUF)	Volume	Opposing Lefts	Lane Use Factor (LUF)	Opposing Volume	Critical Lane Volume (CLV)
evard	194 🚺	£ 265 ← 166	evard	EB	TR	131	0	1.00	131 0	51	1.00	51	182 51
Bradley Boulevard		Pradley Boulevard	ey Boule	WB	T R	166 265	0	1.00 1.00	166 265	194	1.00	194	360 459 *
Bradle	120 → 11 1		Bradle	NB	TR	520	0	1.00	520 0	164	1.00	164	684 * 164
	352	· · ·		SB	TR	441	0	1.00	441 0	11	1.00	11	452 11
Seven Locks Road				Note:				•			,	CLV	1143
								Potomac Co				v/c	0.788
								14	50			1	

· · · · · · · · · · · · · · · · · · ·												
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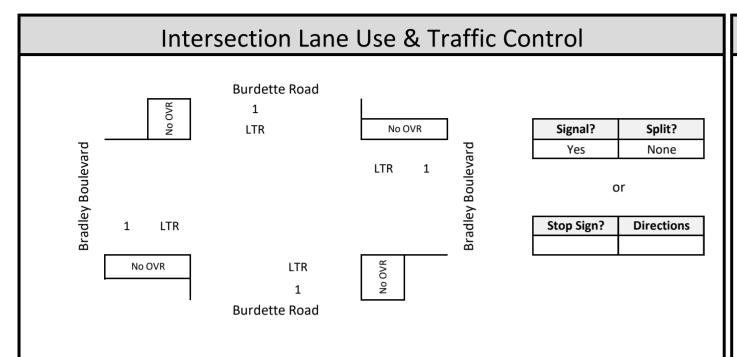
G-3

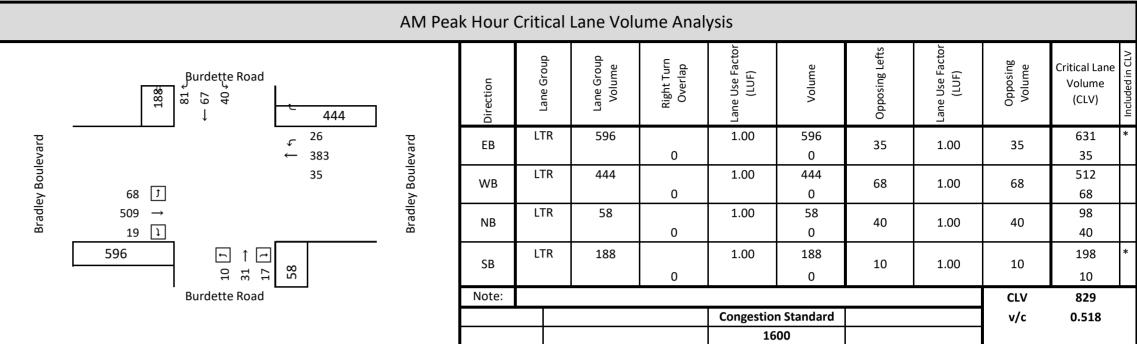
Critical Lane Volume
and
Level of Service Calculations

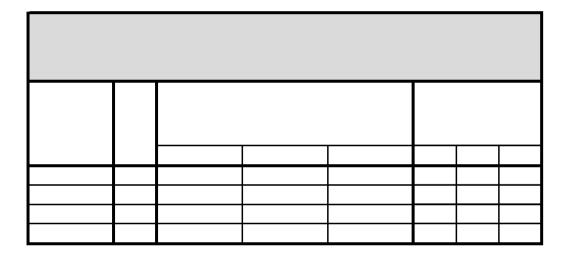
Intersection: Bradley Boulevard / Burdette Road

Jurisdiction: Montgomery County, MD

Scenario/Design Year: Background Future
Computed by: W+A







			R	ight	Tur	n Ov	verla	ар				
	il. Right		Righ	t Vol.		A	djacent C	verlap V	ol.		Overlap	
Approach	Excl.	AM	PM		LUF	AM	PM		LUF	AM	PM	
Eastbound	No	n/a	n/a		n/a	n/a	n/a		n/a	0	0	
Westbound	No	n/a	n/a		n/a	n/a	n/a		n/a	0	0	
Northbound	No	n/a	n/a		n/a	n/a	n/a		n/a	0	0	
Southbound	No	n/a	n/a		n/a	n/a	n/a		n/a	0	0	

Мс	ntgome	ry Coun	ty LATR		
		Lane Use Factors	S		
	Number	Left Turn	Through		
	of Lanes	LUF	LUF		
	1	1	1.00		
	2	0.53	0.53		
	3	0.37	0.37		
	4		0.30		
	5		0.25		

			PM Pea	k Hour (Critical	Lane Volu	ıme Anal	ysis						
	104 104 32 defrate Road 24 € Table 104	441		Direction	Lane Group	Lane Group Volume	Right Turn Overlap	Lane Use Factor (LUF)	Volume	Opposing Lefts	Lane Use Factor (LUF)	Opposing Volume	Critical Lane Volume (CLV)	Included in CLV
evard		<i>⊊</i> 36 ← 367	evard	EB	LTR	309	0	1.00	309 0	38	1.00	38	347 38	
Bradley Boulevard	25 🗇	38	Bradley Boulevard	WB	LTR	441	0	1.00	441 0	25	1.00	25	466 25	*
Bradle	272 → 12 1		Bradle	NB	LTR	81	0	1.00	81 0	24	1.00	24	105 24	
	309 15 1 15 1 1 15 1 1 1 1 1 1 1 1 1 1 1 1			SB	LTR	104	0	1.00	104 0	15	1.00	15	119 15	*
	Burdette Road			Note:								CLV	585	
								Congestion				v/c	0.366	
								16	00					

						ı

G-4

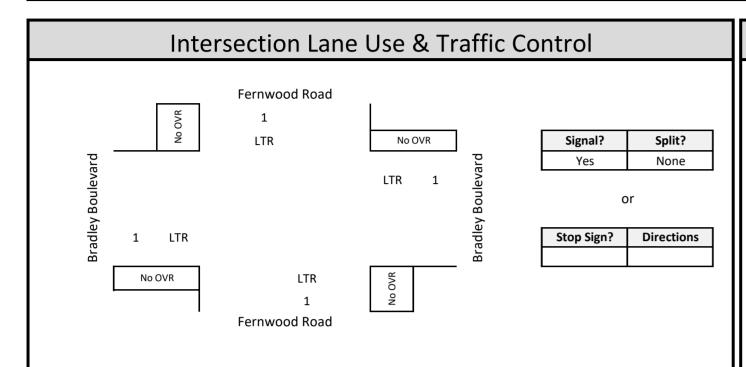
4

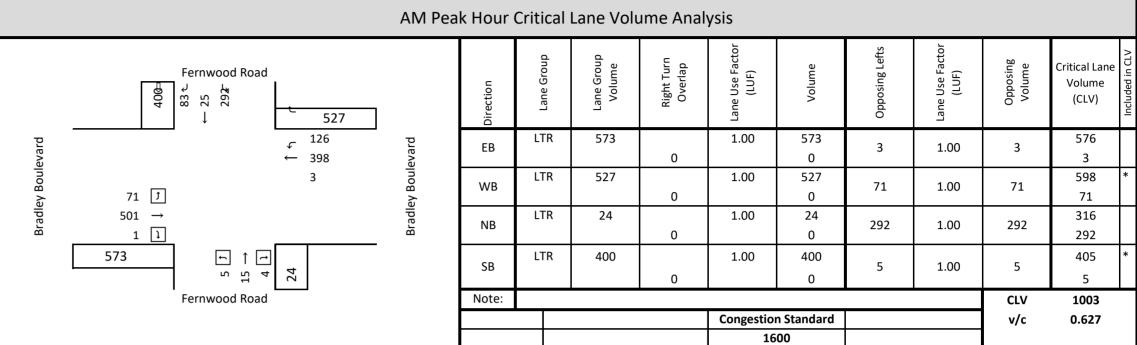
Critical Lane Volume and Level of Service Calculations Intersection: Bradley Boulevard / Fernwood Road

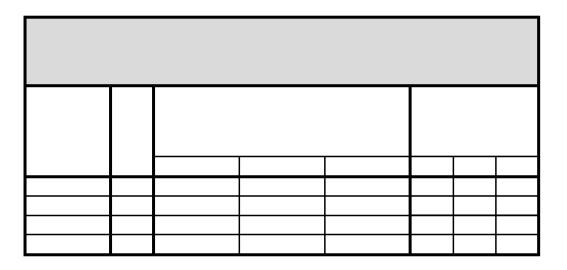
Jurisdiction: Montgomery County, MD

Scenario/Design Year: Background Future

Computed by: W+A







			R	ight	Tur	n O\	/erla	р				
	il. Right		Right	t Vol.		Ad	djacent C	verlap V	ol.		Overlap	
Approach	Excl.	AM	PM		LUF	AM	PM		LUF	AM	PM	
Eastbound	No	n/a	n/a		n/a	n/a	n/a		n/a	0	0	
Westbound	No	n/a	n/a		n/a	n/a	n/a		n/a	0	0	
Northbound	No	n/a	n/a		n/a	n/a	n/a		n/a	0	0	
Southbound	No	n/a	n/a		n/a	n/a	n/a		n/a	0	0	

Мо	ntgome	ry Coun	ty LATR		
		Lane Use Factors	S		
	Number	Left Turn	Through		
	of Lanes	LUF	LUF		
	1	1	1.00		
	2	0.53	0.53		
	3	0.37	0.37		
	4		0.30		
	5		0.25		

		PM Pea	k Hour	Critical	Lane Volu	ıme Anal	ysis					
	243 76 € 20 146 Fernwood Booom		Direction	Lane Group	Lane Group Volume	Right Turn Overlap	Lane Use Factor (LUF)	Volume	Opposing Lefts	Lane Use Factor (LUF)	Opposing Volume	Critical Lane Volume (CLV)
evard	_√ 234 ← 365	evard	EB	LTR	321	0	1.00	321 0	5	1.00	5	326 5
Bradley Boulevard	5 51 J	Bradley Boulevard	WB	LTR	604	0	1.00	604 0	51	1.00	51	655 * 51
Bradle	264 → 6 1	Bradle	NB	LTR	32	0	1.00	32 0	146	1.00	146	178 146
	321		SB	LTR	242	0	1.00	242 0	10	1.00	10	252 * 10
	Fernwood Road		Note:								CLV	907
						•	Congestion	n Standard		·	v/c	0.567
							16	00				

· 						

	→	•	•	←	4	<i>></i>
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations		7		4	W	
Traffic Volume (veh/h)	539	34	50	384	27	41
Future Volume (Veh/h)	539	34	50	384	27	41
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	599	38	56	427	30	46
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			637		1138	599
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			637		1138	599
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			94		86	91
cM capacity (veh/h)			947		210	502
Direction, Lane #	EB 1	EB 2	WB 1	NB 1		
Volume Total	599	38	483	76		
Volume Left	0	0	56	30		
Volume Right	0	38	0	46		
cSH	1700	1700	947	324		
Volume to Capacity	0.35	0.02	0.06	0.23		
Queue Length 95th (ft)	0.00	0.02	5	22		
Control Delay (s)	0.0	0.0	1.7	19.5		
Lane LOS	0.0	0.0	A	C		
Approach Delay (s)	0.0		1.7	19.5		
Approach LOS	0.0			С		
Intersection Summary						
Average Delay			1.9			
Intersection Capacity Utiliz	zation		65.4%	IC	Hlavala	of Service
	LaliUII			IC	U Level (ii Seivice
Analysis Period (min)			15			

3: Burdette Road & Bradley Boulevard #191

06/08/2021

Lane Configurations A		ၨ	→	•	•	←	•	4	†	-	\	ļ	1
Traffic Volume (yph) 68 512 19 35 387 26 10 31 17 40 67 81 loted Flow (yphp) 68 512 19 35 387 26 10 31 17 40 67 81 loted Flow (yphp) 1900 1900 1900 1900 1900 1900 1900 190	Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Traffic Volume (vph) 68 512 19 35 387 26 10 31 17 40 67 81 totare Volume (vph) 68 512 19 35 387 26 10 31 17 40 67 81 total Column (vph) 68 512 19 35 387 26 10 31 17 40 67 81 total Exercise (vph) 1900 1900 1900 1900 1900 1900 1900 190	Lane Configurations		4			4			4			4	
Ideal Flow (yphpl)	Traffic Volume (vph)	68		19	35		26	10		17	40		81
Lane Width 12 12 12 12 12 12 12 12 13 12 12 14 12 13 16 17 10 10 10 10 10 10 10 10 10 10 10 10 10	Future Volume (vph)	68	512	19	35	387	26	10	31	17	40	67	81
Total Lost time (s)	Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.0	Lane Width	12	12	12	12	12	12	12	13	12	12	14	12
Fit Protected	Total Lost time (s)		5.0			5.0			5.0			5.0	
Fil Protected 0.99 1.00 0.99 0.99 Satid. Flow (pror) 1844 1841 1833 1852 Fil Permitted 0.90 0.93 0.93 0.91 Satid. Flow (perm) 1668 1711 1715 1707 Peak-hour factor, PHF 0.89 0.89 0.89 0.89 0.89 0.89 0.89 0.89	Lane Util. Factor		1.00			1.00			1.00			1.00	
Satd. Flow (prot)	Frt		1.00			0.99			0.96			0.94	
Fit Permitted 0.90 0.93 0.93 0.91 Saldt, Flow (perm) 1668 1711 1715 1707 Peak-hour factor, PHF 0.89 0.89 0.89 0.89 0.89 0.89 0.89 0.89	Flt Protected		0.99			1.00			0.99			0.99	
Satu Flow (perm) 1668 1711 1715 1707	Satd. Flow (prot)		1844			1841			1833			1852	
Peak-hour factor, PHF 0.89	Flt Permitted		0.90			0.93			0.93			0.91	
Adj. Flow (vph) 76 575 21 39 435 29 11 35 19 45 75 91 RTOR Reduction (vph) 0 1 0 0 1 0 0 16 0 0 35 0 Lane Group Flow (vph) 0 671 0 0 502 0 0 49 0 0 176 0 176 0 Lane Group Flow (vph) 0 671 0 0 502 0 0 49 0 0 176 0 176 0 Lane Group Flow (vph) 0 671 0 0 502 0 0 49 0 0 176 0 176 0 Lane Group Flow (vph) 0 671 0 0 502 0 0 0 49 0 0 176	Satd. Flow (perm)		1668			1711			1715			1707	
Adj. Flow (vph) 76 575 21 39 435 29 11 35 19 45 75 91 RTOR Reduction (vph) 0 1 0 0 1 0 0 16 0 0 35 0 Lane Group Flow (vph) 0 671 0 0 502 0 0 49 0 0 176 0 176 0 Lane Group Flow (vph) 0 671 0 0 502 0 0 49 0 0 176 0 176 0 Lane Group Flow (vph) 0 671 0 0 502 0 0 49 0 0 176 0 176 0 Lane Group Flow (vph) 0 671 0 0 502 0 0 0 49 0 0 176	Peak-hour factor, PHF	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89
RTOR Reduction (vph) 0 1 0 0 1 0 0 16 0 0 35 0 Lane Group Flow (vph) 0 671 0 0 502 0 0 49 0 0 176 0 Turn Type Perm NA		76	575	21	39	435	29	11	35	19	45	75	91
Lane Group Flow (vph) 0 671 0 0 502 0 49 0 0 176 0 Turn Type Perm NA Perm NA Perm NA Perm NA Protected Phases 6 2 8 4 4 Permitted Phases 6 2 8 4 4 Actuated Green, G (s) 55.1 55.1 13.1 13.1 13.1 Effective Green, g (s) 55.1 55.1 13.1 13.1 13.1 Actuated g/C Ratio 0.70 0.70 0.17 0.17 0.17 Clearance Time (s) 5.0 5.0 5.0 5.0 5.0 Vehicle Extension (s) 6.0 6.0 6.0 3.0 1.0 Lane Grap Cap (vph) 1175 1205 287 285 v/s Ratio Perm c0.40 0.29 0.03 c0.10 v/s Ratio Perm c0.44 0.57 0.48 27.9 30.2	, , ,	0	1	0	0	1	0	0	16	0	0	35	0
Turn Type Perm NA Perm NA Perm NA Perm NA Perm NA Perm NA Perm NA Protected Phases 6 2 8 4 Actuated Green, G (s) 55.1 55.1 13.1 13.1 13.1 13.1 Actuated green, g (s) 55.1 55.1 13.1 13.1 13.1 13.1 Actuated green, g (s) 55.1 55.1 55.1 13.1 13.1 13.1 Actuated green, g (s) 55.0 55.0 5.0 5.0 5.0 5.0 5.0 5.0 5.0 5		0	671	0	0	502	0	0	49	0	0	176	0
Protected Phases 6 2 8 4 Permitted Phases 6 2 8 4 Actuated Green, G (s) 55.1 55.1 13.1 13.1 Effective Green, g (s) 55.1 55.1 13.1 13.1 Actuated g/C Ratio 0.70 0.70 0.17 0.17 Clearance Time (s) 5.0 5.0 5.0 5.0 Vehicle Extension (s) 6.0 6.0 6.0 3.0 Lane Grp Cap (vph) 1175 1205 287 285 v/s Ratio Prot v/s Ratio Prot v/s Ratio Prot v/s Ratio Perm 0.42 0.17 0.62 Uniform Delay, d1 5.7 4.8 27.9 30.2 Progression Factor 1.00 1.00 1.00 1.00 Incremental Delay, d2 1.3 1.1 0.8 3.9 Delay (s) 7.1 5.9 28.7 34.2 Level of Service A A C C Approach		Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Permitted Phases 6													
Actuated Green, G (s) 55.1 55.1 13.1 13.1 13.1 Effective Green, g (s) 55.1 55.1 13.1 13.1 13.1 Actuated g/C Ratio 0.70 0.70 0.70 0.17 0.17 0.17 Clearance Time (s) 5.0 5.0 5.0 5.0 5.0 Vehicle Extension (s) 6.0 6.0 6.0 6.0 3.0 Lane Grp Cap (vph) 1175 1205 287 285 Vehicle Extension Control Delay 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.0		6			2			8			4		
Effective Green, g (s) 55.1 55.1 13.1 13.1 Actuated g/C Ratio 0.70 0.70 0.17 0.17 Clearance Time (s) 5.0 5.0 5.0 5.0 Vehicle Extension (s) 6.0 6.0 6.0 3.0 Lane Grp Cap (vph) 1175 1205 287 285 V/s Ratio Prot V/s Ratio Perm c0.40 0.29 0.03 c0.10 V/c Ratio 0.57 0.42 0.17 0.62 Uniform Delay, d1 5.7 4.8 27.9 30.2 Progression Factor 1.00 1.00 1.00 1.00 Incremental Delay, d2 1.3 1.1 0.8 3.9 Delay (s) 7.1 5.9 28.7 34.2 Level of Service A A C C Approach LOS A A C C Intersection Summary HCM 2000 Level of Service B HCM 2000 Volume to Capacity ratio 0.58 Actuated Cycle Length (s) 78.2 Sum of lost time (s) 10.0			55.1			55.1			13.1			13.1	
Actuated g/C Ratio 0.70 0.70 0.17 0.17 Clearance Time (s) 5.0 5.0 5.0 5.0 Vehicle Extension (s) 6.0 6.0 6.0 3.0 Lane Grp Cap (vph) 1175 1205 287 285 v/s Ratio Prot v/s Ratio Perm c0.40 0.29 0.03 c0.10 v/c Ratio 0.57 0.42 0.17 0.62 Uniform Delay, d1 5.7 4.8 27.9 30.2 Progression Factor 1.00 1.00 1.00 1.00 Incremental Delay, d2 1.3 1.1 0.8 3.9 Delay (s) 7.1 5.9 28.7 34.2 Level of Service A A C C Approach Delay (s) 7.1 5.9 28.7 34.2 Approach LOS A A A C C Intersection Summary HCM 2000 Control Delay 11.6 HCM 2000 Level of Service B Actuated Cycle Length (s) 78.2 Sum of lost time (s) 10.0													
Clearance Time (s) 5.0 5.0 5.0 5.0 Vehicle Extension (s) 6.0 6.0 3.0 Lane Grp Cap (vph) 1175 1205 287 285 v/s Ratio Prot v/s Ratio Perm c0.40 0.29 0.03 c0.10 v/c Ratio 0.57 0.42 0.17 0.62 Uniform Delay, d1 5.7 4.8 27.9 30.2 Progression Factor 1.00 1.00 1.00 1.00 Incremental Delay, d2 1.3 1.1 0.8 3.9 Delay (s) 7.1 5.9 28.7 34.2 Level of Service A A C C Approach Delay (s) 7.1 5.9 28.7 34.2 Approach LOS A A C C Intersection Summary HCM 2000 Control Delay 11.6 HCM 2000 Level of Service B HCM 2000 Volume to Capacity ratio 0.58 Actuated Cycle Length (s) 78.2 Sum of lost time (s) 10.0 Intersection Capac													
Vehicle Extension (s) 6.0 6.0 6.0 3.0 Lane Grp Cap (vph) 1175 1205 287 285 v/s Ratio Prot v/s Ratio Perm c0.40 0.29 0.03 c0.10 v/c Ratio 0.57 0.42 0.17 0.62 Uniform Delay, d1 5.7 4.8 27.9 30.2 Progression Factor 1.00 1.00 1.00 1.00 Incremental Delay, d2 1.3 1.1 0.8 3.9 Delay (s) 7.1 5.9 28.7 34.2 Level of Service A A C C Approach Delay (s) 7.1 5.9 28.7 34.2 Approach LOS A A C C Intersection Summary B HCM 2000 Control Delay 11.6 HCM 2000 Level of Service B HCM 2000 Volume to Capacity ratio 0.58 A Sum of lost time (s) 10.0 Intersection Capacity Utilization 70.4% ICU Level of Service <t< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></t<>													
Lane Grp Cap (vph) 1175 1205 287 285 v/s Ratio Prot v/s Ratio Perm c0.40 0.29 0.03 c0.10 v/c Ratio 0.57 0.42 0.17 0.62 Uniform Delay, d1 5.7 4.8 27.9 30.2 Progression Factor 1.00 1.00 1.00 1.00 Incremental Delay, d2 1.3 1.1 0.8 3.9 Delay (s) 7.1 5.9 28.7 34.2 Level of Service A A C C Approach Delay (s) 7.1 5.9 28.7 34.2 Approach LOS A A C C Intersection Summary HCM 2000 Control Delay 11.6 HCM 2000 Level of Service B HCM 2000 Volume to Capacity ratio 0.58 Actuated Cycle Length (s) 78.2 Sum of lost time (s) 10.0 Intersection Capacity Utilization 70.4% ICU Level of Service C Analysis Period (min) 15													
v/s Ratio Prot v/s Ratio Perm c0.40 0.29 0.03 c0.10 v/c Ratio 0.57 0.42 0.17 0.62 Uniform Delay, d1 5.7 4.8 27.9 30.2 Progression Factor 1.00 1.00 1.00 1.00 Incremental Delay, d2 1.3 1.1 0.8 3.9 Delay (s) 7.1 5.9 28.7 34.2 Level of Service A A C C C Approach Delay (s) 7.1 5.9 28.7 34.2 A Approach LOS A A C C C Intersection Summary 11.6 HCM 2000 Level of Service B HCM 2000 Volume to Capacity ratio 0.58 A C C Actuated Cycle Length (s) 78.2 Sum of lost time (s) 10.0 Intersection Capacity Utilization 70.4% ICU Level of Service C Analysis Period (min) 15													
v/s Ratio Perm c0.40 0.29 0.03 c0.10 v/c Ratio 0.57 0.42 0.17 0.62 Uniform Delay, d1 5.7 4.8 27.9 30.2 Progression Factor 1.00 1.00 1.00 1.00 Incremental Delay, d2 1.3 1.1 0.8 3.9 Delay (s) 7.1 5.9 28.7 34.2 Level of Service A A C C C Approach Delay (s) 7.1 5.9 28.7 34.2 Approach LOS A A C C C Intersection Summary Intersection Summary Intersection Capacity ratio Intersection Capacity (s) Intersection Capacity (s) Intersection Capacity (s) Intersection Capacity Utilization Intersection Service C C Analysis Period (min) 15 ICU Level of Service C C						.200							
V/c Ratio 0.57 0.42 0.17 0.62 Uniform Delay, d1 5.7 4.8 27.9 30.2 Progression Factor 1.00 1.00 1.00 1.00 Incremental Delay, d2 1.3 1.1 0.8 3.9 Delay (s) 7.1 5.9 28.7 34.2 Level of Service A A C C Approach Delay (s) 7.1 5.9 28.7 34.2 Approach LOS A A C C Intersection Summary HCM 2000 Control Delay 11.6 HCM 2000 Level of Service B HCM 2000 Volume to Capacity ratio 0.58 Actuated Cycle Length (s) 78.2 Sum of lost time (s) 10.0 Intersection Capacity Utilization 70.4% ICU Level of Service C Analysis Period (min) 15			c0.40			0.29			0.03			c0.10	
Uniform Delay, d1 5.7 4.8 27.9 30.2 Progression Factor 1.00 1.00 1.00 1.00 Incremental Delay, d2 1.3 1.1 0.8 3.9 Delay (s) 7.1 5.9 28.7 34.2 Level of Service A A C C Approach Delay (s) 7.1 5.9 28.7 34.2 Approach LOS A A C C Intersection Summary HCM 2000 Control Delay 11.6 HCM 2000 Level of Service B HCM 2000 Volume to Capacity ratio 0.58 Actuated Cycle Length (s) 78.2 Sum of lost time (s) 10.0 Intersection Capacity Utilization 70.4% ICU Level of Service C Analysis Period (min) 15													
Progression Factor 1.00 1.00 1.00 1.00 Incremental Delay, d2 1.3 1.1 0.8 3.9 Delay (s) 7.1 5.9 28.7 34.2 Level of Service A A C C Approach Delay (s) 7.1 5.9 28.7 34.2 Approach LOS A A C C Intersection Summary B HCM 2000 Level of Service B HCM 2000 Volume to Capacity ratio 0.58 Actuated Cycle Length (s) 78.2 Sum of lost time (s) 10.0 Intersection Capacity Utilization 70.4% ICU Level of Service C Analysis Period (min) 15													
Incremental Delay, d2	,												
Delay (s) 7.1 5.9 28.7 34.2 Level of Service A A C C Approach Delay (s) 7.1 5.9 28.7 34.2 Approach LOS A A C C Intersection Summary HCM 2000 Control Delay 11.6 HCM 2000 Level of Service B HCM 2000 Volume to Capacity ratio 0.58 Actuated Cycle Length (s) 78.2 Sum of lost time (s) 10.0 Intersection Capacity Utilization 70.4% ICU Level of Service C Analysis Period (min) 15													
Level of Service A A A C C C Approach Delay (s) 7.1 5.9 28.7 34.2 Approach LOS A A C C C Intersection Summary HCM 2000 Control Delay 11.6 HCM 2000 Level of Service B HCM 2000 Volume to Capacity ratio 0.58 Actuated Cycle Length (s) 78.2 Sum of lost time (s) 10.0 Intersection Capacity Utilization 70.4% ICU Level of Service C Analysis Period (min) 15													
Approach Delay (s) 7.1 5.9 28.7 34.2 Approach LOS A A C C C Intersection Summary HCM 2000 Control Delay 11.6 HCM 2000 Level of Service B HCM 2000 Volume to Capacity ratio 0.58 Actuated Cycle Length (s) 78.2 Sum of lost time (s) 10.0 Intersection Capacity Utilization 70.4% ICU Level of Service C Analysis Period (min) 15													
Approach LOS A A C C Intersection Summary HCM 2000 Control Delay 11.6 HCM 2000 Level of Service B HCM 2000 Volume to Capacity ratio 0.58 Actuated Cycle Length (s) 78.2 Sum of lost time (s) 10.0 Intersection Capacity Utilization 70.4% ICU Level of Service C Analysis Period (min) 15													
HCM 2000 Control Delay 11.6 HCM 2000 Level of Service B HCM 2000 Volume to Capacity ratio 0.58 Actuated Cycle Length (s) 78.2 Sum of lost time (s) 10.0 Intersection Capacity Utilization 70.4% ICU Level of Service C Analysis Period (min) 15	Approach LOS					_							
HCM 2000 Volume to Capacity ratio Actuated Cycle Length (s) 78.2 Sum of lost time (s) 10.0 Intersection Capacity Utilization 70.4% ICU Level of Service C Analysis Period (min) 15	Intersection Summary												
Actuated Cycle Length (s) 78.2 Sum of lost time (s) 10.0 Intersection Capacity Utilization 70.4% ICU Level of Service C Analysis Period (min) 15	HCM 2000 Control Delay			11.6	Н	CM 2000	Level of	Service		В			
Actuated Cycle Length (s) 78.2 Sum of lost time (s) 10.0 Intersection Capacity Utilization 70.4% ICU Level of Service C Analysis Period (min) 15	HCM 2000 Volume to Capacity	ratio		0.58									
Analysis Period (min) 15	Actuated Cycle Length (s)			78.2	Sı	um of lost	time (s)			10.0			
Analysis Period (min) 15		1		70.4%	IC	CU Level	of Service	:		С			
	Analysis Period (min)			15									
5 - 1 Land 5.04p	c Critical Lane Group												

4: Fernwood Road & Bradley Boulevard #191

06/08/2021

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			44			4	
Traffic Volume (vph)	71	504	1	3	402	126	5	15	4	292	25	83
Future Volume (vph)	71	504	1	3	402	126	5	15	4	292	25	83
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	12	11	12	12	11	12	12	12	12	12	12	12
Total Lost time (s)		5.0			5.0			5.5			5.5	
Lane Util. Factor		1.00			1.00			1.00			1.00	
Frt		1.00			0.97			0.98			0.97	
Flt Protected		0.99			1.00			0.99			0.96	
Satd. Flow (prot)		1789			1743			1804			1747	
Flt Permitted		0.87			1.00			0.93			0.77	
Satd. Flow (perm)		1569			1739			1693			1391	
Peak-hour factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Adj. Flow (vph)	78	554	1	3	442	138	5	16	4	321	27	91
RTOR Reduction (vph)	0	0	0	0	8	0	0	3	0	0	11	0
Lane Group Flow (vph)	0	633	0	0	575	0	0	22	0	0	428	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		6			2			8			4	
Permitted Phases	6			2			8			4		
Actuated Green, G (s)		45.6			45.6			31.9			31.9	
Effective Green, g (s)		45.6			45.6			31.9			31.9	
Actuated g/C Ratio		0.52			0.52			0.36			0.36	
Clearance Time (s)		5.0			5.0			5.5			5.5	
Vehicle Extension (s)		3.0			3.0			3.0			3.0	
Lane Grp Cap (vph)		813			901			613			504	
v/s Ratio Prot												
v/s Ratio Perm		c0.40			0.33			0.01			c0.31	
v/c Ratio		0.78			0.64			0.04			0.85	
Uniform Delay, d1		17.1			15.3			18.1			25.8	
Progression Factor		1.00			1.00			1.00			1.00	
Incremental Delay, d2		4.7			3.4			0.0			12.6	
Delay (s)		21.9			18.7			18.1			38.5	
Level of Service		С			В			В			D	
Approach Delay (s)		21.9			18.7			18.1			38.5	
Approach LOS		С			В			В			D	
Intersection Summary												
HCM 2000 Control Delay			25.1	H	CM 2000	Level of	Service		С			
HCM 2000 Volume to Capacity	ratio		0.81									
Actuated Cycle Length (s)			88.0		um of los				10.5			
Intersection Capacity Utilization	1		101.6%	IC	CU Level	of Service	:		G			
Analysis Period (min)			15									
c Critical Lane Group												

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Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<u> </u>	Į,	1100	<u>₩Ы</u>	¥	HUIT
Traffic Volume (veh/h)	287	24	35	383	26	42
Future Volume (Veh/h)	287	24	35	383	26	42
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94
Hourly flow rate (vph)	305	26	37	407	28	45
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			331		786	305
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			331		786	305
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			97		92	94
cM capacity (veh/h)			1228		350	735
Direction, Lane #	EB 1	EB 2	WB 1	NB 1		
Volume Total	305	26	444	73		
Volume Left	0	0	37	28		
Volume Right	0	26	0	45		
cSH	1700	1700	1228	517		
Volume to Capacity	0.18	0.02	0.03	0.14		
Queue Length 95th (ft)	0	0	2	12		
Control Delay (s)	0.0	0.0	1.0	13.1		
Lane LOS			Α	В		
Approach Delay (s)	0.0		1.0	13.1		
Approach LOS				В		
Intersection Summary						
Average Delay			1.6			
Intersection Capacity Utiliz	zation		51.2%	IC	U Level c	f Service
Analysis Period (min)			15			

3: Burdette Road & Bradley Boulevard #191

06/08/2021

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (vph)	25	276	17	38	370	36	15	29	37	24	32	48
Future Volume (vph)	25	276	17	38	370	36	15	29	37	24	32	48
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	12	12	12	12	12	12	12	13	12	12	14	12
Total Lost time (s)		5.0			5.0			5.0			5.0	
Lane Util. Factor		1.00			1.00			1.00			1.00	
Frt		0.99			0.99			0.94			0.94	
Flt Protected		1.00			1.00			0.99			0.99	
Satd. Flow (prot)		1842			1835			1790			1842	
Flt Permitted		0.95			0.95			0.93			0.92	
Satd. Flow (perm)		1760			1754			1678			1717	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	27	300	18	41	402	39	16	32	40	26	35	52
RTOR Reduction (vph)	0	1	0	0	2	0	0	35	0	0	42	0
Lane Group Flow (vph)	0	344	0	0	480	0	0	53	0	0	71	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		6			2			8			4	
Permitted Phases	6			2			8			4		
Actuated Green, G (s)		55.5			55.5			8.6			8.6	
Effective Green, g (s)		55.5			55.5			8.6			8.6	
Actuated g/C Ratio		0.75			0.75			0.12			0.12	
Clearance Time (s)		5.0			5.0			5.0			5.0	
Vehicle Extension (s)		6.0			6.0			6.0			3.0	
Lane Grp Cap (vph)		1318			1313			194			199	
v/s Ratio Prot												
v/s Ratio Perm		0.20			c0.27			0.03			c0.04	
v/c Ratio		0.26			0.37			0.27			0.36	
Uniform Delay, d1		2.9			3.2			29.9			30.2	
Progression Factor		1.00			1.00			1.00			1.00	
Incremental Delay, d2		0.3			0.8			2.1			1.1	
Delay (s)		3.2			4.0			32.0			31.3	
Level of Service		Α			Α			С			С	
Approach Delay (s)		3.2			4.0			32.0			31.3	
Approach LOS		А			А			С			С	
Intersection Summary												
HCM 2000 Control Delay			9.1	Н	CM 2000	Level of	Service		Α			
HCM 2000 Volume to Capacit	y ratio		0.36									
Actuated Cycle Length (s)			74.1	S	um of lost	t time (s)			10.0			
Intersection Capacity Utilization	n		48.7%	IC	CU Level	of Service)		А			
Analysis Period (min)			15									
c Critical Lane Group												

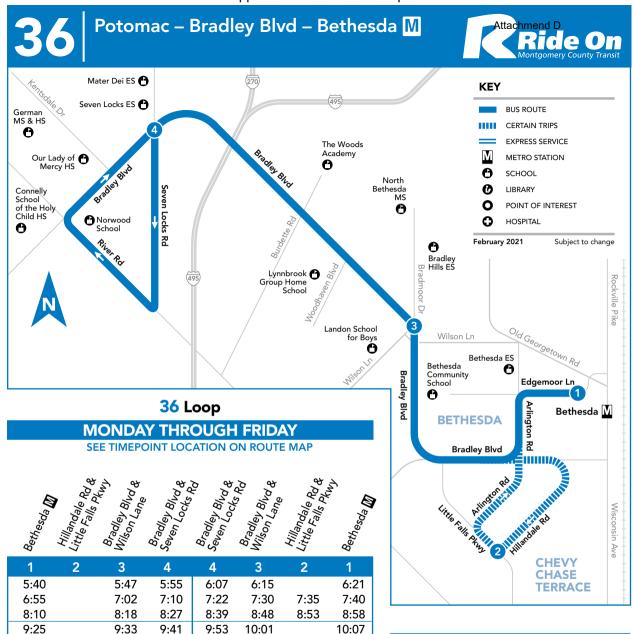
4: Fernwood Road & Bradley Boulevard #191

06/08/2021

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (vph)	51	268	6	5	368	234	10	19	3	146	20	76
Future Volume (vph)	51	268	6	5	368	234	10	19	3	146	20	76
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	12	11	12	12	11	12	12	12	12	12	12	12
Total Lost time (s)		5.0			5.0			5.5			5.5	
Lane Util. Factor		1.00			1.00			1.00			1.00	
Frt		1.00			0.95			0.99			0.96	
Flt Protected		0.99			1.00			0.99			0.97	
Satd. Flow (prot)		1782			1706			1812			1732	
Flt Permitted		0.86			1.00			0.90			0.80	
Satd. Flow (perm)		1542			1703			1658			1420	
Peak-hour factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Adj. Flow (vph)	53	276	6	5	379	241	10	20	3	151	21	78
RTOR Reduction (vph)	0	0	0	0	13	0	0	2	0	0	22	0
Lane Group Flow (vph)	0	335	0	0	612	0	0	31	0	0	228	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		6			2			8			4	
Permitted Phases	6			2			8			4		
Actuated Green, G (s)		45.2			45.2			16.5			16.5	
Effective Green, g (s)		45.2			45.2			16.5			16.5	
Actuated g/C Ratio		0.63			0.63			0.23			0.23	
Clearance Time (s)		5.0			5.0			5.5			5.5	
Vehicle Extension (s)		3.0			3.0			3.0			3.0	
Lane Grp Cap (vph)		965			1066			378			324	
v/s Ratio Prot												
v/s Ratio Perm		0.22			c0.36			0.02			c0.16	
v/c Ratio		0.35			0.57			0.08			0.70	
Uniform Delay, d1		6.4			7.9			21.9			25.6	
Progression Factor		1.00			1.00			1.00			1.00	
Incremental Delay, d2		0.2			2.2			0.1			6.7	
Delay (s)		6.7			10.1			22.0			32.3	
Level of Service		A			B			C			C	
Approach Delay (s) Approach LOS		6.7 A			10.1 B			22.0 C			32.3 C	
Intersection Summary												
HCM 2000 Control Delay			14.0	H	CM 2000	Level of	Service		В			,
HCM 2000 Volume to Capacit	y ratio		0.61									
Actuated Cycle Length (s)			72.2	Sı	um of lost	t time (s)			10.5			
Intersection Capacity Utilization	n		79.6%			of Service)		D			
Analysis Period (min)			15									
c Critical Lane Group												

Higher Ground Education, Inc July 2021

APPENDIX H BUS ROUTES/MAPS



Please arrive at your stop several minutes ahead of your bus' scheduled arrival.

Since safe service is a priority at Ride On, buses may be delayed dueq to traffic or weather.

There is NO Saturday or Sunday service on this route

NOTES:

10:40

11:55

1:10

2:25

3:40

4:55

6:10

7:30

5:02

6:17

10:48

12:03

1:18

2:33

3:48

5:08

6:23

7:37

10:56

12:11

1:26

2:41

3:56

5:16

6:31

7:45

11:08

12:23

1:38

2:53

4:08

5:28

6:43

7:57

11:16

12:31

1:46

3:01

4:17

5:37

6:52

8:04

AM

11:22

12:37

1:52

3:07

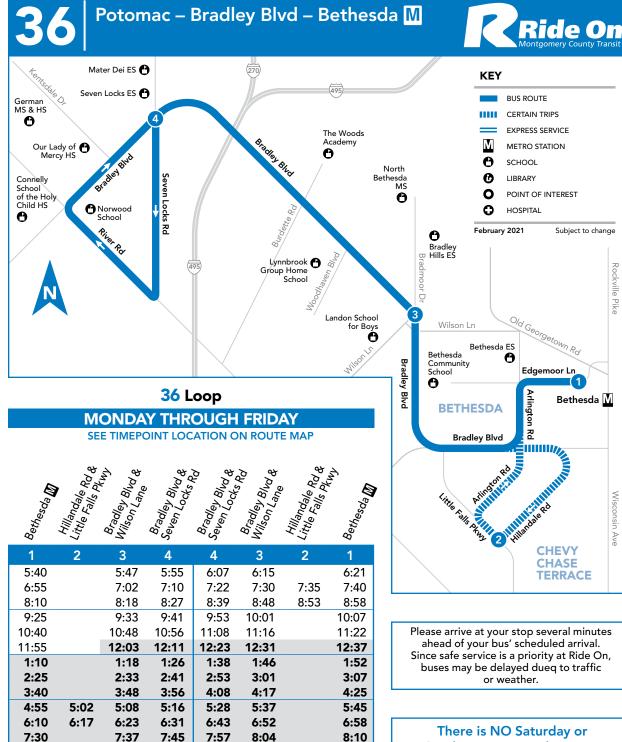
4:25

5:45

6:58

8:10

PM



HOW TO RIDE A RILLING D

Check schedule for timepoint nearest your location. Wait at the blue and white RIDE ON bus stop sign. Arrive several minutes before scheduled time. Have exact fare ready (drivers do not make change).

- Not all stops are listed on a public timetable.
- If you are unfamiliar with your stop, sit or stand behind the line near the front of the bus and ask the bus driver to notify you when your stop is approaching.
- Ask the bus driver if you are not sure if the bus goes to your stop.
- If you have internet access (at home or somewhere else, such as a public library), it may be easier for you to use an online trip planner rather than a paper timetable.
- Be mindful of changes in the schedule, for holidays or bad weather.
- Please observe the following rules for all patrons: No eating, drinking, or smoking.
- Electronic devices may be played with earphones set at low level.

HOW TO READ A TIMETABLE

- Find the schedule for the day of the week and the direction you wish to ride.
- Find the timepoints closest to your origin and destination. The timepoints are shown on the route map and indicate the time the bus is scheduled to be at the particular location. Your nearest bus stop may be between timepoints.
- Read down the column to see the times when a trip will be at the given timepoint. Read the times across to the right to see when the trip reaches other timepoints.

Montgomery County assures that no person shall, on the grounds of race, color, or national origin, as provided by Title VI of the Civil Rights Act of 1964 and the Civil Rights Act of 1987, be excluded from the participation in, be denied the benefits of, or be otherwise subjected to discrimination under any program or activity. For more information of the file a complaint, please contact the Montgomery County Office of Human Rights.

There is NO Saturday or Sunday service on this route

РМ

AM

NOTES:

Regular Fare, Token, or SmarTrip® \$2.00 SmarTrip® Fare Transfer from MetroRail \$1.50 Seniors and persons with disability with valid ID (including attendant-eligible) except during free periods: Senior/Disabled SmarTrip® or Cash \$1.00 Senior/Disabled SmarTrip $^{\circledR}$ Transfer from Metrorail \$0.50 Seniors age 65 years or older with a Senior SmarTrip® card or valid Metro Senior ID Card or with valid Medicare Card and Photo ID from 9:30 am - 3:00 pm Monday through Friday and Saturday from 8:30 am - 4:00 pm.

Person with disability with Metro Disabled ID Card from 9:30 am - 3:00 pm Monday through Friday and Saturday from 8:30 am - 4:00 pm.

Person with disability with Metro Disability ID Card – Attendant Eligible from 9:30 am – 3:00 pm Mon. through Fri. and Sat. from 8:30 am - 4:00 pm. Attendant rides half fare or free depending on time.

MetroAccess - Certified Customer with ID MetroAccess - Companion

Children under age 5

Anytime

Local Bus Transfer with SmarTrip®

Children 5 to 18 with a Youth Cruiser SmarTrip® Card or student ID

FREE

FREE

GUARANTEED RIDE HOME

When you take Metrobus, Metrorail and Ride On to work, you are eligible to participate in the free Commuter Connections Guaranteed Ride Home Program. To register and to receive program details call:

Commuter Services at 301-770-POOL(7665).

METROACCESS

Alternative paratransit service to this Ride On route for people with certified disabilities is available. Call MetroAccess at 301-562-5360.



Appendix H: Bus Routes/Maps

WELCOME TO RIDE ON

RIDE ON is a community bus service operated by the Montgomery County Department of Transportation.

RIDE ON operates over 75 routes that serve all 13 Montgomery County Metrorail stations and 7 MARC stations.

For detailed information, or to have timetables mailed, call 311.

Outside Montgomery County 240-777-0311

Visit our web site at: www.rideonbus.com

Real Time information is available at:

www.rideonrealtime.com

Regular Mailing Address:

Montgomery County DOT Division of Transit Services 101 Monroe Street, 5th Floor Rockville, MD 20850

For more information, or to request this document in an alternate format or translated into another language, please call 311, or outside Montgomery County 240-777-0311.

Para más información o para pedir este documento en un formato diferente o traducido a otro idioma, por favor, llame al 311 o de fuera del Condado de Montgomery al 240-777-0311.

如需更多信息、需要以其它格式提供本文檔或需要將本文檔翻 譯成其它語言,請撥打311。如果您不在蒙哥馬利郡,請撥打 240-777-0311.

자세한 정보를 원하시거나 본 문서를 다른 형식 또는 다른 언어로의 번 역본으로 원하실 경우, 전화번호 311, 또는 몽고메리 카운티 이외의 지역 에서는 240-777-0311로 연락하시기 바랍니다.

ስተጨማሪ መረጃ፣ ወይም ይህንን ዶኩመንት በተስዋጭ መልክ ስመጠየቅ ወይም ወደሌላ ቋንቋ ስማስተርንም፣ ስባከዎትን በ 311 ወይም ከሞንትንመሪ ካውንቲ ውጪ 240-777-0311 ይደውሉ።

Pour plus d'informations ou pour recevoir un exemplaire de ce document sous un format différent ou traduit dans une autre langue, veuillez composer le 311 ou le 240-777-0311, à l'extérieur du comté de Montgomery.

Để tìm hiểu thêm, hoặc để yêu cầu cung cấp tài liệu này theo định dạng khác hay chuyển ngữ sang ngôn ngữ khác, vui lòng gọi 311 hoặc số 240-777-0311 nếu gọi từ bên ngoài Quân Montgomery.

HOLIDAY SCHEDULE

Weekday Schedule operates on Indigenous Peoples' Day Saturday Schedule operates on Independence Day

Sunday Schedule operates on New Year's Day, Memorial Day, Labor Day, Thanksgiving Day, Christmas Day Special Schedule operates on MLK, Jr. Day, Presidents' Day,

Veterans Day

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Subscribe to text alerts by texting MONTGOMERY RIDEON to 468311

voutube.com/RideOnMCT

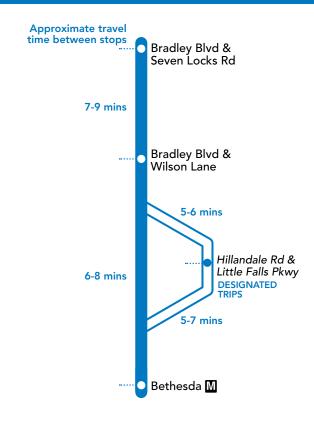


Thank You for Riding with Us!

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EFFECTIVE: FEBRUARY 28, 2021





SERVICE DAYS

MONDAY - FRIDAY



Telephone 311 Online at www.rideorph192.com Real Time Info at www.rideonrealtime.com